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# **Tamworth Regional Council**

## **Report for Manilla Flood Study**

April 2012  
FINAL



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## Glossary

**Annual Exceedance Probability (AEP)** - AEP (measured as a percentage) is a term used to describe flood size. AEP is the long-term probability between floods of a certain magnitude. For example, a 1% AEP flood is a flood that occurs on average once every 100 years. It is also referred to as the '100 year flood' or 1 in 100 year flood'. The terms 100-year flood, 50-year flood, 20-year flood etc, have been used in this study. See also average recurrence interval (ARI);

1e-4% (approx) AEP sometimes referred to as the PMF Event

0.2% AEP sometimes referred to as the 1 in 500 year ARI Event

1% AEP sometimes referred to as the 1 in 100 year ARI Event

2% AEP sometimes referred to as the 1 in 50 year ARI Event

5% AEP sometimes referred to as the 1 in 20 year ARI Event

10% AEP sometimes referred to as the 1 in 10 year ARI Event

20% AEP sometimes referred to as the 1 in 5 year ARI Event

**Afflux** - The increase in flood level upstream of a constriction of flood flows. A road culvert, a pipe or a narrowing of the stream channel could cause the constriction.

**Australian Height Datum (AHD)** - A common national plane of level approximately equivalent to the height above sea level. All flood levels; floor levels and ground levels in this study have been provided in meters AHD.

**Average annual damage (AAD)** - Average annual damage is the average flood damage per year that would occur in a nominated development situation over a long period of time.

**Average recurrence interval (ARI)** - ARI (measured in years) is a term used to describe flood size. It is a means of describing how likely a flood is to occur in a given year. For example, a 100-year ARI flood is a flood that occurs or is exceeded on average once every 100 years. The terms 100-year flood, 50-year flood, 20-year flood etc., have been used in this study. See also annual exceedance probability (AEP).

**Catchment** - The land draining through the main stream, as well as tributary streams.

**Development Control Plan (DCP)** - A DCP is a plan prepared in accordance with Section 72 of the *Environmental Planning and Assessment Act, 1979* that provides detailed guidelines for the assessment of development applications.

**Design flood level** - A flood with a nominated probability or average recurrence interval, for example the 1% AEP flood is commonly use throughout NSW.

**OEH (formerly DECCW, DECC, DNR, DLWC, DIPNR)** - Office of Environment and Heritage. Covers a range of conservation and natural resources science and programs, including native vegetation, biodiversity and environmental water recovery to provide an integrated approach to natural resource management. This department was formed in April 2007.

**Discharge** - The rate of flow of water measured in terms of volume per unit time, for example, cubic metres per second (m<sup>3</sup>/s). Discharge is different from the speed or velocity of flow, which is a measure of how fast the water is moving.

**Ecologically sustainable development (ESD)** - Using, conserving and enhancing natural resources so that ecological processes, on which life depends, are maintained, and the total quality of life, now and in the future, can be maintained or increased. A more detailed definition is included in the Local Government Act 1993.



**Effective warning time** - The time available after receiving advice of an impending flood and before the floodwaters prevent appropriate flood response actions being undertaken. The effective warning time is typically used to move farm equipment, move stock, raise furniture, evacuate people and transport their possessions.

**Emergency management** - A range of measures to manage risks to communities and the environment. In the flood context it may include measures to prevent, prepare for, respond to and recover from flooding.

**EP&A Act** - Act Environmental Planning and Assessment Act, 1979

**Extreme flood** - An estimate of the probable maximum flood (PMF), which is the largest flood likely to occur.

**Flood** - A relatively high stream flow that overtops the natural or artificial banks in any part of a stream, river, estuary, lake or dam, and/or local overland flooding associated with major drainage before entering a watercourse, and/or coastal inundation resulting from super-elevated sea levels and/or waves overtopping coastline defences excluding tsunami.

**Flood awareness** - An appreciation of the likely effects of flooding and knowledge of the relevant flood warning, response and evacuation procedures.

**Flood hazard** - The potential for damage to property or risk to persons during a flood. Flood hazard is a key tool used to determine flood severity and is used for assessing the suitability of future types of land use.

**Flood level** - The height of the flood described either as a depth of water above a particular location (e.g. 1m above a floor, yard or road) or as a depth of water related to a standard level such as Australian Height Datum (e.g. the flood level was 7.8m AHD). Terms also used include flood stage and water level.

**Flood liable land** - Land susceptible to flooding up to the Probable Maximum Flood (PMF). Also called flood prone land. Note that the term flood liable land now covers the whole of the floodplain, not just that part below the flood planning level, as indicated in the superseded Floodplain Development Manual (NSW Government, 2005).

**Flood Planning Levels (FPLs)** - The combination of flood levels and freeboards selected for planning purposes, as determined in floodplain management studies and incorporated in floodplain management plans. The concept of flood planning levels supersedes the designated flood or the flood standard used in earlier studies.

**Flood Prone Land** - Land susceptible to flooding up to the Probable Maximum Flood (PMF). Also called flood liable land.

**Flood Proofing** - A combination of measures incorporated in the design, construction and alteration of individual buildings or structures subject to flooding, to reduce or eliminate damages during a flood.

**Flood stage** see flood level.

**Flood Study** - A study that investigates flood behaviour, including identification of flood extents, flood levels and flood velocities for a range of flood sizes.

**Floodplain** - The area of land that is subject to inundation by floods up to and including the Probable Maximum Flood event, that is, flood prone land or flood liable land.

**Floodplain Risk Management Study** – Studies carried out in accordance with the Floodplain Development Manual and assess options for minimising the danger to life and property during floods.

**Floodplain Risk Management Plan** - The outcome of a Floodplain Management Risk Study.

**Floodway** - Those areas of the floodplain where a significant discharge of water occurs during floods. Floodways are often aligned with naturally defined channels. Floodways are areas that, even if only partially blocked, would cause a significant redistribution of flood flow, or a significant increase in flood levels.



**Freeboard** - A factor of safety expressed as the height above the design flood level. Freeboard provides a factor of safety to compensate for uncertainties in the estimation of flood levels across the floodplain, such as wave action, localised hydraulic behaviour and impacts that are specific event related, such as levee and embankment settlement, and other effects such as “greenhouse” and climate change.

**High Flood Hazard** - For a particular size flood, there would be a possible danger to personal safety, able-bodied adults would have difficulty wading to safety, evacuation by trucks would be difficult and there would be a potential for significant structural damage to buildings.

**Hydraulics Term** - given to the study of water flow in waterways, in particular, the evaluation of flow parameters such as water level and velocity.

**Hydrology Term** - given to the study of the rainfall and runoff process; in particular, the evaluation of peak discharges, flow volumes and the derivation of hydrographs (graphs that show how the discharge or stage/flood level at any particular location varies with time during a flood).

**LGA** - Local Government Area, or Council boundary.

**Local catchments** - Local catchments are river sub-catchments that feed river tributaries, creeks, and watercourses and channelised or piped drainage systems.

**Local Environmental Plan (LEP)** – A Local Environmental Plan is a plan prepared in accordance with the *Environmental Planning and Assessment Act*, 1979, that defines zones, permissible uses within those zones and specifies development standards and other special matters for consideration with regard to the use or development of land.

**Local overland flooding** - Local overland flooding is inundation by local runoff within the local catchment.

**Local runoff** - local runoff from the local catchment is categorised as either major drainage or local drainage in the NSW Floodplain Development Manual, 2005.

**Low flood hazard** - For a particular size flood, able-bodied adults would generally have little difficulty wading and trucks could be used to evacuate people and their possessions should it be necessary.

**Flows or discharges** - It is the rate of flow of water measured in terms of volume per unit time.

**Merit approach**- The principles of the merit approach are embodied in the *Floodplain Development Manual* (NSW Government, 2005) and weigh up social, economic, ecological and cultural impacts of land use options for different flood prone areas together with flood damage, hazard and behaviour implications, and environmental protection and well being of the State's rivers and floodplains.

**Overland flow path** - The path that floodwaters can follow if they leave the confines of the main flow channel. Overland flow paths can occur through private property or along roads. Floodwaters travelling along overland flow paths, often referred to as 'overland flows', may or may not re-enter the main channel from which they left — they may be diverted to another watercourse.

**Peak discharge** - The maximum flow or discharge during a flood.

**Present value** - In relation to flood damage, is the sum of all future flood damages that can be expected over a fixed period (usually 20 years) expressed as a cost in today's value.

**Probable Maximum Flood (PMF)** - The largest flood likely to ever occur. The PMF defines the extent of flood prone land or flood liable land, that is, the floodplain.

**Reliable access** - During a flood, reliable access means the ability for people to safely evacuate an area subject to imminent flooding within effective warning time, having regard to the depth and velocity of floodwaters, the suitability of the evacuation route, and other relevant factors.

**REP** - Regional Environmental Plan. A plan prepared in accordance with the EPA Act that provides objectives and controls for a region, or part of a region. For example, the Georges River REP.



**Risk** - Chance of something happening that will have an impact. It is measured in terms of consequences and likelihood. In the context of this study, it is the likelihood of consequences arising from the interaction of floods, communities and the environment.

**RORB/RAFTS** - The software programs used to develop a computer model that analyses the hydrology (rainfall-runoff processes) of the catchment and calculates hydrographs and peak discharges. Known as a hydrological models.

**Runoff** - the amount of rainfall that ends up as flow in a stream, also known as rainfall excess.

**SES** - State Emergency Service of New South Wales

**Stage–damage curve** - A relationship between different water depths and the predicted flood damage at that depth.

**Velocity** - the term used to describe the speed of floodwaters, usually in m/s (metres per second).  
10km/h = 2.7m/s.

**Water surface profile** - A graph showing the height of the flood (flood stage, water level or flood level) at any given location along a watercourse at a particular time.



# 1. Introduction

## 1.1 NSW Flood Prone Land Policy

The primary objective of the New South Wales Government's Flood Prone Land Policy is to reduce the impact of flooding and flood liability on individual owners and occupiers of flood prone property, and to reduce private and public losses resulting from floods, utilising ecologically positive methods wherever possible.

Through the Office of Environment and Heritage (OEH) previously the Department of Environment, Climate Change and Water (DECCW) the Department of Planning (DoP) and the State Emergency Service (SES), the NSW Government provides technical assistance to local government on all flooding and land use planning matters. The Floodplain Development Manual (NSW Government, 2005) is provided to assist Councils to meet their obligations through the preparation of floodplain risk management plans. Figure 1-1 from the Manual documents the process for plan preparation, implementation and review. Tamworth Regional Council is responsible for local land use planning in Manilla. Through its Floodplain Risk Management Committee, Tamworth Regional Council proposes to prepare a comprehensive floodplain risk management plan for the study area in accordance with the NSW Government's "Floodplain Development Manual: the management of flood liable land", April 2005.

## 1.2 Key Issue

Manilla town is located at the confluence of the Manilla and Namoi Rivers (Appendix A) which drain to Keepit Dam in the New England region of NSW. Manilla is located 43 km to the northwest of Tamworth and has population of approximately 2100 (Australian Bureau of Statistics Census Data, 2006). The two rivers draining through Manilla command a considerable catchment area of some 5150 km<sup>2</sup>. The catchments are primarily rural, with a number of towns located in the catchment.

Floodwaters in both catchments tend to rise quickly and isolate communities and properties for several days. Many houses can be inundated in flood events necessitating evacuations. Some rainfall and river gauging data in the catchment is available, and significant events have been recorded in Manilla. Significant events noted in the literature and mentioned by residents, are the 1955 event and the 1964 flood. The 1964 flood was considered the worst in 100 years at the time.

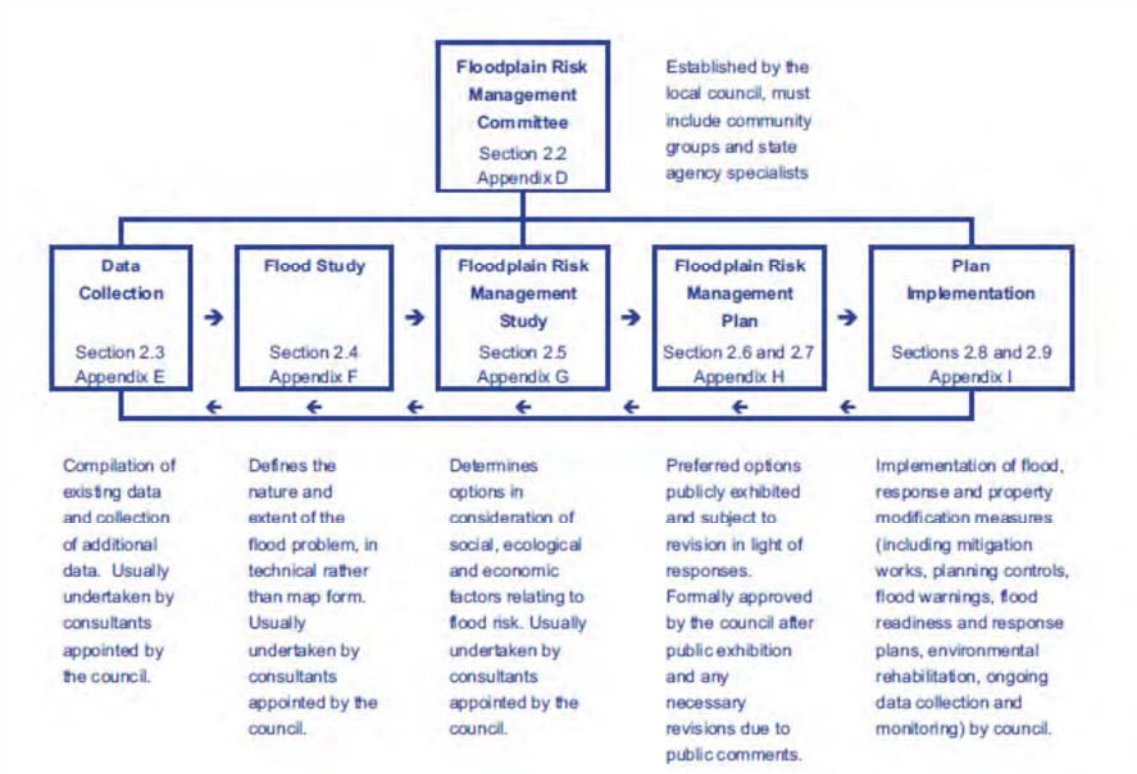
## 1.3 Study Objectives

The primary objective of this study was to define the main-stem flood behaviour under historical conditions and design flood behaviour under existing and future climate conditions in the study area. The study produced information on flood levels, depths, velocities, flows, hydraulic categories, and provisional hazard categories for a full range of design and historical flood events. In addition, the study produced estimates of flood damage.

To achieve this objective, the study collected, compiled and reviewed all available relevant data (including survey, aerial photography and satellite imagery). The design events included the 0.5%, 1%, 2%, 10% and 20% AEP events together with the Probable Maximum Flood (PMF). In addition, the potential impacts of climate change on flooding were assessed together with a number of other sensitivity analysis.



Hydrologic and hydraulic modelling was undertaken to satisfy the study objectives. The models and results produced in this study are intended to form the basis for a subsequent floodplain risk management studies by Tamworth Regional Council, where detailed assessment of flood mitigation options and floodplain risk management measures will be undertaken.



**Figure 1-1 Flood Plain Risk Management Process (NSW Government, 2005)**



## 2. Background

### 2.1 Catchment Description

The upper catchments of the Manilla and Namoi River catchments are located to the northwest of Tamworth in the New England region of NSW (Appendix A). Both rivers drain large catchments (2075 km<sup>2</sup> and 3075 km<sup>2</sup> respectively) before they confluence at Manilla, approximately 25 km upstream of Keepit Dam on the Namoi River.

The Manilla River headwaters are located near Split Yard Mountain in the Nandewar Range and drains in a north easterly direction to the Plumthorpe area, before turning south-east towards the town of Barraba. Approximately 20km south of Barraba, the Manilla River discharges into Split Rock Dam (storage volume 400 GL). From Split Rock Dam, the Manilla River drains southwards for a further 25km before it confluent with the Namoi River within the town of Manilla.

The Namoi River headwaters are located in the Warrahbah National Park and drains in a south westerly direction to the confluence with the Manilla River. Near the headwaters, the Namoi River has its confluences with the Macdonald River, which drains a large (3075 km<sup>2</sup>) catchment. The Macdonald River headwaters are located in the Dividing Range approximately 70km west of Tamworth. It discharges through the towns of Woolbrooke and Bendemeer along upstream of the confluence with the Namoi River.

A number of significant tributaries discharge to the Manilla, Namoi and Macdonald River as follows:

- Barraba Creek at Barraba;
- Bohra Creek at Upper Manilla;
- Yarramanbully and Halls Creek near Manilla; and
- A number of tributaries along the length of the MacDonald River.

The catchments in the upstream reaches of these creeks are generally steep and heavily forested. Lower reaches are mostly rural in nature. In many areas, particularly towards Manilla, the river generally has a deep, well defined, channel with a wide floodplain. River slopes vary from 1% to 2%.

A number of significant floods have occurred in Manilla as follows:

- A significant event in 1840;
- A significant event in February 1864 in which 4 out of the 12 residents were drowned;
- A significant event in 1910;
- Since 1941 when gauging started there have been 16 minor, 3 moderate and 2 major floods;
- The February 1955 flood was a major event and this was noted to be the highest since the 1910 event;
- The January 1964 was also a major event and is enshrined in many residents' memories. It was noted as the worst in 100 years; and
- More recent flood events occurred in 1971, 1984, 1998 and 2004.

Floodwaters in both catchments tend to rise quickly and isolate communities and properties for several days. Many houses can be inundated in flood events necessitating evacuations. The nature of flooding





varies considerably from in-stream flood ways to areas where the floodwaters inundate floodplains at bends in the river and where floodwaters backup into the lower reaches of tributary creeks. There are a number of rainfall and river gauging data in the catchment, with records dating back as far as 1916. While this data is available, its usefulness to inform the flood study is discussed in ensuing sections of this report.

## 2.2 Previous Studies

The 2007 study, *Assessment of Flood Risk in Various Towns and Villages* (Bewsher 2007), provides a comprehensive synopsis of the historical flooding at Manilla and was undertaken with the following scope:

*Tamworth Regional Council commissioned Bewsher Consulting Pty Ltd to conduct a preliminary assessment of the flood problem at 11 towns and villages throughout the Local Government Area (LGA). The principle aim of the assessment was to identify flood problems, prioritise the towns and villages according to the general scope of the problem, and to develop a strategic plan for the preparation of detailed flood studies and floodplain management studies and plans, as stipulated in the New South Wales Government's Floodplain Development Manual (April 2005).*

*A prioritised plan for future flood studies and floodplain management studies and plans within the Tamworth Regional Council LGA was presented. The principal factor for allocating priorities was the number of buildings located within the historic flood extent. The typical flood velocities and available warning times at each location were also been considered. Another important consideration is the frequency of the historical event under consideration.*

*By this method, the town of Manilla was given a high priority for future studies. The 1964 flood inundated many homes, with significant flood depths and velocities. While the construction of Split Rock Dam on the Manilla River in 1988 may have lessened the frequency of flooding from the Manilla River, the location of relatively large number of houses on the floodplain is noted as a cause for concern. (Bewsher 2007),*

The 2007 study, *Assessment of Flood Risk in Various Towns and Villages* provided the following synopsis of flooding at Manilla:

*With a population of about 2,000 people, Manilla is the largest of 11 towns and villages covered in this study. The town is located at the junction of the Manilla River and Namoi River (combined catchment area 5,130 km<sup>2</sup>) (Figure 3.2).*

*Manilla had just been surveyed when on 12-13 February 1864 a flood drowned four of the 12 inhabitants and destroyed houses and Veness' store (built in 1853 near the foot of Market Street). Appendix A contains an article about the flood originally published in 1949 and reprinted in the Manilla Express in 1964. Surveyor Arthur Dewhurst estimated that the flood peak was ten feet (about 3m) higher than the previous record flood in 1840. Mr Macleod, author of the article in 1949, put it "at least 5 feet higher" than the big 1840 and 1910 floods. Significantly he adds:*

*"What made it more dangerous was the fact that it rose suddenly at night when the inhabitants were asleep".*

*Since 1941, flows in the Namoi River at Manilla Railway Bridge have been gauged. During this time, 21 floods have exceeded the "minor" flood designation at Manilla (16 minor, 3 moderate, 2 major) (Manilla Local Flood Plan, p.A-4). The major floods occurred on 25 February 1955 and 14 January 1964.*



*The February 1955 flood was the highest since January 1910 and is said to have affected three homes, with the little damage to property. The January 1964 flood is said to have affected about 60 properties, which compares to 137 buildings estimated to be located within the flood footprint by using the mapped flood extent and SPOT image. Possibly many of these properties experienced only shallow ground flooding and so were not considered “affected”.*

*The Manilla Express described the 1964 flood as the worst for 100 years. The following details are gleaned from the newspapers of the day:*

- *About one-third of the town had to be evacuated as floodwaters rose and entered low lying areas of the town shortly after 2pm (on Tuesday 14 January);*
- *All the families in North Manilla, Namoi Street, River Street, the lower ends of Rowan and Market Streets, and the low-lying areas of West Manilla were evacuated;*
- *At one stage the rate of rise was estimated at 18 inches (0.45m) every ten minutes;*
- *The flood peaked at about 6pm and receded quickly;*
- *Homes in the worst hit areas were almost complete covered by floodwaters;*
- *Some houses were lifted off their foundations while others were partially demolished;*
- *A small house near the corner of Court and River Streets was swept away;*
- *In River Street the PMG line staff offices and store rooms were completely swept away leaving only the foundations to indicate that a building had once stood there, and the showground was devastated;*
- *A large tree broke its way through the front double windows of Mr Ridley’s home, and a television and piano were destroyed and a bedroom suite floated through the window;*
- *An elderly couple in West Manilla had to spend the night in the ceiling of their house when floodwaters prevented a rescue boat from reaching them;*
- *Manilla’s loss was expected to exceed £250,000;*
- *One poultry farmer at West Manilla lost 2,000 fowls valued at £1,500-£2,000.*

*Mr Toby Grant, long-time resident at North Manilla, was interviewed to gain an appreciation of the behaviour of the 1964 flood. At 25 River Street, the flood reached a depth of about 1.5m over the floor which is raised about 0.3m above the ground. The flood reached a level of 1ft below the top of the petrol bowzers at the North Manilla store. About 20 houses were inundated above flood level in North Manilla, When a log hit the door to 25 River Street, the force of water broke the wall on the opposite side. The torrent was such that a neighbour’s house was “washed away”, another house was washed off its foundations, and a cement booth with 6 inch thick walls was washed into the middle of the arena at the showground.*

*Photographs of the 1964 flood confirm the severe depths of flooding. Only the roofs remained above water at some houses.*

*Newspaper reports of people forced to climb onto roofs and be rescued by boat suggests little prior warning of the flood. However, it is noted that the Manilla Shire Clerk was notified of the rising floodwater upstream at Barraba at 9am on the day of the flood, and a radio announcement was made (Barraba Chronicle, Wednesday January 22, 1964, p.2). The timing of the 1964 flood was fortunate, in contrast to the 1864 flood. It appears that this was a key to the different consequences in terms of loss of life:*



*"Perhaps the one thing we have to be thankful for is the fact that this disaster for our town struck when it did – in the daylight. Had it come in darkness we shudder to think what the consequences could have been" (Manilla Express, Wednesday January 22, 1964, p.8, editorial).*

*"Had the flood occurred at night, there must have been enormous loss of life". (Bignall, 1980, p.36).*

*The frequency of the 1964 flood was estimated as a 50-100 year ARI event by LM&P (1982, p3-31). The construction of Split Rock Dam in 1988 on the Manilla River about 30km upstream from Manilla might mitigate floods at Manilla – LM&P (1982, p3-32) offer the following judgement:*

*"It is conceivable that if Split Rock storage was operating it could have effected a significant reduction in the record 1964 flood and in the 1974 flood. On the other hand, the volumes involved in the 1955 and 1971 flood were so great that Split Rock storage may not have achieved much mitigation of peak flow rates".*

*Split Rock Dam may be useful in mitigated floods even if full due to the routing of the flood through the storage, especially if as suggested in the Manilla Local Flood Plan (pB-1) the synchronisation of flows between the Manilla and Namoi Rivers exert an important influence on flood behaviour at Manilla.*

*"The most severe flooding in Manilla has occurred when high flows in the Namoi River were backed up or held up by a major flood in the Manilla River"*

*However, Toby Grant insists that the Namoi River was not in (major) flood in the 1964 event. Historical flood levels at the North Ceurindi gauge on the Namoi River about 16km upstream from its junction with the Manilla River lend some support to this view (see Table 2.3).*

*Today, adequate warning of floods should be available in Manilla. According the NSW State Flood Plan, the Bureau of Meteorology is required to provide seven hours' notice of flooding. The lessons of the 1864 and 1964 floods indicate that a flood warning system that reaches every household, combined with updated emergency plans and a flood-aware community, are vital tools to reduce the threat of life at Manilla. (Bewsher 2007).*

## 3. Data

### 3.1 Introduction

For the purposes of undertaking the flood study, and to calibrate models, it was necessary to source the following key data:

- Concurrent rainfall and runoff data for significant flood events that can be used for calibration of the hydrological model parameters;
- Pluviographic rainfall data at 6 minute intervals to provide information on historic storm temporal patterns;
- Daily rainfall data to provide spatial distribution of rainfall events;
- Runoff gauge data, including gauge history and rating curves, to determine hydrographs of flood events; and
- Topographic survey data for the compilation of flood models and for the purposes of flood modelling.

### 3.2 River Data

#### 3.2.1 Runoff Gauges

A number of river gauges are located along the Manilla, Namoi and Macdonald Rivers (Refer to Appendix B). A number of these gauges were suitable for calibration, as tabulated below, as they:

- Are located at appropriate positions along the river channels for the purposes of the study;
- Had captured significant flood events;
- Had reasonable gauging data and rating tables to provide information on reported hydrographs during flood events. It is important to note, that while operational dates may span a number of years, for some of the gauges earlier measurements were only captured as daily totals; and
- Had data periods that were concurrent with pluviographic rainfall data in the catchment.

**Table 3-1 Runoff Gauge Data**

Gauge Number	Gauge Name	River	Operational Dates	Comments
419005	Namoi River At North Cuerindi	Namoi	01/01/1916 – 01/01/2009	Namoi River 16 km Upstream Of Manilla Confluence
419020	Manilla River At Brabri	Manilla	01/01/1948 – 01/01/2010	Manilla River 6 km Upstream Of Manilla Confluence
419022	Namoi River At Manilla Railway Bridge	Namoi	01/01/1952 – 01/01/2010	Namoi River 0.7 km Downstream Of Manilla Confluence
419053	Manilla River At Black Springs	Manilla	01/01/1972 – 01/01/2010	Manilla River 0.9 km Upstream Of Split Rock Dam



### 3.2.2 Flood Frequency Analysis

Flood frequency analysis was undertaken using the data provided by the Pinneena (Version 9.3) software, for the Namoi River at Manilla Railway Bridge gauge (419022). While the Pinneena software provides the ability to undertake flood frequency analysis, this has no ability to screen data and assess impacts of data outliers. To this end the flood frequency analysis was undertaken manually. In respect to the flood frequency, the following was noted referring to Appendix B:

- ▶ Daily flow measurements were undertaken for the full record from approximately 1953 onwards. From 1974 onwards automatic stage recordings were available, providing a better measurement of the instantaneous flood peak. For Manilla, which experiences rapidly rising water levels in a flood, the instantaneous flood peak could be larger than the average daily measurement;
- ▶ Of the gaugings undertaken from 19/03/1941 to 14/10/2009 to calculate the rating curves, a large number of measurements are noted as being “doubtful” and in the range of flow, less than approximately 500 m<sup>3</sup>/s. Thus the rating curve for larger flows needs to be treated with circumspection; and
- ▶ The more extreme gaugings were measured during the 1955 event, with a maximum measured discharge of less than approximately 2000 m<sup>3</sup>/s. The maximum stage measured was 9.335m. This is significantly less than the extrapolated rating curve which extends to 14.29m. Thus, again, the rating curve for extreme events needs to be treated with circumspection;

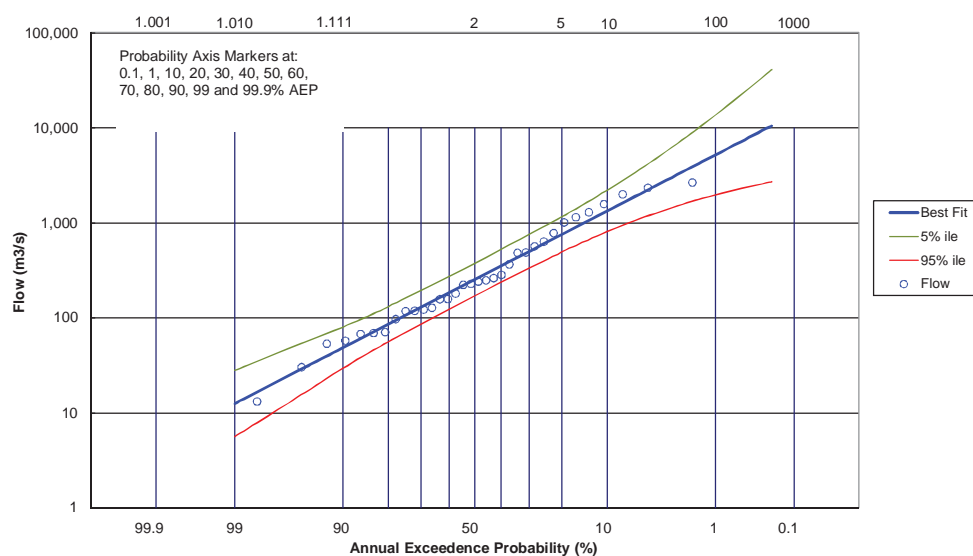
The manual flood frequency was undertaken for two data periods as follows:

- ▶ Pre-construction of Split Rock Dam - Use was made of the daily flow measurements. Although instantaneous measurements were available for part of this period (namely 1974 to 1987), these were not included to ensure the same data type was used; and
- ▶ Pre-construction of Split Rock Dam - Use was made of the instantaneous flow measurements;

The results of the flood frequency analysis are shown in the figures and tables below. Further supporting information is provided in Appendix B. From the results, the 1% AEP event could be expected to be approximately 5100 m<sup>3</sup>/s at the Manilla Railway Bridge before construction of Split Rock Dam. Under current conditions, after construction of Split Rock Dam, the 1% AEP event could be expected to be approximately 3600 m<sup>3</sup>/s at the Manilla Railway Bridge, however this will be heavily dependent of the storage status of the Split Rock Dam at the time of the event.

For the purposes of model calibration, flood volumes were estimated for particular events by using measured gauging of water level and flow at various gauges to develop a stage-flow relationship. From this relationship flow hydrographs at 15-minute increments for the Manilla and Namoi rivers were developed.

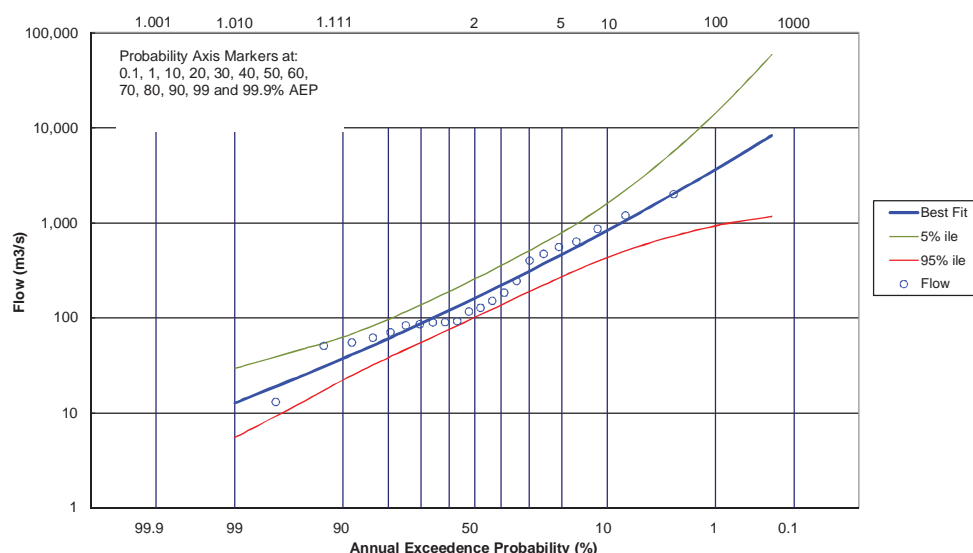
**Figure 3-1 Flood frequency analysis - pre-construction of Split Rock Dam**



**Table 3-2 Flood frequency analysis results**

AEP	Pre-Split Rock Dam Flood Peak ( $m^3/s$ ) (based on daily data)	Post-Split Rock Dam Flood Peak ( $m^3/s$ ) (based on instantaneous data)
20%	752	461
10%	1329	826
2%	3613	2427
1%	5140	3610
0.5 %	7107	5232

**Figure 3-2 Flood Frequency Analysis - Post-Construction Of Split Rock Dam**



### 3.3 Rainfall Data

A number of rainfall gauges (Refer to Appendix B) are located within the Manilla, Namoi and Macdonald river catchments as tabulated below. While the daily rainfall data gauges are numerous, there are only a few pluviograph gauges in the catchment that provide temporal information on historic storms. The daily rainfall gauges are useful to determine any spatial distributions of rainfall that may have occurred during a significant storm event. The pluviographic rainfall stations 54102 (Barraba) and 55136 (Woolbrook) were primarily used for the purposes of calibration, as they:

- ▶ Had the longest period of record, coincidental with the runoff gauge data;
- ▶ Provided 6 minute rainfall data for a number of significant storm events;
- ▶ Are located at appropriate positions along the river channels for the purposes of the study; and
- ▶ Are located within the catchment.

Where possible, calibration was undertaken for events using the Barraba gauge pluviographic rainfall temporal patterns to represent rainfall in sub-catchments draining to the Manilla River and the Woolbrook gauge pluviographic rainfall temporal patterns to represent rainfall in sub-catchments draining to the Namoi and Macdonald rivers. In the event that data for any particular calibration event was missing from either gauge the pattern from the other was applied to all sub-catchments and weighted according to daily rainfall totals in the catchments for the event period.



**Table 3-3 A selection of available Rainfall Data**

Gauge Number	Type	Operational Dates	Comments
Barraba - 54102	Pluviograph	1971 – Present	Calibration With Manilla River
Woolbrook - 55136	Pluviograph	1972 – Present	Calibration With Namoi And Macdonald Rivers
Manilla Post Office - 055031	Daily	1883 – Present	Confirmation of rainfall distribution with Woolbrook and Barraba Gauges
Barraba Post Office - 054003	Daily	1964 – Present	Confirmation of rainfall distribution with Woolbrook and Barraba Gauges
Bendemeer - 55109	Daily	1958 – Present	Confirmation of rainfall distribution with Woolbrook and Barraba Gauges

### 3.4 Calibration Event Data

The flow gaugings in Table 3-1 were interrogated to abstract significant events for calibration purposes, for which concurrent pluviographic rainfall data was available. Events that were considered appropriate for calibration are listed in Table 3-4 below, with concurrent pluviographic rainfall data provided in Appendix B.

**Table 3-4 Calibration Event Data (at Manilla Railway)**

Event	Flood Peak Date	Flood Peak Level (m)	Flow (m <sup>3</sup> /s)
1971	30/01/1971	10.285 m	2565
1984	30/01/1974	8.293 m	1650
1998	07/09/1998	9.219 m	2050
2004	18/01/2004	6.315 m	940
1964*	14/01/1964	14 m	5800

\*approximate data only

### 3.5 Topographic Survey

As part of the study, survey data was compiled as follows:

- ▶ DTM data to describe the topography and floodplains of the Manilla and Namoi Rivers through the town of Manilla;
- ▶ Terrestrial survey of key features such as bridges and areas where the methodology used to produce the DTM were limited; and
- ▶ Survey of floor levels, for the purposes of flood damage assessments.





The survey was undertaken by Aerometrex and Baxter Geo Consulting, using Tamworth Regional Council's orthorectified photography for both towns. A survey QA report is provided in Appendix I. The photography was surveyed at 20cm pixel resolution (GSD) in March 2008 and had the following specifications:

- Flying Height: 7670 ft;
- Horizontal Accuracy (Ortho): +/- 0.40m RMSE;
- Horizontal Accuracy (Feature): +/- 0.20m RMSE;
- Vertical Accuracy of 0.17m (to 68% or 1 Sigma confidence); and
- Vertical accuracy of 0.34m (to 95% or 2 Sigma confidence).

Aerometrex processed the data and:

- Provided coded breaklines in dxf (for input into Civil 3D & 12D);
- Spot heights at an interval of 10m in a digital format which will be suitable for input into Civil 3D & 12D;
- Provided four spot heights adjacent to houses or buildings in the expected flood area approximately 350 per site; and
- Provide a georeferenced image in ECW format.

Thereafter Baxter Geo Consulting:

- Undertook additional survey control under instruction from Aerometrex;
- Provided survey control to Aerometrex;
- Received processed data from Aerometrex and loaded it into Civil 3D for validation;
- Undertook field validation of selected points;
- Located features, such as bridges, weirs and extra points along the river banks; and
- Established floor levels within the PMF when it has been determined by modelling.

### **3.6 Split Rock Dam**

Split Rock Dam was constructed in 1987 to provide irrigation water to the Namoi valley, to protect the river environment and to supply additional water for towns along the Namoi River. Split Rock Dam has a height of 66m and holds a volume of 397.37GL of water at full capacity.

Parameters for configuration of Split Rock Dam as part of the Manilla RORB hydrological model were obtained from NSW State Water. The reservoir was configured with 2 spillways (spillway level and dam crest level), a height-storage relationship and a "weir only" outlet to simulate storage in the reservoir.

It is important to note that Split Rock Dam is currently being upgraded, with a 2m high concrete parapet wall as part of the Dam Safety Upgrade Program, however the capacity of the dam is not intended to be increased with this upgrade. This upgrade is aimed at ensuring compliance with ANCOLD and DSC safety guidelines. Table 3-5 below shows the key dimensions used in the configuration of the Split Rock Dam Reservoir in the RORB hydrological model.

In the environmental assessment for the Keepit Dam Upgrade (State Water excerpt), State Water commissioned the Department of Natural Resources to synthesise the past 100-years of rainfall and river



flow data in the Namoi River catchment using the Integrated Water Quality and Quantity Model (IQQM) software model (DIPNR 2005). This model was used to predict storage levels in the reservoir (CSIRO, 2007). The study indicated that in the 5 largest flood events in the Namoi River, the dam would be at greater than 98% capacity prior to the event occurring. The Keepit Dam Upgrade Environmental Assessment therefore concluded that it would be reasonable to assume that in events greater than the 1% AEP event, the dam would be at full supply capacity. The report also indicated that the average number of years between spills at Split Rock Dam would be 7.1 years.

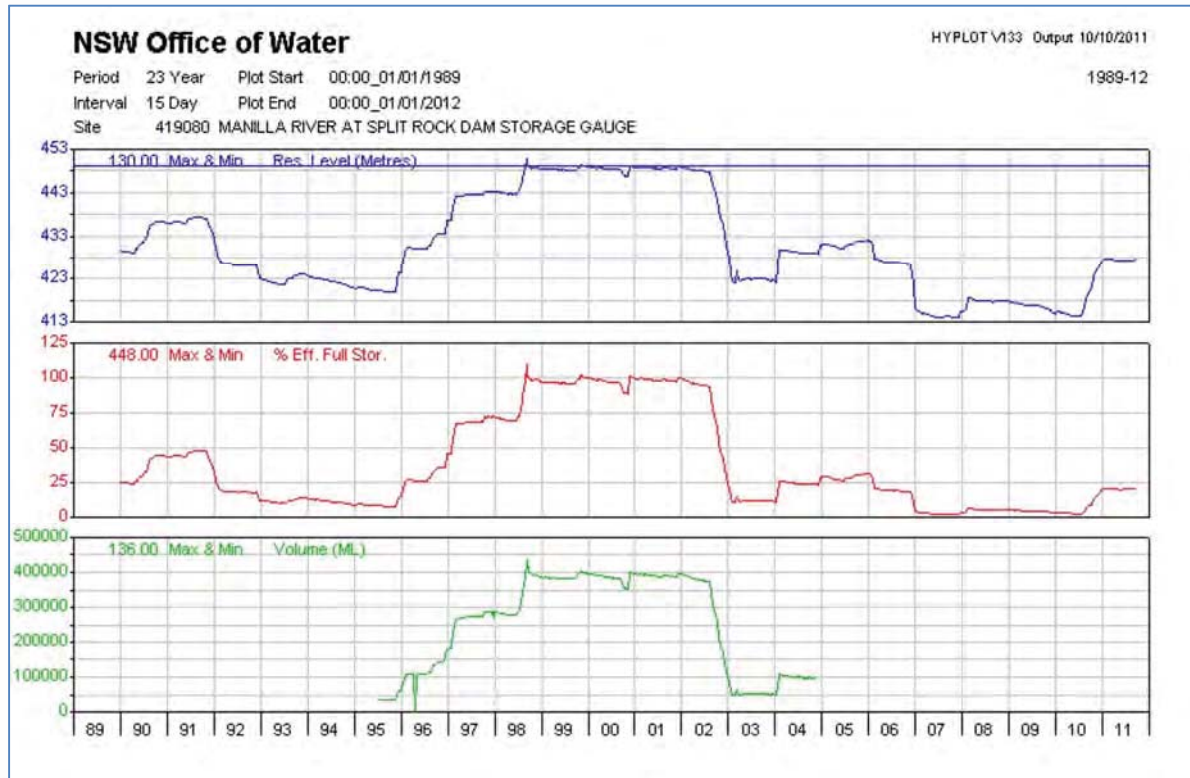
The historical record of dam volume, level and percentage full is shown in Figure 3-3. From this figure, the following is noted:

- ▶ The dam remained below 50% full dropping steadily to 10% full in the first 8 to 9 years of operation;
- ▶ After 1995 the dam filled over a period of some 3 years;
- ▶ Near the first year of being full, the 1998 flood event occurred, which resulted in discharge over the spillway; and
- ▶ Since 2003, the dam sharply dropped in volume and has been operating between empty and 25% full. (More recently, we understand the volume has risen to 40% full).

**Table 3-5 Key Split Rock Dam Dimensions used in Model Configuration**

Parameter	Description
Full Supply Level/Spillway Level	449.0m
Dam Crest Level	460.9m
Spillway Width	96.5m
Crest Length	484m

Figure 3-3 Split Rock Dam Storage Data (www.waterinfo.nsw - real-time data)





## 4. Community Consultation

### 4.1 Overview

The primary objectives of the flood study consultation activities were as follows:

- ▶ Informing the relevant government agencies that the study is being undertaken, outlining its objectives and inviting agencies to provide any relevant data they may hold and / or advise of any particular issues of concern;
- ▶ Similarly informing relevant local community groups; and
- ▶ Similarly informing the general public.

### 4.2 Floodplain Risk Management Committee

The purpose of the Floodplain Risk Management Committee is to:

- ▶ Act as both a focus and forum for the discussion of technical, social, economic, environmental and cultural issues and for the distillation of possibly differing viewpoints on these issues into a management plan; and
- ▶ Ensure that all stakeholders (often with competing desires) are equally represented. As such, the composition and roles of committee members are matters of key importance.

The Floodplain Risk Management Committee does not have any formal powers. Rather, it has an advisory role, but an important one. The principal objective of the committee is to assist the Council in the development and implementation of a management plan for the area(s) under its jurisdiction.

A Floodplain Risk Management Committee was convened by Tamworth Regional Council and included representatives from Council State Government and the committee. The committee met on:

- ▶ 20 July 2011;
- ▶ 3 August 2011;
- ▶ 16 September 2011; and
- ▶ 20 October 2011.

Minutes of the meeting are provided in Appendix C.

### 4.3 Consultation Activities

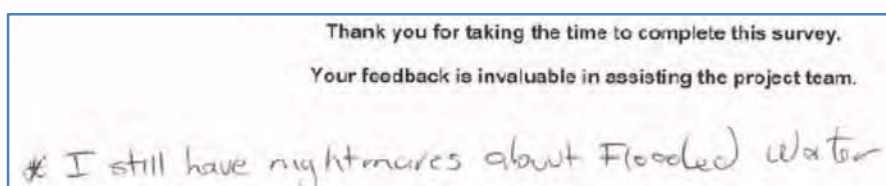
#### 4.3.1 2011.08 - Project Notification, Newsletter and Survey

A public notice was placed in the local Manilla Express newspapers in August 2011. In addition a project information sheet and survey was forwarded to the residents in Manilla (Appendix C). A total of 97 survey responses were received. Key issues raised in the survey were:

- ▶ A large number of the residents had experienced flooding in the town of Manilla, first hand;
- ▶ Flood levels tend to rise and recede very rapidly, often within the space of a day;

- ▶ Preparation for flooding includes regular observation of river levels and lifting of belongings. In addition, listening to advice on local radio stations and from the SES. Stock, vehicles and other equipment is moved to higher ground;
- ▶ Isolation of residents is common;
- ▶ Many residents often mentioned the 1964 floods and these have been etched in memories of the Manilla community. One resident noted 8 m of flood waters through the house while another had their house completely submerged;
- ▶ A number of residents noted damages associated with flooding to dwellings;
- ▶ Residents noted that flooding had been reduced by Split Rock Dam, however could still be a high risk, if Split Rock Dam was full as occurred in the 1998 flood;
- ▶ For the school, a key response was to get school children home safely;
- ▶ A number of residents noted stormwater problems around town, after heavy localised rainfall; and
- ▶ Residents noted issues with regards to fallen trees and debris blocking flood waters and the river channel. One resident noted an observation that water holes had silted up post construction of Split Rock Dam.

In additions a number of surveys provided flood markers around town.



#### **4.3.2 2011.10.20 - Meeting with the Flood Plain Management Committee**

A meeting was held with the Floodplain Management Committee at the offices of Tamworth Regional Council. The meeting discussed the approach of the study and the program. A number of issues pertaining to the survey were discussed. In attendance were Council officers, OHE and SES. Minutes of the meeting can be found in Appendix C, together with a transcript of the presentation.

#### **4.3.3 2011.12.14 - Meeting with the Manilla Community**

A meeting was held at the Manilla Town Hall with the Manilla Community and the Floodplain Management Committee. A presentation of the overall project was provided and the community had the opportunity to provide input to the study. In particular the purpose of the flood study in the context of the overall Floodplain Risk Management Process was highlighted. The community provided valuable input with regards to experiences during flood events. In particular a number of the community had experienced the 1964 flood event.

At the meeting, preliminary flood maps were presented to the community to identify issues relating to their experiences. In attendance were Council officers, OHE and SES. The attendance can be found in Appendix C, together with a transcript of the presentation.



## 5. Hydrologic Model Configuration and Calibration (RORB)

### 5.1 General

The hydrology for the Manilla flood study was developed using the RORB hydrological model. The model was setup as an end of catchment model, producing flood hydrographs for the Namoi and Manilla Rivers upstream of the Manilla town.

*RORB is a general runoff and stream flow routing program used to calculate flood hydrographs from rainfall and other channel inputs. It subtracts losses from rainfall to produce rainfall-excess and routes this through catchment storage to produce runoff hydrographs at any location. It can also be used to design retarding basins and to route floods through channel networks.*

*The program requires a data file to describe the particular features of the stream network being modelled and is run interactively. It can be used both for the calculation of design hydrographs and for model calibration by fitting to rainfall and runoff data of recorded events.*

*The model is a spatially distributed, nonlinear, and applicable to both urban and rural catchments. It makes provision for temporal and areal variation of rainfall and losses and can model flows at any number of gauging stations. In addition to normal channel storage, specific modelling can be provided for retarding basins, storage reservoirs, lakes or large flood plain storages. Base flow and other channel inflow and outflow processes, both concentrated and distributed, can be modelled. (RORB 6 User Manual).*

### 5.2 Configuration

Compilation of the RORB model included:

- ▶ Catchment delineation, in accordance with the RORB procedures. For the Manilla model a total of 21 subcatchments were delineated;
- ▶ Catchment parameter determination, namely subcatchment area and reach lengths and slopes;
- ▶ Event rainfall and concurrent flow data compilation, for calibration; and
- ▶ Design rainfall determination for generating design storm rainfall events, for the 0.5%, 1%, 2%, 10% and 20% AEP events together with the Probable Maximum Flood (PMF).

The RORB model was simulated for a range of durations ranging up to 72 hours. For each event the critical duration was reported. Lag times were based on average slopes and flow velocities, ranging between 1 m/s and 2 m/s depending on slope. Percentage of impervious areas, used in the hydrology model, was 5% to represent the rural nature of the catchment. Catchment maps and sub-catchment delineation are provided in Appendix D.

### 5.3 Calibration

#### 5.3.1 General

The RORB model was calibrated by variation of model parameters to obtain a good fit of the calculated to the measured hydrograph. The parameter  $k_c$  is the main means of achieving a fit. The parameter,  $k_c$ ,

can be decreased to increase the hydrograph peaks and decrease the lag time. Conversely, increasing  $k_c$  does the opposite. In addition to  $k_c$ , varying the initial loss is also an important means of achieving a fit. A further means is by altering the  $m$  value however use of this parameter for calibration is less common.

### 5.3.2 Regional $k_c$ Parameter

A number of regional estimates for the determination of  $k_c$  are available throughout the literature and in the Australian Rainfall and Runoff (AR&R 2001). A number of these are offered within the RORB model for use in calibration. For the Manilla RORB model, possible regional estimates of  $k_c$  parameters are tabulated below.

**Table 5-1 Manilla RORB model Regional  $k_c$  Parameter Estimates**

Method	$k_c$ Estimate
Eastern NSW (Kleemola) (Eqn 3.20, ARR (Book V))	62.18
Australia Wide – Dyer (1994) data (Pearse et al, 2002)	93.73
Australia Wide – Yu (1989) data (Pearse et al, 2002)	78.93
RORB Default – Eqn 2.4 (RORB Manual)	157.81

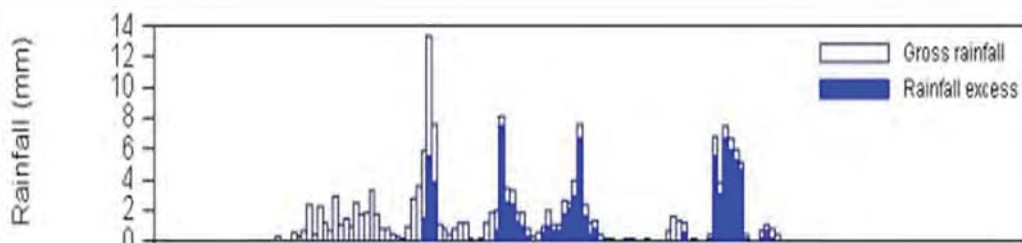
### 5.3.3 Calibration – 1971 Storm (30/01/1971)

This event gives an indication of flooding magnitude prior to construction of Split Rock Dam. The event started with rainfall on 28/01/1971 which lasted for 5 days. The peak rainfall intensity recorded at the Barraba pluviographic rainfall station 54102 was 25.5 mm/hr in 6 minutes (at 12pm on the 29<sup>th</sup> January 1971). During this event this station recorded a total rainfall of 166.4mm. During the same period the Woolbrook pluviographic gauge recorded 92.9mm of rain. Preceding the storm, the antecedent conditions within the catchment were dry. Figure 5-1 below shows the rainfall recorded at the Woolbrook Rainfall station. The best fit calibration achieved for this event was using the regional Eastern NSW (Kleemola)  $k_c$  of 62.18, and an  $m$  value of 0.8, as shown in Figure 5-2. Initial and continuing loss parameters at the Manilla gauge were 45mm and 4 mm/hr respectively, with lower losses applied to subsequent rainfall bursts recorded during this event. The losses used in this calibration are within acceptable range according to the *RORB Version 6 User Manual*. Table 5-2, provides key calibration statistics. From the figure and the table, the following is noted:

- Reasonable approximation of the 3 rainfall burst, in the flood record;
- Flood peak approximation is considered reasonable; and
- Reasonable approximation of the flood volume.



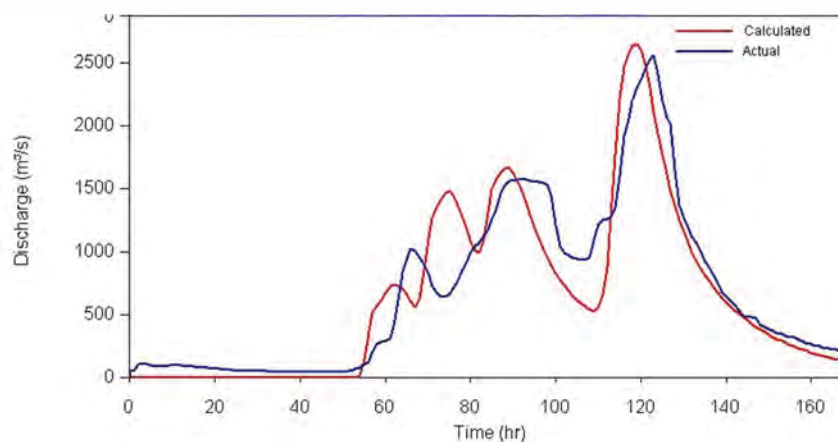
**Figure 5-1 Woolbrook Rainfall Pattern**



**Table 5-2 1971 Storm Calibration Statistics**

Item	Observed	Simulated	Difference
Storm Peak	2563	2654	+3.6%
Storm Volume	0.41E+09	0.39E+09	-5.3%
Lag (time to peak)	123	119	-3.3%

**Figure 5-2 1971 Rainfall Event and Calibrated hydrograph at Manilla Railway Bridge (Gauge 419022)**



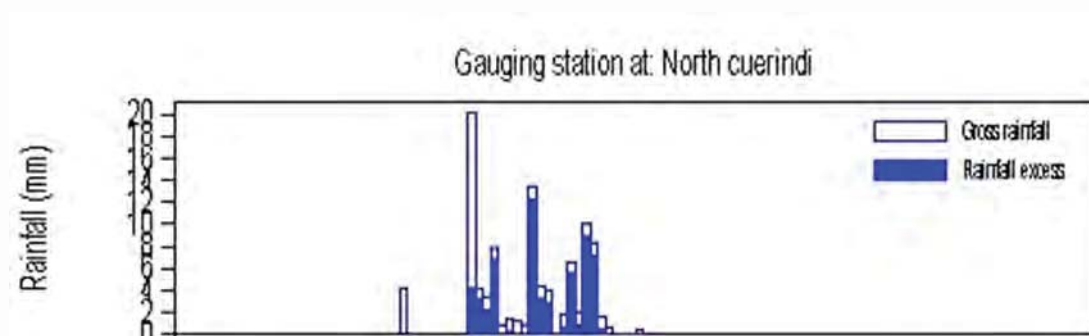
### 5.3.4 Calibration – 1998 Storm (03/09/1998)

This event gives an indication of flooding magnitude after construction of Split Rock Dam, and the measured storage level in the dam at the time of the event was configured in the model. The 1998 event started with rainfall on 03/09/1998 which lasted for 3 days. The peak rainfall intensity recorded at the Barraba pluviographic rainfall station 54102, was 47.7 mm/hr in 6 minutes recorded at 12am on the 4<sup>th</sup> September 1998. During this period, 98mm was recorded at Barraba. During this same Period a total rainfall of 111.2mm was recorded at the Woolbrook Gauge. No pluvio data for the Woolbrook gauge was available for this event and therefore the Barraba rainfall pattern was applied to all catchments. Preceding the storm, the antecedent conditions within the catchment were dry.



Figure 5-3 below shows the rainfall recorded at the Barraba station. The best fit calibration achieved for this event was again using the regional Eastern NSW (Kleemola) kc of 62.18, and an m value of 0.8.

**Figure 5-3 Barraba Rainfall Pattern**



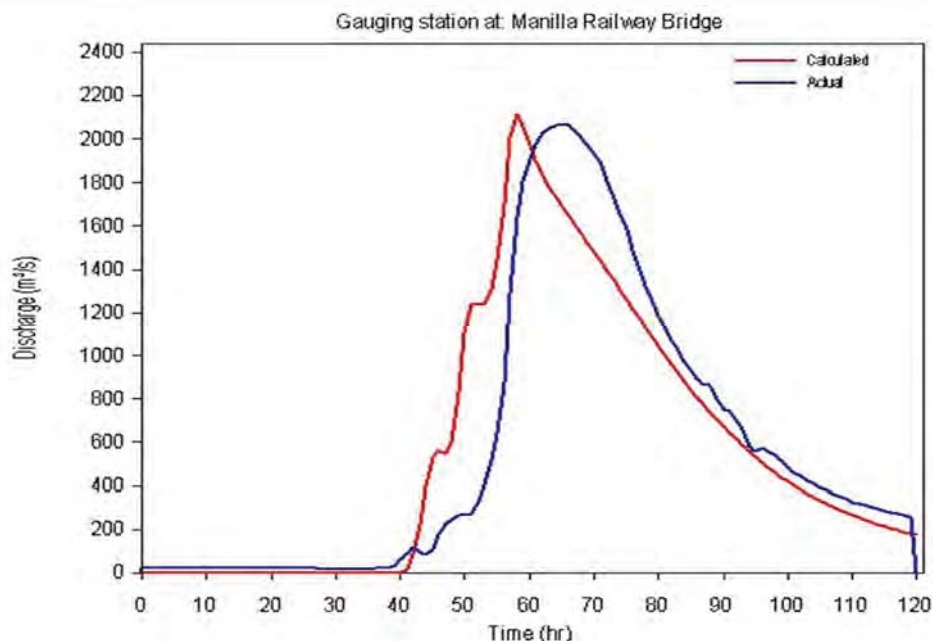
This event was simulated with Split Rock Dam at full capacity, as was the case during the event. Initial and continuing loss parameters at the Barraba gauge were 20mm and 1.9 mm/hr respectively and 25mm and 1.2 mm/hr for Woolbrook, which are within acceptable range according to the *RORB Version 6 User Manual*. Table 5-3 provides key calibration statistics. From the figure and the table, the following is noted:

- Reasonable approximation of the flood hydrograph shape;
- Flood peak approximation is considered good with an estimate of the peak to less than 1%;
- Good approximation of the flood volume; and
- The calculated peak arrives earlier than the observed peak, however this could be related to the impacts of Split Rock, and the likely impact on timing of the two peaks.

**Table 5-3 1998 Storm Calibration Statistics**

Item	Observed	Simulated	Difference
Storm Peak	2065	2115	+2.4%
Storm Volume	0.25E+09	0.25E+09	-0.1%
Lag (time to peak)	65	58	-10.8%
Peak Dam Water Level	450.77	451.02	+0.6%

**Figure 5-4 1998 Rainfall Event and Calibrated hydrograph at Manilla Railway Bridge (Gauge 419022)**



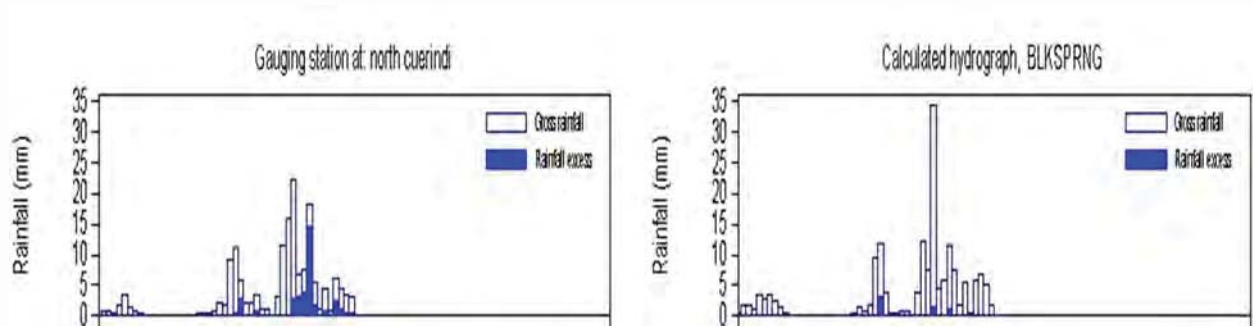
### 5.3.5 Calibration – 2004 Storm (13/01/2004)

This event also includes the effects of Split Rock Dam and the measured storage level in the dam at the time of the event was configured in the model. The 2004 event started with rainfall on 13/01/2004 which lasted for 5 days. The peak rainfall intensity recorded by the Barraba gauge (54102) was 71.4 mm/hr, which was recorded during a 6-minute time increment on the 16<sup>th</sup> January 2004 at 9 pm. During this period a total rainfall of 165mm was recorded by the Barraba Gauge. For this same event, the Woolbrook gauge (55136) recorded 134 mm over the same period and recorded a peak rainfall intensity of 65.9 mm/hr during a 6-minute time increment on the 16<sup>th</sup> January 2004 at 8 pm. An interrogation of daily rainfall gauges throughout the catchment showed similar daily rainfall totals throughout the catchments.

Preceding the storm, the antecedent conditions within the catchment were rather wet, having received 40 mm of rainfall in the preceding 5 days. Figure 5-5 below shows the rainfall recorded at the Woolbrook and Barraba pluvio stations. The best fit calibration achieved for this event was again using the regional Eastern NSW (Kleemola) kc of 62.18, and an m value of 0.8, as shown in Figure 5-6. Initial and continuing loss parameters at the Manilla gauge were 40mm and 4 mm/hr respectively, which are within acceptable range according to the *RORB Version 6 User Manual*. Table 5-4 provides key calibration statistics. From the figure and the table, the following is noted:

- ▶ Reasonable approximation of the flood hydrograph shape, however with less favourable approximation of catchment lag;
- ▶ Flood peak approximation is considered less favourable, however the simulated peak is slight conservative compared to the observed peak; and
- ▶ Reasonable approximation of the flood volume.

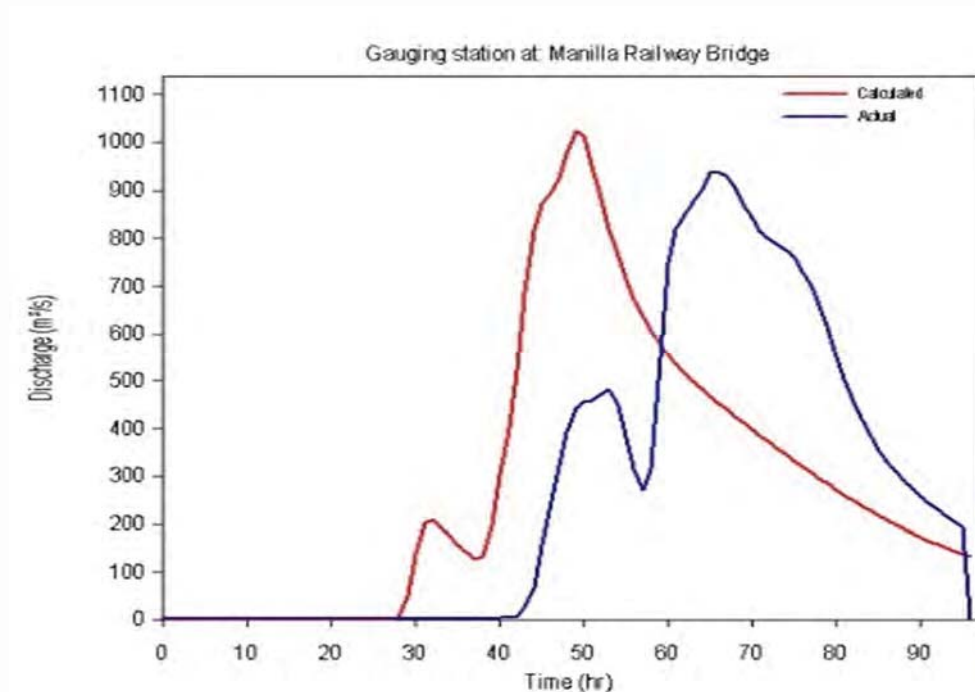
**Figure 5-5 Rainfall Patterns for Woolbrook and Barraba Gauges**



**Table 5-4 2004 Storm Calibration Statistics**

Item	Observed	Simulated	Difference
Storm Peak	939	1025	+9.2%
Storm Volume	0.99E+08	0.10E+09	+3.3%
Lag ( time to peak)	66	49	-25.8%

**Figure 5-6 2004 Rainfall Event - Calibrated hydrograph at Manilla Railway Bridge (Gauge 419022)**



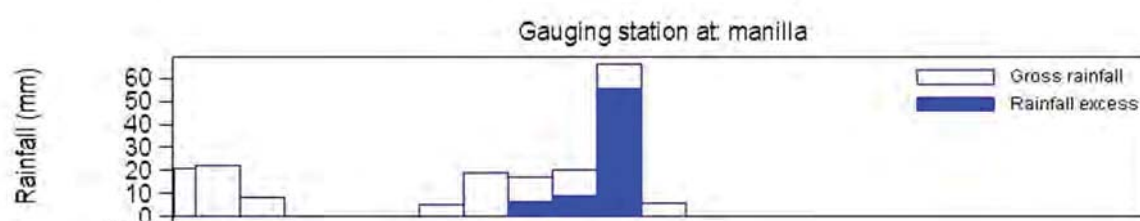
### 5.3.6 Calibration – 1964 Storm (14/01/1964)

This event is the largest event on record for Manilla. As data records for this event were not available from existing runoff gauges, the calibration was undertaken using data obtained from the Namoi Valley Flood Investigation Trust, Namoi River Flood Investigation – Report 1 (NVFIT 1964) and the Flood Information Reports, Floods in the Namoi Valley (1955, 1962, 1964, 1971, 1974, 1976), NSW Water Resources Commission (WRC). This event started with rainfall on the 13/01/1964 which lasted for 3 days. Rainfall information was available in 3-hourly intervals with a peak rainfall depth recorded at the Barraba rainfall gauge (54102) of approximately 66 mm in a 3-hour interval. During this event a total of 184mm was recorded by the Barraba gauge. Figure 5-5 below shows the rainfall recorded at the Barraba pluviographic station. The best fit calibration achieved for this event was again using the regional Eastern NSW (Kleemola) kc of 62.18, and an m value of 0.8, as shown in Figure 5-6. Initial and continuing loss parameters at the Manilla gauge were 35mm and 4 mm/hr respectively, which are within acceptable range according to the *RORB Version 6 User Manual*. Table 5-4 provides key calibration statistics. From the figure and the table, the following is noted:

- ▶ Reasonable approximation of the flood hydrograph shape, however with less favourable approximation of catchment lag;
- ▶ Flood peak approximation is within 2.2%, and the simulated peak is slight conservative compared to the observed peak; and
- ▶ Good approximation of the flood volume to within 4.5%.

It will be shown in later sections that this event compares favourably with the 1% AEP design event for the pre-Split Rock Dam scenario.

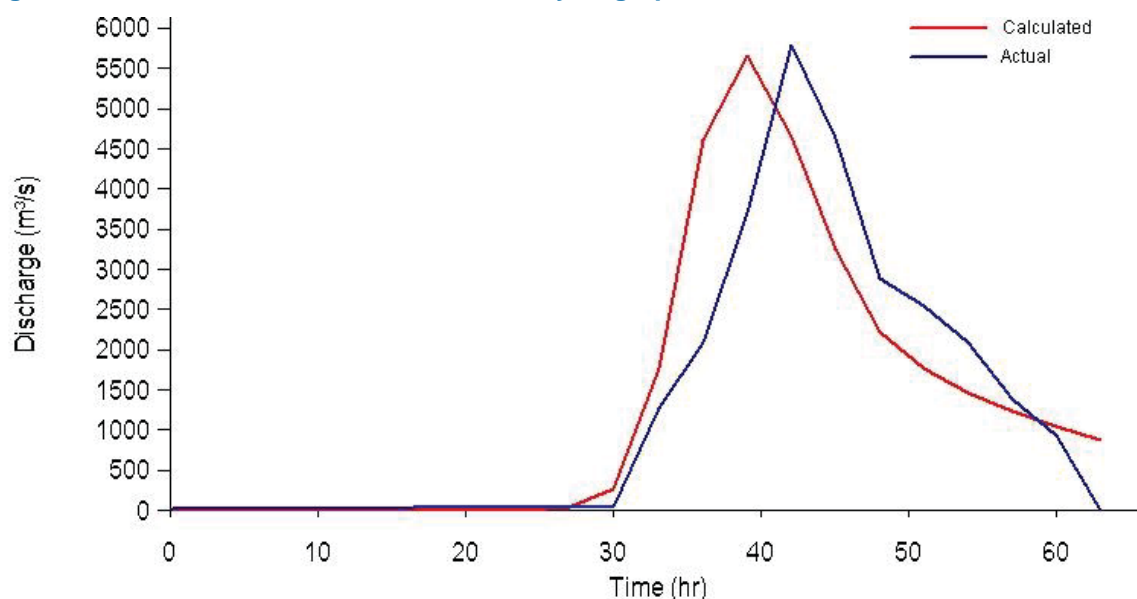
**Figure 5-7 Rainfall Pattern for Barraba Gauge**



**Table 5-5 1964 Storm Calibration Statistics**

Item	Observed	Simulated	Difference
Storm Peak	5787	5658	-2.2%
Storm Volume	2.98E+08	3.11E+08	+4.5%
Lag ( time to peak)	42	39	-7.1%

**Figure 5-8 1964 Rainfall Event - Calibrated hydrograph at Manilla**



### 5.3.7 Summary

The RORB manual (RORB Manual, Section 7.3) stresses that users need to be realistic in expectations of accuracy for calibrations, indicates that accuracies in the order of  $\pm 15\%$  could be expected in the underlying flow data used for calibrations.

In general the RORB calibrations achieved a difference in flood peak and flood volume of approximately  $\pm 5\%$ , using the regional (Kleemola)  $k_c$  value of 62.18. Initial and continuing loss rates determined for each calibration are also noted to be very similar and consistent.

Given the matters raised in Section 3.2 relating to gauging and rating curve accuracies, it was considered that the calibrations achieved were acceptable and further optimisation of the calibration was not warranted. On this basis, it was decided to accept the regional (Kleemola) RORB  $k_c$  parameter of 62.18.



## 6. Hydraulic Model Configuration and Calibration (TUFLOW)

### 6.1 General

The flood conveyance through Manilla was calculated using the TUFLOW hydraulic model.

*TUFLOW is a computer program for simulating depth-averaged, two and one-dimensional free-surface flows such as occurs from floods and tides. TUFLOW was originally developed for modelling two-dimensional (2D) flows, and stands for Two-dimensional Unsteady FLOW. However, it incorporates the full functionality of the ESTRY 1D network or quasi-2D modelling system based on the full one-dimensional (1D) free-surface St Venant flow equations (see below). The 2D solution algorithm is based on Stelling 1984, and is documented in Syme 1991. It solves the full two-dimensional, depth averaged, momentum and continuity equations for free-surface flow. The scheme includes the viscosity or sub-grid-scale turbulence term that other mainstream software omit. The initial development was carried out as a joint research and development project between WBM Oceanics Australia and The University of Queensland in 1990. The project successfully developed a 2D/1D dynamically linked modelling system (Syme 1991). Latter improvements from 1998 to today focus on hydraulic structures, flood modelling, advanced 2D/1D linking and using GIS for data management (Syme 2001a, Syme 2001b). TUFLOW has also been the subject of extensive testing and validation by WBM Pty Ltd and others (Barton 2001, Huxley, 2004).*

*TUFLOW is specifically orientated towards establishing flow and inundation patterns in coastal waters, estuaries, rivers, floodplains and urban areas where the flow behaviour is essentially 2D in nature and cannot or would be awkward to represent using a 1D model. A powerful feature of TUFLOW is its ability to dynamically link to 1D networks using the hydrodynamic solutions of ESTRY, ISIS and XP-SWMM. The user sets up a model as a combination of 1D network domains linked to 2D domains, i.e. the 2D and 1D domains are linked to form one overall model. (BMT WBM 2010)*

### 6.2 Configuration

The model extent for the purposes of flood mapping was defined in collaboration with Tamworth Regional Council. The final model extent was adjusted slightly to provide model stability and negate the effects of boundary conditions, as shown in Appendix E. The TUFLOW model compilation configured the key parameters as described in Table 6.1 using the following methodology:

- ▶ DTM data for the local area was imported into a digital terrain-modelling program (12D) and triangulated to represent the ground surface;
- ▶ A TUFLOW grid was generated with a cell size of 5 m<sup>2</sup>. Each point in the grid was given an elevation based on its location in the DTM. The grid size was chosen because this is a compromise between the accuracy of the DTM data, simulation run time, model stability, and the accuracy of the results;
- ▶ All bridges within the floodplain were configured using the terrestrial survey data. These were configured within the 2D model grid;
- ▶ The flood hydrographs output by the RORB model were configured as inflows, distributed over the upstream floodplain cross-sections for both the Namoi and Manilla Rivers. Downstream boundary conditions were configured as a flow stage relationship, as shown in Figure 6-1; and



- Based on aerial photography and site inspections, hydraulic roughness coefficients for the floodplain were digitised for the floodplain and input to the model. These coefficients were digitised as a range of surfaces. Table 6-1 lists the roughness categories used in this model.

**Table 6-1 TUFLOW Modelling Parameters**

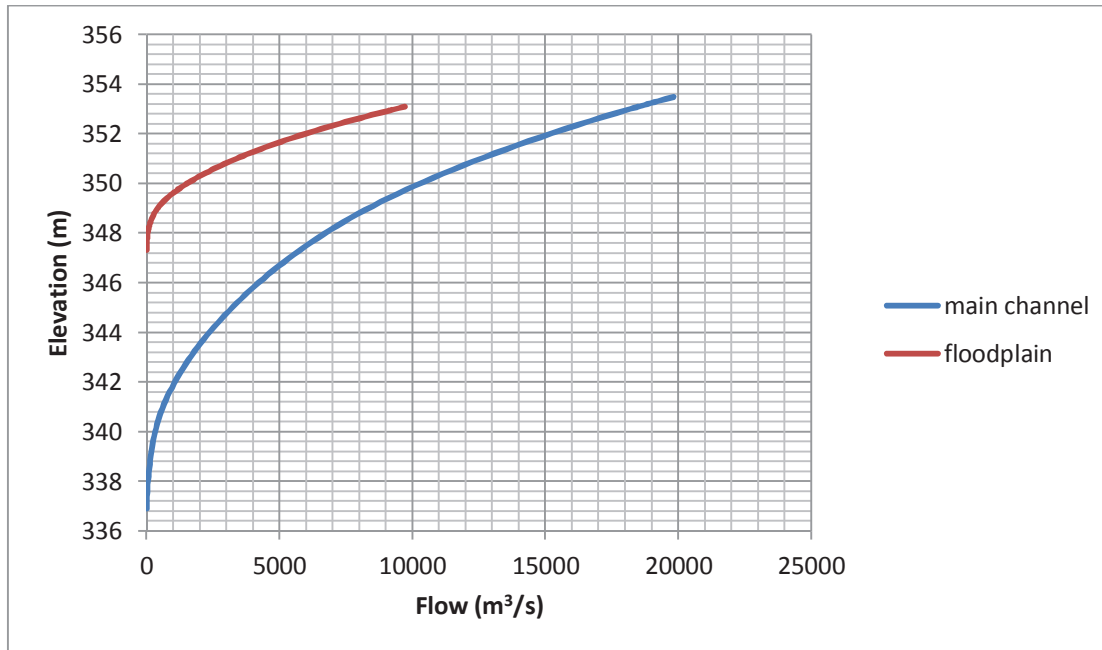
Feature	Value
Time step	1 second
Grid size	5m x 5m
Manning's "n" – roads	0.02
Manning's "n" – light to medium vegetation, with some trees	0.06
Manning's "n" – medium vegetation, thicker trees and some bush	0.10
Manning's "n" – dense vegetation, mostly dense bush	0.12
Manning's "n" – creeks and natural channels	0.03
Manning's "n" – developed areas (residential, commercial, industrial, farm sheds)	0.5
Manning's "n" – houses or blocked out with storage areas (zero conveyance)	2

### 6.2.1 Downstream Boundary Conditions

Downstream boundary conditions were configured in the TUFLOW model as a stage discharge relationship. The floodplain downstream of Manilla town flattens out considerably, with flood channels in the natural topography on the right bank of the river. This resulted in instabilities in the flood model during larger events. To this end the downstream boundary was adjusted to a position where the topography provided a natural constriction. Figure 6-1 shows the adopted elevation – discharge relationship used for the downstream boundary condition.



**Figure 6-1 Elevation-Discharge Relationship for Downstream Boundary Condition**



### 6.2.2 Significant Structures in the Floodplain

Referring to Appendix E, the following significant structures located in the Manilla floodplain were configured in the TUFLOW model.

- **Mandowa Street Bridge - Bridge 1:** This 85m concrete span structure is located to the north west of town upstream of the Manilla confluence on the Manilla River. It consists of a 2-lane concrete deck span and 5 concrete pylons with headstocks. This results in a waterway opening of 15.5m and a diagonal waterway opening of 18.4. The road overflow level is 350.85m AHD. Approach roads to the structure are lower than the bridge deck and overflow would be initiated before inundation of the deck;
- **Manilla Street Bridge - Bridge 2:** This 300m span structure is located to the northeast of town. It consists of 1 trussed span and 20 circular steel piers. This results in a waterway opening of 38.6m and a diagonal waterway opening of 39.8m. The road overflow level is 357.6 m AHD. Approach roads to the structure are level with the bridge deck and overflow would not be initiated before inundation of the deck;
- **Manilla Railway Bridge – Bridge 3:** This 520m spanning structure is located south of town. It consists of 2 approach spans on wooden piers over the floodplain and a central trussed span on concrete pylons spanning the Manilla River. This result in a waterway opening of 75m and a diagonal waterway opening of 76m. The deck overflow level is 357.3 m AHD. Approach roads to the structure are higher than the bridge deck and overflow would be initiated before inundation of the deck;
- **Higgin's Avenue Bridge - Bridge 4:** This 63m span structure is located south of town. It consists of a single 2 lane concrete deck span on 6 low set square concrete piers with headstocks. This results in a waterway opening of 8.5m and a diagonal waterway opening of 8.7m. The road overflow level is





341m AHD. Approach roads to the structure are higher than the bridge deck and overflow would not be initiated before inundation of the deck;

- ▶ Manilla Weir – Manilla weir was constructed in 1939 and is located 2.2km upstream of the Manilla confluence on the Namoi River. It consists of a 42.8m wide weir with a stepped spillway on the northern side. The upstream side of the weir is at an elevation of 346.2mAHD and drops to 344.15mAHD on the downstream side; and
- ▶ Rushes Creek Road Bridge – Bridge 5: this 29.5m spanning bridge crosses rushes creek to the south-east of town. It consists of a single 2-lane span with a road overflow level of 351.2mAHD. The bridge has been recently renewed and a low level crossing upstream has been removed with the natural creek channel reinstated.

### 6.3 Validation against Flood Levels at Manilla Railway Bridge

The 1971 flood event was simulated in TUFLOW in order to calibrate the model. The simulated flood levels were reviewed and model parameters adjusted in order to replicate the recorded peak water level at the Manilla Railway bridge during this event.

The table below shows favourable agreement of the simulated flood peak compared to the measured peak during the 1971 event

**Table 6-2 Comparison of Flood Levels at the Manilla Railway Gauge during the 1971 Event**

Event	Flood Peak Measured at the Gauge	Flood Peak simulated in the Hydraulic Model
1971 Storm (30/01/1971)	348.89 m AHD	349.12 m AHD

### 6.4 Validation against observed Flood Markers

A number of flood level markers, recorded during the 1964 event, exist in and around Manilla. These flood markers have been surveyed and tabulated below. While the flood markers were surveyed as part of the current study they are considered to be of low reliability, having been generally noted by residents (with exception of the Plaque). Since these flood markers were associated with the 1964 flood, and this flood closely represented a 1% AEP event (see Section 7.2.6), the flood markers have been overlaid on the 1% AEP design event in Appendix F, Figure 5.4, assuming there was no attenuation of Manilla flood hydrographs by Split Rock Dam. This scenario was achieved by re-simulating the RORB and TUFLOW models, and removing Split Rock Dam from the models. Referring to the figure and comparison, the following is noted:

- ▶ In the area of River Street, the simulated 1% AEP flood without Split Rock Dam compares favourably with observed flood levels; and
- ▶ In North Manilla, good agreement is achieved for the properties near Lloyd Street. The property adjacent to River Street, noted a significantly higher flood level, however this seems inconsistent with the two other flood level measurements close by.

The 1964 and 1% AEP design event, assuming there was no attenuation of Manilla flood hydrographs by Split Rock Dam, thus compare favourably with the measured flood markers.



**Table 6-3 Identified Flood Markers**

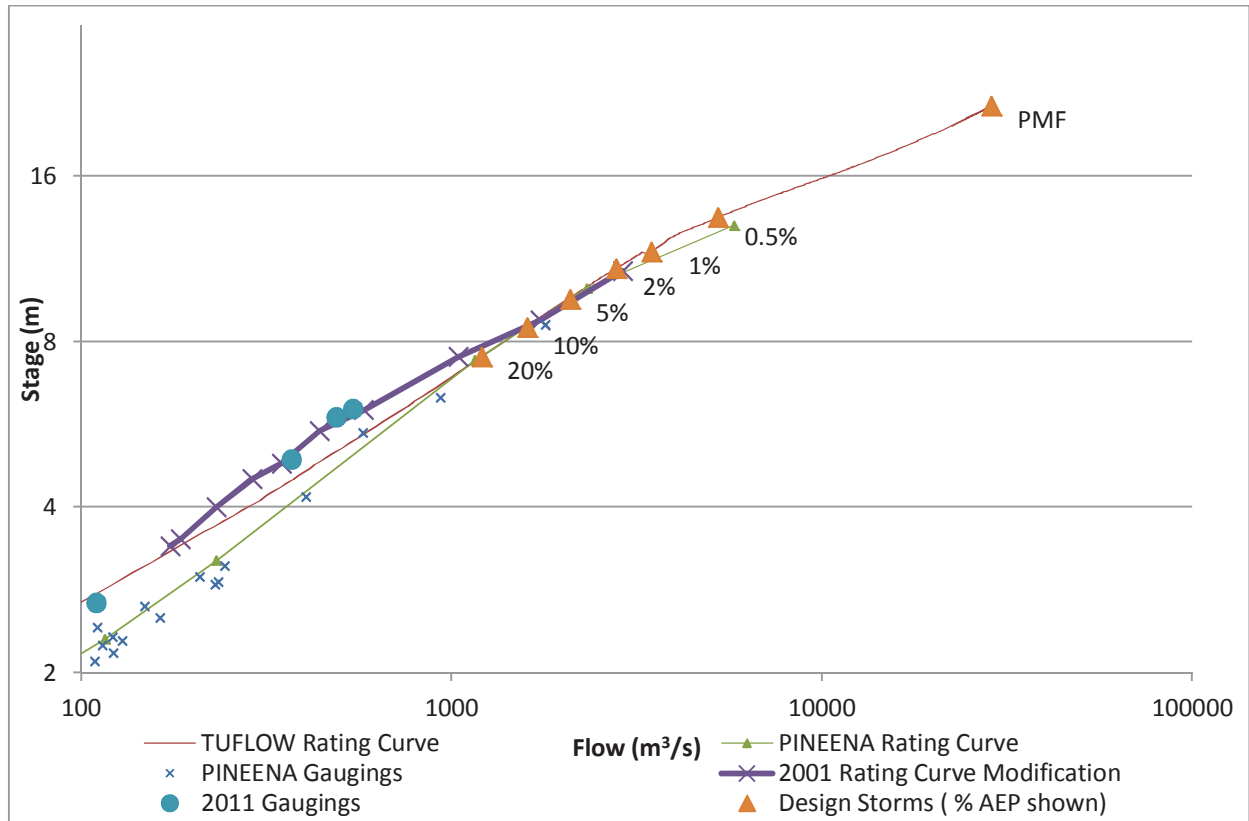
ID	Location/Type	Surveyed Level (m AHD)	Comments
FLOOD MARK	Stain On Wall	354.3	Dewhurst "8m mark"
FLOOR	113 Namoi	354.2	
RIDGE	Charles St	356.1	maybe gutter
RIDGE	Charles St	356.1	
FLOOD	Plaque	353.4	1158ft = 352.96m
FLOOD	85 River St	353.5	top of window
FLOOD	Motel Driveway	351.9	passerby

## 6.5 Comparison and extension of the Manilla Railway Bridge Gauge Rating Curve

The stage-discharge relationship simulated for the design events using the calibrated TUFLOW model at the Manilla Railway Bridge gauge was plotted against the PINEENA rating curve, in Figure 6-2. It is noteworthy that OEH updated the PINEENA rating curve with gauging data captured in 2011.

Although minor deviation occurs for events more frequent than required by the current study, the TUFLOW rating curve is located approximately half way between the OEH PINEENA and 2011 rating curves. For the design events simulated as part of the current study, a good correlation between the TUFLOW and PINEENA curves is observed.

**Figure 6-2 Comparison of PINEENA Rating Curve with TUFLOW Result**





## 7. Design Flood Behaviour

### 7.1 Overview

To determine the design flood behaviour, both the RORB and TUFLOW models were simulated, using the parameters derived through the calibrations together with design rainfall in accordance with the Australian Rainfall and Runoff (AR&R 2001). The simulations were undertaken as follows:

- ▶ The RORB model was simulated using a kc value 62.18 together with design rainfall and rainfall loss estimates in accordance with the Australian Rainfall and Runoff. Each event was simulated for a range of durations and the critical duration storm hydrograph was input to the model; and
- ▶ The results from the RORB model were used as boundary conditions for the TUFLOW model which was simulated for each event.

Further details on the input used for the simulations are provided below.

### 7.2 Flood Hydrographs

#### 7.2.1 Design Rainfall

Design rainfall events were derived in accordance with the procedures of the Australian Rainfall and Runoff, Region 2 (AR&R 2001). The Intensity Frequency Duration parameters adopted for the Manilla catchment are listed in Table 7-1.

**Table 7-1 Manilla IFD Parameters**

Parameter	Value
2yr 1hr	27.33
2yr 12hr	4.65
2yr 72 hr	1.35
50yr 1hr	51.04
50yr 12hr	8.71
50yr 72 hr	2.31
Skew	0.32
F2 Value	4.34
F50 Value	16.04
Zone	E

#### 7.2.2 Probable Maximum Precipitation and Flood (PMP & PMF)

Given the size of catchment and recommended thresholds, the Probable Maximum Precipitation was compiled using the Bureau of Meteorology Australia Generalised Tropical Storm Method – Revised



Version (GTSMR – BOM 2003). The PMP rainfall depths derived for a range of durations using the method are tabulated below.

**Table 7-2 PMP Rainfall Depths**

Duration (hrs)	PMP Rainfall Depth (mm)
24	585
36	682
48	772
72	931
96	1063
120	1120

The PMP rainfall depths were simulated in the RORB model assuming Split Rock Dam was at full supply level to calculate the PMF. Loss factors as discussed in Section 7.2.3 were applied. The PMF flood peak at the Manilla Railway Bridge was calculated to be 28700 m<sup>3</sup>/s, with a critical duration of 36 hours.

### 7.2.3 Rainfall Losses

Rainfall losses were adopted in accordance with the Australian Rainfall and Runoff (AR&R 2001) Book 2 and Book 6. These recommend the losses as listed in Table 7-3.

**Table 7-3 Rainfall Losses**

Event	Initial Loss	Continuing Loss
Up to and including the 1% AEP event	15 mm	2.5 mm/hr
1% event up to the PMF	15 mm	1 mm/hr
PMF	15 mm	1 mm/hr

### 7.2.4 Design Flood Peaks at Manilla

The simulation of the RORB model was undertaken for a number of events and a number of durations, up to and including the PMF. For each event the critical duration design flood hydrograph was identified, and input as upstream boundary condition inflow. The flood peaks determined for each event are summarised below.

It is important to note that these inflows inherently contain assumptions on Split Rock Dam, which was configured to be at full supply level at the time of the event.

A further simulation was undertaken for the 1% AEP event, where the Split Rock Dam was not considered (namely removed from the model), for the purposes of comparisons to the Probabilistic Rational Method and the Flood Frequency Analysis. The flood peak at the Manilla railway bridge was calculated to be 5900 m<sup>3</sup>/s.



**Table 7-4 RORB Design Flood Peaks (with Split Rock Dam)**

Flood event AEP	Manilla River Flood Peak m <sup>3</sup> /s	Namoi River Flood Peak m <sup>3</sup> /s	At the Manilla Railway Bridge m <sup>3</sup> /s
20%	360	970	1205
10%	500	1280	1600
5%	680	1660	2090
2%	960	2200	2780
1%	1225	2710	3470
0.5%	2020	3305	5244
PMF	11385	17410	28710

### 7.2.5 Probabilistic Rational Method

The Probabilistic Rational Method was used to provide an additional estimate of the flood peak for the 1% AEP event. This method is not suitable for catchment sizes of area greater than 250 km<sup>2</sup> and inherently does not necessarily account for catchment effects such as attenuation. However the method gives an indication of the flood peak order of magnitude.

Using the Probabilistic Rational Method, the 1% AEP flood peak was estimated as 5300 m<sup>3</sup>/s. In considering this estimate, it must be noted, that no influences of Split Rock Dam have been considered.

### 7.2.6 Comparison of 1% AEP Flood Peak Estimates

The derived flood peak estimates for a range of events using a number of methods have been tabulated below. Referring to the table below, it is noted that the various methods compare favourably, providing further confidence in the RORB model calibration. In addition the 1% design flood agrees favourably with the 1964 flood peak (approximately 5700m<sup>3</sup>/s) at the Manilla Railway bridge for the scenario without Split Rock Dam attenuating flows.

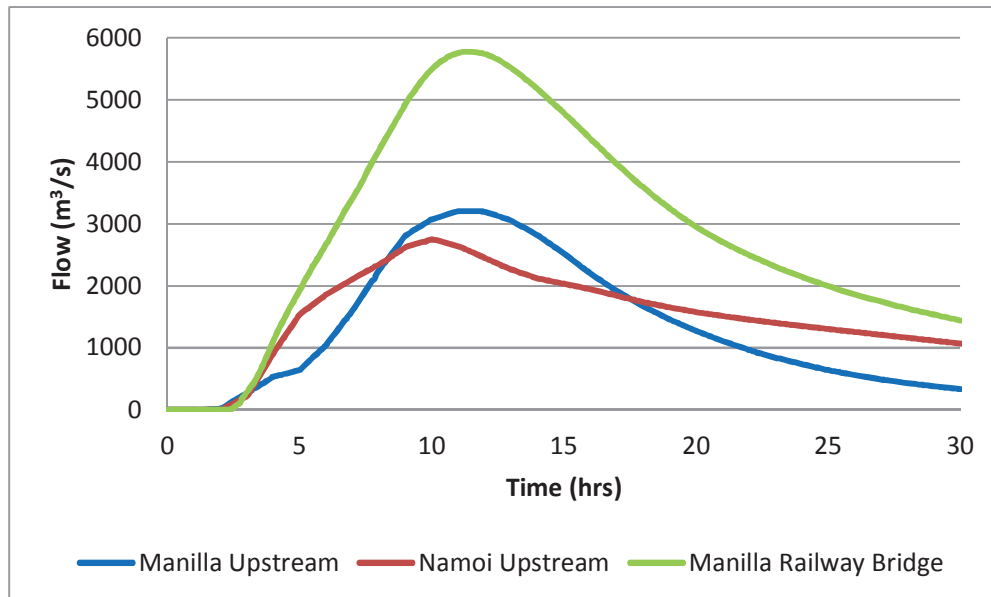
**Table 7-5 Comparison of 1% AEP Flood Peak Estimates at Manilla Railway Bridge**

Flood event AEP	Flood Frequency Analysis (Section 3.2.2)	Calibrated RORB Model (Section 7.2.4)	Probabilistic Rational Method (Section 7.2.5)
Without Split Rock Dam attenuating flows (namely removed from the model)			
1%	5140 m <sup>3</sup> /s	5900 m <sup>3</sup> /s	5300 m <sup>3</sup> /s
With Split Rock Dam attenuating flows			
1%	3610 m <sup>3</sup> /s	3470 m <sup>3</sup> /s	n/a

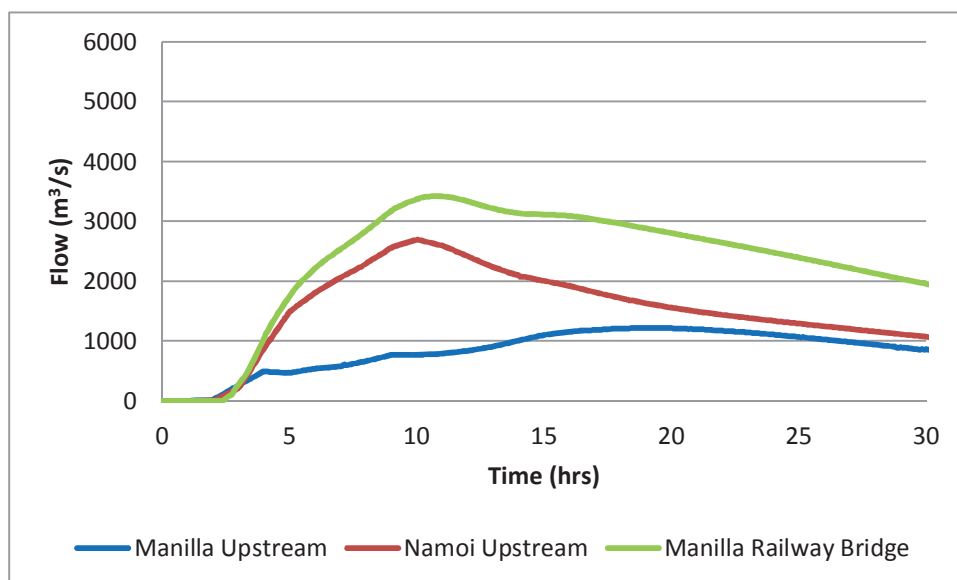
From Figure 7-1 and Figure 7-2 below, the flow attenuation in the Manilla River upstream of Manilla due to Split Rock Dam is clearly evident. The peak flow in the Manilla River upstream of the Manilla township

is reduced by 37%. The attenuation of this peak results in a reduction in the peak flow downstream of the Namoi-Manilla confluence of 60%.

**Figure 7-1 1% AEP Pre-Dam Hydrographs (Critical 18hr storm)**



**Figure 7-2 1% AEP Post-Dam Hydrographs (Critical 18hr Storm)**



## 7.3 Flood Behaviour

### 7.3.1 Upstream Boundary Conditions - Flood Peaks

For the upstream boundary conditions to the TUFLOW model, the design flood hydrographs discussed in Section 7.2.4 were input to the model.





### 7.3.2 Downstream Boundary Conditions

Downstream boundary conditions were configured in the TUFLOW model as per the existing conditions simulations, namely by using a stage discharge relationship

### 7.3.3 Results

The results of the design flood simulations have been provided as a series of flood maps (Appendix F), namely as:

- ▶ a series of maps showing flood depth in blue, overlain by flood level contours;
- ▶ a series of maps showing flood velocities; and
- ▶ a series of maps showing flood hazard in accordance with the NSW Floodplain Development Manual.

Referring to the flood maps, the following is noted:

- ▶ In a 20% AEP event, flow is mostly contained to the river channels. Some spillage onto the floodplain could be expected in the area of Lloyd Street in North Manilla. In a 1% AEP event, flooding would be expected in North Manilla, north of the confluence. Wide spread flooding would also be expected at Lloyd, Charles and Manilla Streets. South and east of the confluence, areas of River Street would be inundated as would the Dewhurst and Rowan Street areas of town. Flood depths vary, from shallow depths along the edge of the floodplain to depths in excess of 5 to 10 m near the creek channel. West of the Namoi and Manilla Rivers, small areas of the floodplain would be inundated;
- ▶ Flow velocities associated with the river channel and immediately adjacent floodplain, are high, around 2 m/s and greater. There are a number of areas in the northern and western floodplains, where flow velocities are in the order of 1 m/s to 2 m/s. Further away from the main channels, the flow velocities are much lower at around 0.5 to 1 m/s;
- ▶ Large areas of the floodplain can be designated as high hazard, on account of deep flow and/or rapid flow velocities. This includes areas of town in particular along River Street and areas of North Manilla;
- ▶ In larger events; a number of areas exist, where flood runners could develop in the floodplain. This is particularly evident on the southern flood plain on the Namoi River and along River Street near the oval; and
- ▶ In a PMF wide spread flooding would be expected. Flood depths would be in excess of 10m, immediately adjacent to the creeks.

The results of the 1% AEP flood simulations have been used to derive the Flood Planning Area, namely the land below the Flood Planning Level on a Flood Planning Map. The Flood Planning Level refers to the level of a 1% AEP flood event plus 0.50m freeboard. For the current study, the Flood Planning Area Extent has been determined by abstracting the peak 1% AEP flood level plus 0.5m, at approximately 100m to 150m intervals across the entire floodplain, perpendicular to the flow. This has been intersected with the topography to produce the Flood Planning Area Extent line. In determining this Flood Planning Area Extent line, some anomalies could exist on the edge of the floodplain, when comparing to the 2D flood model results where localised flood effects could exist. The Flood Planning Map for Manilla is provided in Appendix I.



## 7.4 Sensitivity Analyses

### 7.4.1 Overview

A number of sensitivity analysis were undertaken to determine the impacts of parameters and assumptions on flood behaviour. This was achieved by making the adjustments to the models and re-simulation of both the RORB and TUFLOW models where appropriate. Since the most important event used in planning in NSW is the 1% AEP event, the assessments were primarily done for this event. The results are presented as difference maps in Appendix G. The items/assumptions assessed in the sensitivity analysis were:

- Sensitivity of rainfall loss parameters on the design flood hydrographs and flood levels;
- Sensitivity of Manning's roughness assumptions on flood levels;
- Sensitivities of culvert and bridge blockages and loss assumptions; and
- Future Climate impacts on rainfall and flood levels.

### 7.4.2 Sensitivity of Rainfall Loss Parameters

To assess the impacts of rainfall loss parameter assumptions, both the RORB and the TUFLOW models were re-simulated using the amended assumptions tabulated below. The impacts on the simulated flood peaks using the RORB model are shown in Table 7-7, generally showing a 14 to 20% increase in flood peak. The impacts on the 1% AEP flood level is presented in Appendix G, showing that:

- A reduction in rainfall losses as tabulated below could lead to increases of 300 to 400 mm across a number of areas in the floodplain; and
- These increases would increase the extent of flooding, particularly around Dewhurst and Rowan Streets, where additional properties would be inundated.

**Table 7-6 Rainfall Loss Sensitivity Values**

Event	Initial Loss	Continuing Loss	Initial Loss	Continuing Loss
	Default Value		Sensitivity Value	
Up to and including the 1% AEP event	15 mm	2.5 mm/hr	10 mm	2.0 mm/hr
1% event up to the PMF	15 mm	1 mm/hr	10 mm	0.5 mm/hr
PMF	15 mm	1 mm/hr	10 mm	0.5 mm/hr

**Table 7-7 Rainfall Loss Sensitivity impacts on Flood Peaks (at Manilla Railway Bridge)**

Flood event AEP	Default Value	Sensitivity Value
20%	1230	1510
10%	1600	1895



Flood event AEP	Default Value	Sensitivity Value
5%	2090	2480
2%	2780	3230
1%	3450	3920

### 7.4.3 Sensitivity to Manning's Roughness Assumptions

To assess the impacts of roughness assumptions, the TUFLOW model was re-simulated using the amended roughness assumptions tabulated below. These generally represent between a 10% and 40% increase in topography roughness. The impacts on the 1% AEP flood level is presented in Appendix G, showing that:

- Increases in roughness as defined in the table below could lead to increases of up to 1m across large areas of the flood plain; and
- A number of areas in the flood plain would be expected to increase in extent. For example the residential areas around River Street and Rowan Streets would be expected to experience more wide spread flooding.

**Table 7-8 Roughness Sensitivity Values**

Feature	Default Value	Sensitivity Value
Manning's "n" – roads	0.02	0.025
Manning's "n" – light vegetation mostly grass	0.06	0.08
Manning's "n" – medium vegetation, thicker trees and some bush	0.10	0.12
Manning's "n" – dense vegetation, mostly dense bush	0.12	0.15
Manning's "n" – creeks and natural channels	0.03	0.045
Manning's "n" – developed areas (residential, commercial, industrial, farm sheds)	0.5	0.5
Manning's "n" – houses or blocked out with storage areas (zero conveyance)	2	2

### 7.4.4 Sensitivities of Culvert and Bridge Blockages and Loss Assumptions

To assess the impacts of culvert and bridge blockages, the TUFLOW model was re-simulated using the amended waterway opening assumptions tabulated below. These generally represent the impacts should

debris block bridges during flood events, potentially resulting in local increase in upstream flood levels and potential redistribution of flood flows. To simulate the blockages, the flood level at the structure was determined, and half of the flow depth was blocked below the bridges structure, starting at the flood water surface. The impacts on the 1% AEP flood level is presented in Appendix G, showing that:

- ▶ Flood level increases in the order of 0.2 to 0.4m could be expected throughout the study area, increasing to as much as 0.8m to 1m, immediately upstream of the bridges. This is due to local attenuation at the bridge crossings; and
- ▶ The localised blockage at the Rushes Creek Road crossing is most severe, since this is generally the bridge with the smallest waterway opening in the study area, thus blockage would have the most significant impact.

**Table 7-9 Culvert and Bridge Blockages and Loss Values**

Structure	Default Value	Sensitivity Value
Mandowa Street Bridge	85m concrete span structure Waterway opening of 15.5m and a diagonal waterway opening of 18.4 5 concrete pylons with headstocks	50% blockage has been assumed from the flood water surface
Manilla Street Bridge	300m span structure 1 trussed span and 20 circular steel piers Waterway opening of 38.6m and a diagonal waterway opening of 39.8m	50% blockage has been assumed from the flood water surface
Manilla Railway Bridge	520m spanning structure 2 approach spans on wooden piers over the floodplain and a central trussed span on concrete pylons spanning the Manilla River. Waterway opening of 75m and a diagonal waterway opening of 76m	The trusses on the left and right bank have been assumed to be 100% blocked
Higgin's Avenue Bridge (Causeway)	63m span structure single 2 lane concrete deck span on 6 low set square concrete piers with headstocks Waterway opening of 8.5m and a diagonal waterway opening of 8.7m.	100% blockage has been assumed
Rushes Creek Road Bridge	29.5m span bridge Single 2-lane span with Diagonal waterway opening of 8.7m	50% blockage has been assumed from the flood water surface



#### 7.4.5 Future Climate Impacts on Rainfall

Future climate impacts on rainfall have been assessed generally in accordance with the NSW Government, Department of Environment & Climate Change, Practical Consideration of Climate Change (NSW DECC 2007) guideline. For this assessment the hydrological RORB models was updated to represent future climate rainfall intensities based on the suggestions in the guideline. This recommends simulating 10%, 20% and 30% increases in rainfall intensities. On the basis of this guideline, the estimated future climate rainfall simulated in the RORB model is tabulated below.

The impacts on the simulated flood peaks using the RORB model are shown in Table 7-11, generally showing a significant increase in flood peak. The impacts on the 1% AEP flood level is presented in Appendix G, showing that:

- ▶ A future climate with a 10% increase in rainfall could lead to flood level increases of up to between 0.6m and 0.8m across large areas of the flood plain. For a 30% increase in rainfall, increases of up to between 1.5m and 2.2m across large areas of the flood plain could be expected. The larger increases are generally downstream of the confluence; and
- ▶ A number of areas in the flood plain would be expected to increase in extent.

**Table 7-10 Existing and Future Climate 1% AEP Rainfall**

Rainfall	Critical Duration	Existing Climate	Future Climate (mm)		
			10%	20%	30%
mm/hr	100yr 18 hr	141.6	155.6	169.8	183.9

**Table 7-11 Future Climate 1% AEP Rainfall Sensitivity impacts on Flood Peaks**

Flood event AEP	Default Value	Sensitivity Value		
		10%	20%	30%
20%	1230	1500	1795	2050
10%	1600	1910	2205	2560
5%	2090	2475	2910	3355
2%	2780	3300	3840	4385
1%	3450	4060	4690	5535



## 8. Flood Damage Estimates

### 8.1 Flood Damage Extents

In order to provide a benefit-cost assessment of floodplain management options, it is necessary to estimate the costs of flood damages. Flood damages are determined by assessing the numbers of flood affected properties and then estimating a direct damage cost for a range of flooding events (in terms of flood depth). The resulting depth-damage curves are used as a basis for estimating other direct and indirect costs from flooding.

Flood affected properties were estimated from the results of hydraulic simulations from the flood study, together with floor level survey data collected as part of this study. For each design flood, flood levels were determined at each property location based on these surveyed floor levels. Numbers of inundated properties are shown in Table 8-1. It must be noted that these relatively low levels of inundation noted for events below the 1% AEP event are attributed to the influences of Split Rock dam, and its impact on attenuation of flood peaks in the Manilla River. For this assessment of flood damages, it was assumed Split Rock Dam was at Full Supply Capacity. The flood mapping shows the depth of floor level inundation for a range of events. From the mapping, the following is noted:

- ▶ In events up to the 5% AEP event (Figure D.1) floor levels of properties are not expected to be inundated across the floodplain;
- ▶ In a 2% AEP event, 1 property is expected to experience flooding above the floor level. In a 1 % AEP event the number of properties expected to experience floor level inundation is 9. The properties are primarily in the vicinity of River Street;
- ▶ Beyond the 1% event, the numbers of floor levels inundated increases rapidly with 130 floor levels inundated in a 0.5% event and 545 floor levels expected to be inundated in a PMF.

It must be noted that these relatively low levels of inundation noted for events below the 1% AEP event are attributed to the influences of Split Rock dam, and its impact on attenuation of flood peaks in the Manilla River. For this assessment of flood damages, it was assumed Split Rock Dam was at Full Supply Capacity.

**Table 8-1 Flood Affected Properties**

Flood event AEP	Number of Residential Dwellings Inundated
20%	0
10%	0
5%	0
2%	1
1%	9
0.5%	130
PMF	545

## 8.2 Depth-Damage Relationship

For most residential dwellings, flood damage increases with the depth of flooding. The Floodplain Management (FDM) and Coastal Support Section of the Department of Natural Resources (DNR, now Office of Environment and Heritage) has developed a relationship between flood depth and damage based on various parameters for house and contents value, and flooding characteristics.

The relationship is illustrated in Figure 8-1. Parameters used to derive this relationship are shown below in Table 8-2.

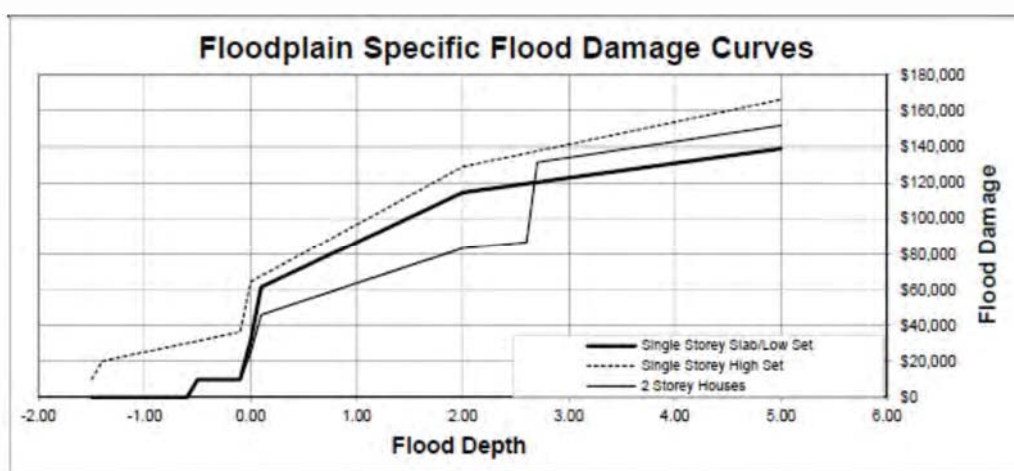


Figure 8-1 Typical Depth Damage Relationships (OEH, 2007)

Table 8-2 Parameters for Depth-Damage Relationship

Parameter	Value
Additional accommodation/ loss of rent	\$220 per week
Average contents value	\$60,000
Average house size	240 m <sup>2</sup>
Clean up costs	\$4,000
External damage	\$6,700
Typical duration of inundation	21 hour
Typical table/bench height	0.9 m
Design Life of Options	20 year
Discount factor for Cost Calculations	7 %



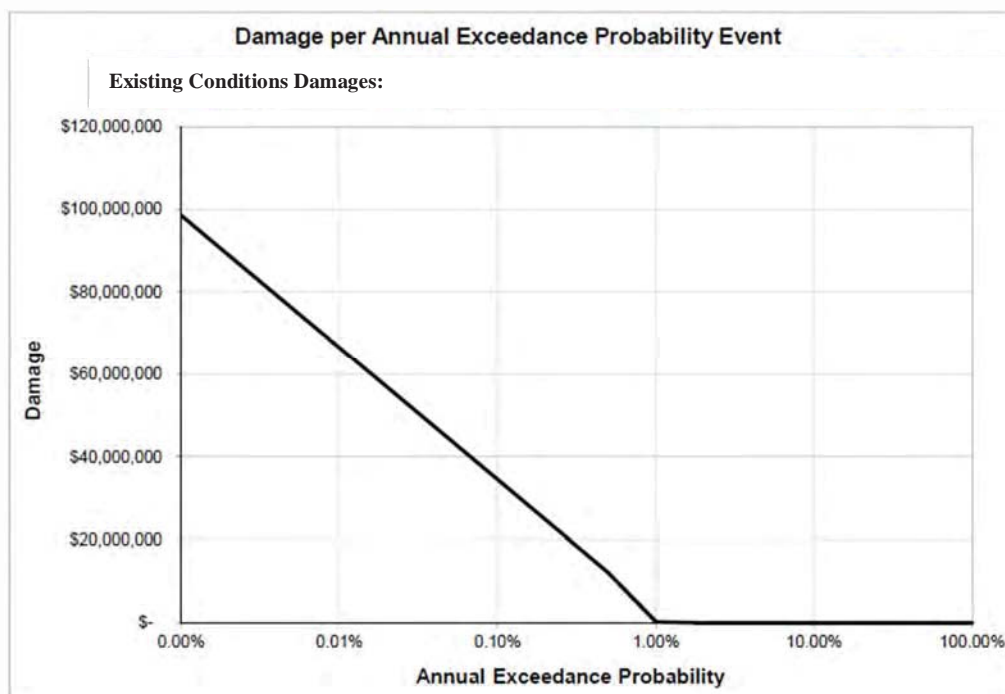
### 8.3 Flood Damage Calculations

The following methodology was used to estimate the Average Annual Damage (AAD) and present value (PV) of the AAD over a 20-year period:

- ▶ The floor levels of properties affected by flooding for a range of flood events were estimated from the flood simulations and floor level survey;
- ▶ The cost of damage for the flooding was estimated for each flood event and depth range, using typical house and contents damage cost and the percentage of damage for the particular depth;
- ▶ A direct damage bill for each storm event was calculated;
- ▶ Flood recurrence interval was plotted against total damage and integrated to find the area under the graph, which provides the AAD; and
- ▶ A present value for the AAD was estimated based on a 7% discount rate over a 20-year period.

For Manilla, the Annual Average Damage Curve for residential properties is shown in Figure 8-2. The Annual Average Damage is estimated at \$300k. Over a 20-year period, this has a Net Present Value of \$3.28 million.

In comparison to other recent studies undertaken in NSW, this damage bill is relatively less. This is attributed to the influences of Split Rock dam, and its impact on attenuation of flood peaks in the Manilla River. For this assessment of flood damages, it was assumed Split Rock Dam was at Full Supply Capacity.



**Figure 8-2 Manilla Damage Curve**

## 9. Summary and Conclusions

- ▶ Manilla town is located at the confluence of the Manilla and Namoi Rivers which drain to Keepit Dam in the New England region of NSW. Manilla is located 43 km to the northwest of Tamworth and has population of approximately 2100. The two rivers draining through Manilla command a considerable catchment area of some 5150 km<sup>2</sup>. The catchments are primarily rural, with number of towns located in the catchment. Floodwaters in both catchments tend to rise quickly and isolate communities and properties for several days. Many houses can be inundated in flood events necessitating evacuations. Some rainfall and river gauging data in the catchment is available, and significant events have been recorded in Manilla. Significant events noted in the literature and mentioned by residents, most notably the 1955 event and the 1964 flood;
- ▶ Tamworth Regional Council is responsible for local land use planning in Manilla. Through its Floodplain Risk Management Committee, Tamworth Regional Council proposes to prepare a comprehensive floodplain risk management plan for the study area in accordance with the NSW Government's "Floodplain Development Manual: the management of flood liable land", April 2005 (The Manual);
- ▶ The primary objective of this study was to define the main-stem flood behaviour under historical conditions and design flood behaviour under existing and future climate conditions in the study area (Figure A2 in Appendix A). The study produced information on flood levels, depths, velocities, flows, hydraulic categories, and provisional hazard categories for a full range of design and historical flood events. In addition, the study produced estimates of flood damage;
- ▶ A number of community consultation activities were undertaken as part of the study, The primary objectives of the flood study consultation activities were to inform the relevant government agencies that the study is being undertaken, informing relevant local community groups, and informing the general public. The information provided by the community, showed that a large number of the residents had experienced flooding in the town of Manilla, first hand and flood levels tend to rise and recede very rapidly. Preparation for flooding includes regular observation of river levels and lifting of belongings. In addition, listening to advice on local radio stations and from the SES. Many residents often mentioned the 1964 floods and these have been etched in memories of the Manilla community. One resident noted 8 m of flood waters through the house while another had their house completely submerged. Residents also noted that flooding had been reduced by Split Rock Dam, however could still be a high risk, if Split Rock Dam was full as occurred in the 1998 flood;
- ▶ The hydrology for the Manilla flood study was developed using the RORB hydrological model. The model was setup as an end of catchment model, producing flood hydrographs for the Namoi and Manilla Rivers upstream of the Manilla town. The RORB model was calibrated by variation of model parameters to obtain a good fit of the calculated to the measured hydrograph. A number of sensitivity analysis were undertaken on model parameters and the RORB model was simulated for a range of durations ranging up to 72 hours;
- ▶ The flood conveyance through Manilla was calculated using the TUFLOW hydraulic model. The model extent for the purposes of flood mapping was defined in collaboration with Tamworth Regional Council. The TUFLOW model compilation configured the key parameters including DTM data for the local area, triangulated to represent the ground surface. All bridges within the floodplain were configured using the terrestrial survey data. These were configured within the 2D model grid;

- ▶ To determine the design flood behaviour, both the RORB and TUFLOW models were simulated, using the parameters derived through the calibrations together with design rainfall in accordance with the Australian Rainfall and Runoff (AR&R 2011);
- ▶ The results show that in a 20% AEP event, flow is mostly contained to the river channels. Some spillage onto the floodplain could be expected in the area of Lloyd Street in North Manilla. In a 1% AEP event, flooding would be expected in North Manilla, north of the confluence. Wide spread flooding would also be expected at Lloyd, Charles and Manilla Streets. South and east of the confluence, areas of River Street would be inundated as would the Dewhurst and Rowan Street areas of town. Flood depths vary, from shallow depths along the edge of the floodplain to depths in excess of 5 to 10 m near the creek channel. West of the Namoi and Manilla Rivers, small areas of the floodplain would be inundated. Flow velocities associated with the river channel and immediately adjacent floodplain, are high, around 2 m/s and greater. There are a number of areas in the northern and western floodplains, where flow velocities are in the order of 1 m/s to 2 m/s. Further away from the main channels, the flow velocities are much lower at around 0.5 to 1 m/s. Large areas of the floodplain can be designated as high hazard, on account of deep flow and/or rapid flow velocities. This includes areas of town in particular along River Street and areas of North Manilla. In larger events, a number of areas exist, where flood runners could develop in the floodplain. This is particularly evident on the southern flood plain on the Namoi River and along River Street near the oval;
- ▶ A sensitivity assessment was undertaken on a number of key parameters. A reduction in rainfall losses could lead to increases of 300 to 400 mm across a number of areas in the floodplain. Increases in roughness could lead to increases of up to 1 m across large areas of the flood plain and a number of areas in the flood plain would be expected to increase in flood extent. A future climate with a 10% increase in rainfall could lead to flood level increases of up to between 0.6m and 0.8m across large areas of the flood plain. For a 30% increase in rainfall, increases of up to between 1.5m and 2.2m across large areas of the flood plain could be expected. The larger increases are generally downstream of the confluence. Flood level increases in the order of 0.2 to 0.4m could be expected throughout the study area due to bridge blockages, increasing to as much as 0.8m to 1m, immediately upstream of the bridges. This is due to local attenuation at the bridge crossings; and
- ▶ A flood damage assessment was undertaken using the Floodplain Management (FDM) and Coastal Support Section of the Department of Natural Resources (DNR, now Office of Environment and Heritage) relationships between flood depth and damage based on various parameters for house and contents value. From this assessment, the Annual Average Damage is estimated at \$ 300k. Over a 20-year period, this has a net present value of \$3.28 million. In comparison to other recent studies undertaken in NSW, this damage bill is relatively less. This is attributed to the influences of Split Rock dam, and its impact on attenuation of flood peaks in the Manilla River. For this assessment of flood damages, it was assumed Split Rock Dam was at Full Supply Capacity.



## 10. References

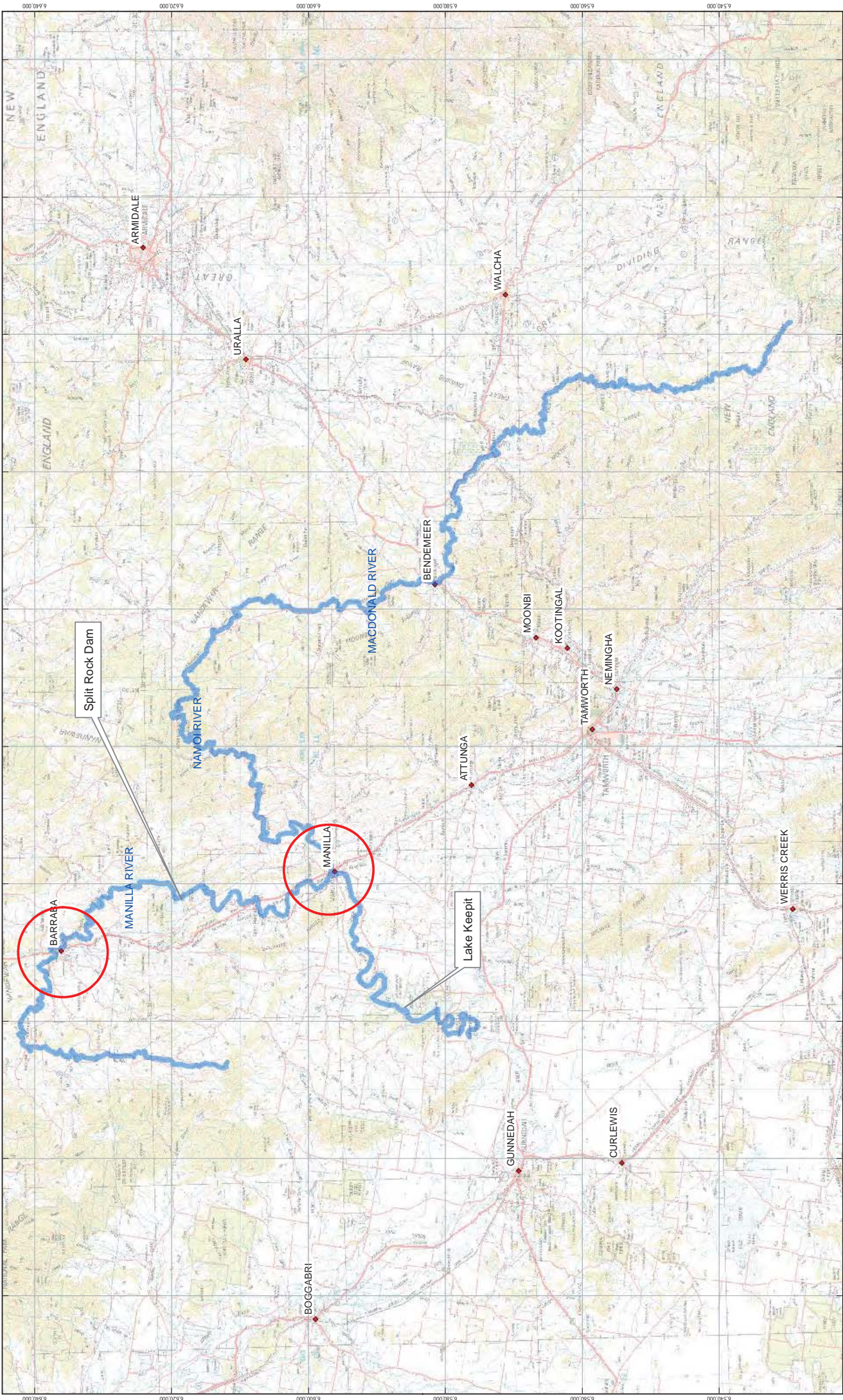
- ▶ NSW Government, 2005, Floodplain Development Manual, Management of Flood Liable Land;
- ▶ NSW DECC 2007, Practical Consideration of Climate Change, NSW Department of Environment & Climate Change;
- ▶ AR&R 2001, Australian Rainfall and Runoff, 2001;
- ▶ Bernard RL, Flood Report Namoi River 1955;
- ▶ Bewsher Consulting, 2007, Assessment of flood risk in various towns and villages, undertaken for Tamworth Regional Council;
- ▶ RORB 6 User Manual, Monash University Department Of Civil Engineering In Conjunction With Sinclair Knight Merz Pty. Ltd. And The Support Of Melbourne Water Corporation, Rorb Version 6 Runoff Routing Program User Manual;
- ▶ BMT WBM 2010, TUFLOW User Manual
- ▶ BOM 2003, Bureau of Meteorology Australia Generalised Tropical Storm Method – Revised Version, November 2003;
- ▶ OEH 2007, Department of Environment and Climate Change, Flood Risk Management Guideline, Residential Flood Damages,
- ▶ NVFIT 1964, Namoi Valley Flood Investigation Trust, Namoi River Flood Investigation – Report 1, SKM, July 1964;
- ▶ WRC, Flood Information Reports, Floods in the Namoi Valley (1955, 1962, 1964, 1971, 1974, 1976), NSW Water Resources Commission;
- ▶ CSIRO (2007). Water availability in the Namoi. A report to the Australian Government from the CSIRO Murray-Darling Basin Sustainable Yields Project. CSIRO, Australia. 154pp;
- ▶ State Water, Keepit Dam Upgrade Environmental Assessment –m excerpt only



## Appendix A

# Location Maps





Tamworth Regional Council  
Manilla and Barraba Flood Study

Locality Map

Figure A.1

Job Number 22-15815/16  
Revision A  
Date 23 December 201

10 Bond Street Sydney NSW 2000 Australia T 61 2 9239 7000 F 61 2 9239 71 99 Eysdmail@ghd.com.au W www.ghd.com.au

LEGEND

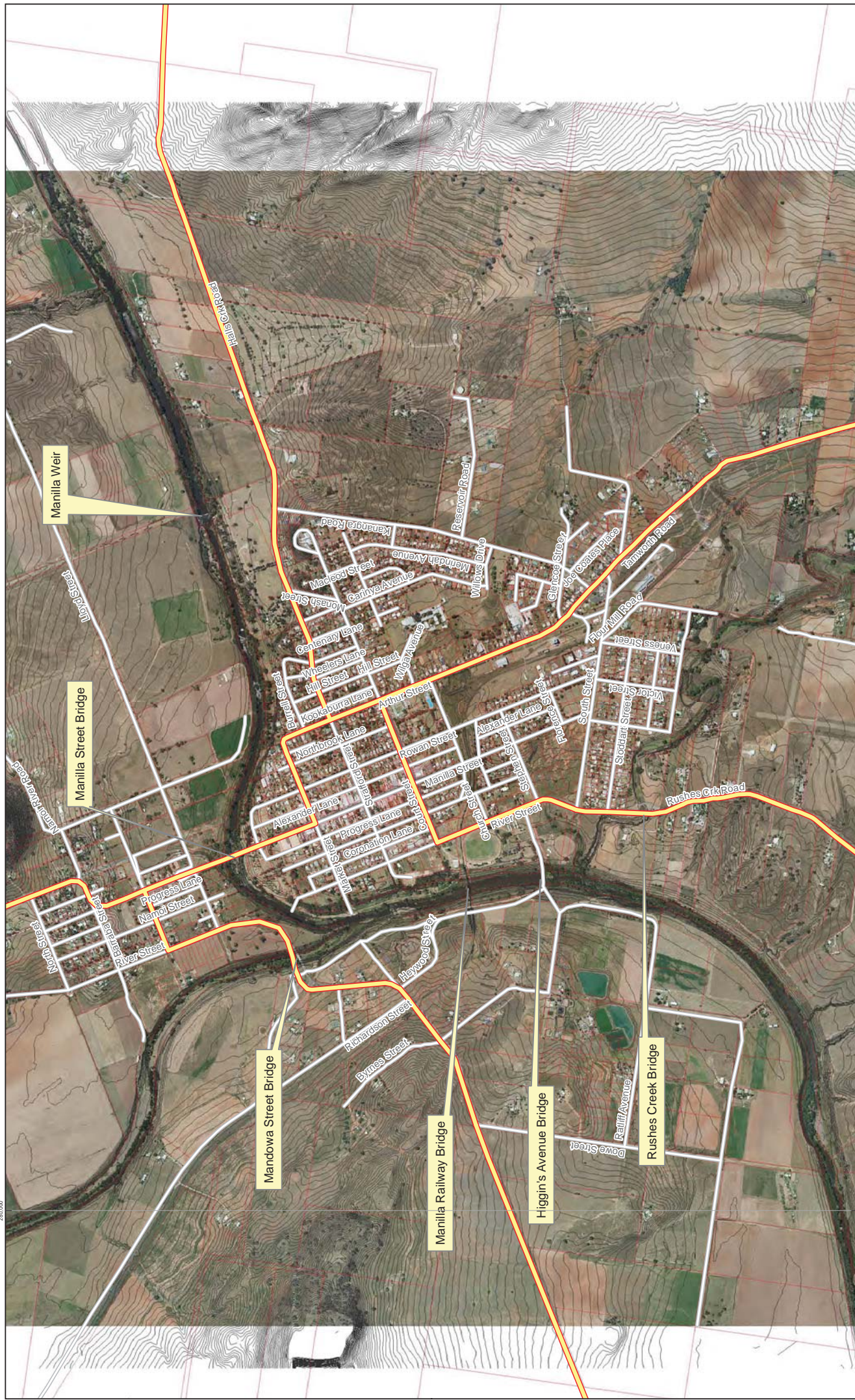


1:500,000  
0 1 2 4 6 8 10  
Kilometres (at A3)

Map Projection: Transverse Mercator  
Horizontal datum: GDA 1984  
Grid: Map Grid of Australia, Zone 56

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G:\1410\72799\FINAL\TEMPLATES\rev 001\MapGIS\GHD\A3-LANDSCAPE.mxd  
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Data source: Data Custodian, Data Set Name/Title, Version/Date. Created by: C. Pappin





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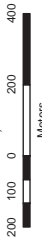
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Highways

Secondary Roads



1:15,000



Map Projection: Transverse Mercator

Grid: Map Grid of Australia, Zone 56  
Datum: Geocentric Datum of Australia 1994

C-14 410107290FINAI TEMPOI ATE SIS DEB 004 VA SCISIGUO-A 2-J ANDSCADE movt

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data source: Data Custodian: Data Set Name/Title: Version/Date: Created by: C. Brown

10 Bond Street Sydney NSW 2000 Australia **T** 61 2 9239 7000 **F** 61 2 9239 7199 **E** [sydmall@ghd.com.au](mailto:sydmall@ghd.com.au) **W** [www.ghd.com.au](http://www.ghd.com.au)

Figure A.2

**Tamworth Regional Council**  
**Manilla and Barraba Flood Study**

Job Number | 22-15815/16

Revision	A
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Study Area

Figure A.2

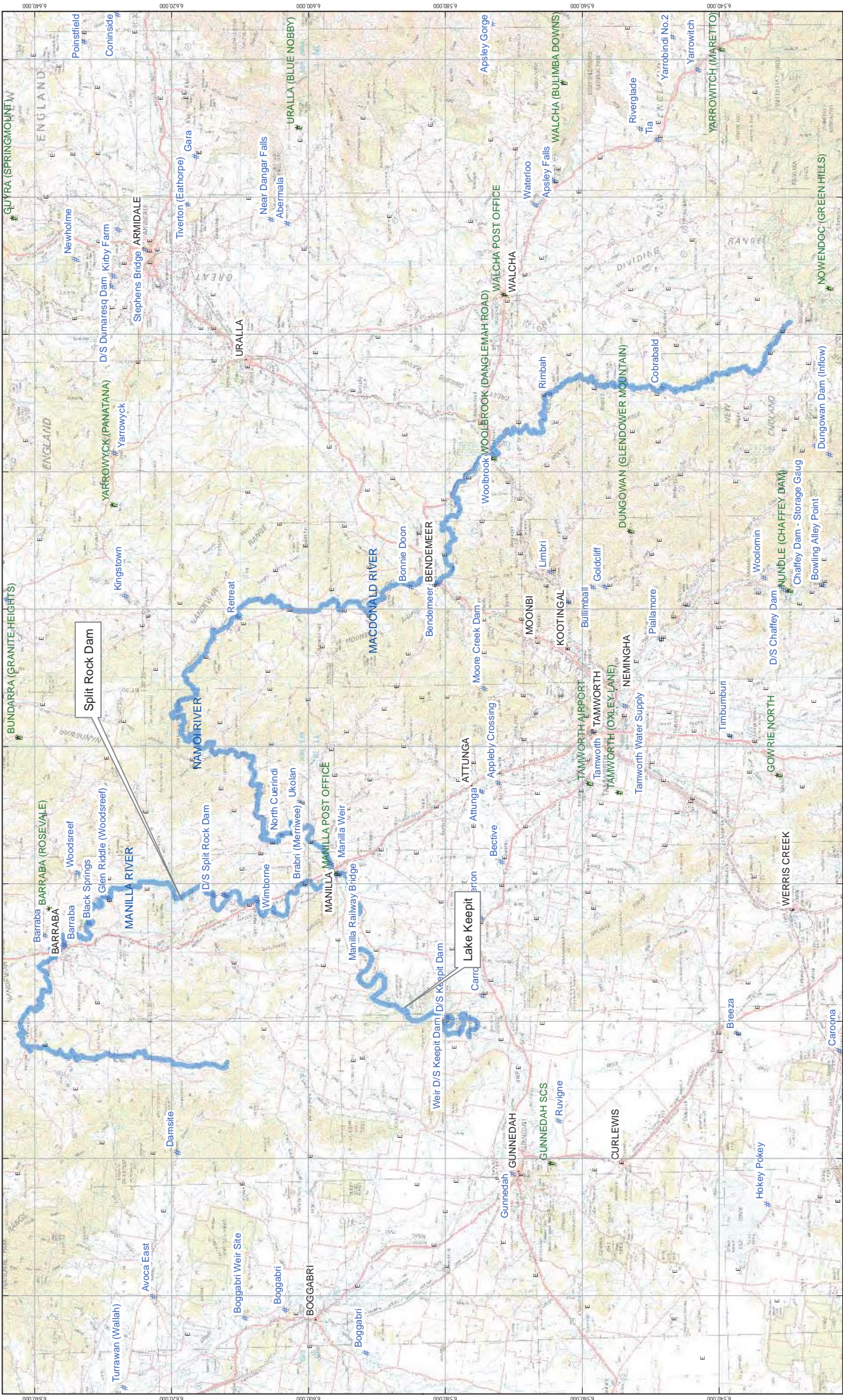




Appendix B

## Gauging Information and Flood Frequency Analysis





1:500,000

0 1 2 4 6 8 10

Kilometres (at A3)

Map Projection: Transverse Mercator

Grid: Map Grid of Australia, Zone 56

O

LEGEND

E

Daily Rainfall Data

#

Pluvio-Rainfall Data

#

River Gauging Data



Tamworth Regional Council	Job Number	22-15815/16
Manilla and Barraba Flood Study	Revision	A
	Date	23 Dec 2011

### Gauging Information

Figure B.1



## Site 419022 NAMOI RIVER AT MANILLA RAILWAY BRIDGE

VarFrom 100.00 Stream Water Level  
VarTo 141.00 Stream Discharge  
Period 01/10/1939 - 30/09/2011

Date	Number	Stage	Flow	Deviation	Rating	-100% Deviation	100%
01:30_19/03/1941	1.0 *	1.130	2245.954000	[N/A]	1	1	1
01:30_20/03/1941	2.0 *	1.842	9069.449000	[N/A]	1	1	1
01:30_24/03/1941	3.0 *	1.334	4166.510000	[N/A]	1	1	1
01:30_03/10/1941	4.0 *	0.584	166.367000	[N/A]	1	1	1
01:30_12/11/1941	5.0 *	0.654	296.035000	[N/A]	1	1	1
01:30_17/04/1942	6.0 *	0.438	53.825000	[N/A]	1	1	1
01:30_18/08/1942	7.0 *	0.756	467.295000	[N/A]	1	1	1
01:30_27/11/1942	8.0 *	0.737	499.100000	[N/A]	1	1	1
01:30_02/02/1943	9.0 *	0.660	232.425000	[N/A]	1	1	1
01:30_16/04/1943	10.0 *	0.457	31.804000	[N/A]	1	1	1
01:30_17/06/1943	11.0 *	0.559	144.347000	[N/A]	1	1	1
01:30_06/08/1943	12.0 *	0.679	296.035000	[N/A]	1	1	1
01:30_04/10/1943	13.0 *	0.978	1416.564000	[N/A]	1	1	1
01:30_17/12/1943	14.0 *	0.673	609.196000	[N/A]	1	1	1
01:30_24/01/1944	15.0 *	0.686	330.288000	[N/A]	1	1	1
01:30_24/10/1944	16.0 *	0.521	95.415000	[N/A]	1	1	1
01:30_22/12/1944	17.0 *	0.330	15.168000	[N/A]	1	1	1
01:30_28/02/1945	18.0 *	1.168	1798.231000	[N/A]	1	1	1
01:30_26/09/1945	19.0 *	0.686	327.841000	[N/A]	1	1	1
01:30_11/12/1945	20.0 *	0.489	80.736000	[N/A]	1	1	1
01:30_08/04/1946	21.0 *	0.464	92.969000	[N/A]	1	1	1
01:30_02/07/1946	22.0 *	0.533	114.988000	[N/A]	1	1	1
01:30_30/09/1946	23.0 *	0.540	161.473000	[N/A]	1	1	1
01:30_19/11/1946	24.0 *	0.343	188.385000	[N/A]	1	1	1
01:30_17/12/1946	25.0 *	0.000	0.000000	[N/A]	1	1	1
01:30_11/02/1947	26.0 *	0.470	61.163000	[N/A]	1	1	1
01:30_16/07/1947	27.0 *	0.514	73.396000	[N/A]	1	1	1
01:30_07/01/1948	28.0 *	1.334	3153.633000	[N/A]	1	1	1
01:30_25/08/1948	29.0 *	0.914	1007.989000	[N/A]	1	1	1
01:30_25/11/1948	30.0 *	0.318	90.523000	[N/A]	1	1	1
01:30_29/07/1952	31.0 *	0.991	1218.393000	[N/A]	1	1	1
01:30_26/09/1952	32.0 *	0.933	1071.598000	[N/A]	1	1	1
01:30_23/10/1952	33.0 *	1.010	1433.691000	[N/A]	1	1	1
01:30_03/12/1952	34.0 *	0.686	296.035000	[N/A]	1	1	1
01:30_14/01/1953	35.0 *	0.610	156.581000	[N/A]	1	1	1
01:30_24/07/1953	36.0 *	0.699	327.841000	[N/A]	1	1	1
01:30_11/09/1953	37.0 *	0.838	729.080000	[N/A]	1	1	1
01:30_14/10/1953	38.0 *	0.679	293.589000	[N/A]	1	1	1
01:30_21/12/1953	39.0 *	0.425	24.465000	[N/A]	1	1	1
01:30_03/06/1954	40.0 *	0.502	51.378000	[N/A]	1	1	1
01:30_13/08/1954	41.0 *	0.613	129.668000	[N/A]	1	1	1
01:30_15/09/1954	42.0 *	0.572	78.290000	[N/A]	1	1	1
01:30_05/11/1954	43.0 *	2.286	11141.694000	[N/A]	1	1	1
02:30_05/11/1954	44.0 *	2.096	9399.733000	[N/A]	1	1	1
02:30_06/11/1954	45.0 *	1.740	4907.824000	[N/A]	1	1	1
02:30_06/11/1954	46.0 *	1.610	6011.226000	[N/A]	1	1	1
01:30_07/11/1954	47.0 *	1.372	3434.989000	[N/A]	1	1	1
01:30_20/12/1954	48.0 *	0.705	366.986000	[N/A]	1	1	1
01:30_10/08/1955	49.0 *	1.080	1485.070000	[N/A]	1	1	1
01:30_24/10/1955	50.0 *	8.573	154623.424000	[N/A]	1	1	1
02:30_24/10/1955	51.0 *	9.335	182025.119000	[N/A]	1	1	1
01:30_25/10/1955	52.0 *	5.448	49665.455000	[N/A]	1	1	1
02:30_25/10/1955	53.0 *	6.325	80492.175000	[N/A]	1	1	1
01:30_26/10/1955	54.0 *	4.178	34922.399000	[N/A]	1	1	1
01:30_27/10/1955	55.0 *	3.124	21050.303000	[N/A]	1	1	1

\* after gauge number indicates Weighted Mean Gauge Height method

Site 419022 NAMOI RIVER AT MANILLA RAILWAY BRIDGE							
VarFrom	100.00	Stream Water Level		VarTo	141.00	Stream Discharge	
Period 01/10/1939 - 30/09/2011							
Date	Number	Stage	Flow	Deviation	Rating	-100% Deviation	100%
01:30_28/10/1955	56.0 *	2.515	14062.906000	[N/A]	1	1	1
00:00_23/11/1955	57.0 *	1.051	14445.940000	[N/A]	1	1	1
01:30_23/11/1955	58.0 *	1.048	14445.925000	[N/A]	1	1	1
01:30_26/01/1956	59.0 *	0.864	7511.099000	[N/A]	1	1	1
01:30_16/04/1956	60.0 *	1.054	1450.817000	[N/A]	1	1	1
01:30_31/07/1956	61.0 *	1.226	2803.774000	[N/A]	1	1	1
01:30_20/09/1956	62.0 *	0.914	1059.365000	[N/A]	1	1	1
01:30_27/11/1956	63.0 *	0.762	462.402000	[N/A]	1	1	1
01:30_13/02/1957	64.0 *	0.648	247.104000	[N/A]	1	1	1
01:30_05/04/1957	65.0 *	0.876	161.473000	[N/A]	1	1	1
01:30_12/06/1957	66.0 *	0.711	124.775000	[N/A]	1	1	1
02:30_12/06/1957	67.0 *	0.711	129.668000	[N/A]	1	1	1
01:30_22/08/1957	68.0 *	0.737	168.814000	[N/A]	1	1	1
01:30_22/10/1957	69.0 *	0.635	63.610000	[N/A]	1	1	1
01:30_13/11/1957	70.0 *	0.616	36.699000	[N/A]	1	1	1
02:30_13/11/1957	71.0 *	0.616	37.677000	[N/A]	1	1	1
01:30_18/12/1957	72.0 *	0.671	112.542000	[N/A]	1	1	1
02:30_18/12/1957	73.0 *	0.686	107.648000	[N/A]	1	1	1
01:30_21/01/1958	74.0 *	0.625	56.271000	[N/A]	1	1	1
01:30_22/01/1958	75.0 *	1.082	1022.668000	[N/A]	1	1	1
01:30_21/02/1958	76.0 *	0.815	349.859000	[N/A]	1	1	1
01:30_25/03/1958	77.0 *	0.648	102.756000	[N/A]	1	1	1
01:30_25/03/1958	78.0 *	0.625	75.844000	[N/A]	1	1	1
01:30_03/06/1958	79.0 *	0.648	73.396000	[N/A]	1	1	1
01:30_02/07/1958	80.0 *	0.808	342.521000	[N/A]	1	1	1
01:30_28/08/1958	81.0 *	0.820	349.859000	[N/A]	1	1	1
01:30_05/11/1958	82.0 *	0.738	310.714000	[N/A]	1	1	1
01:30_01/12/1958	83.0 *	0.610	63.610000	[N/A]	1	1	1
01:30_07/01/1959	84.0 *	1.250	2167.664000	[N/A]	1	1	1
01:30_17/03/1959	85.0 *	0.792	486.868000	[N/A]	1	1	1
01:30_23/04/1959	86.0 *	0.738	332.734000	[N/A]	1	1	1
01:30_01/06/1959	87.0 *	0.695	193.279000	[N/A]	1	1	1
01:30_22/07/1959	88.0 *	0.838	567.605000	[N/A]	1	1	1
01:30_23/09/1959	89.0 *	1.052	1184.142000	[N/A]	1	1	1
01:30_29/10/1959	90.0 *	0.811	496.654000	[N/A]	1	1	1
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01:30_13/01/1960	92.0 *	0.663	181.047000	[N/A]	1	1	1
01:30_02/03/1960	93.0 *	0.631	112.542000	[N/A]	1	1	1
01:30_15/03/1960	94.0 *	0.668	185.939000	[N/A]	1	1	1
01:30_11/05/1960	95.0 *	0.674	159.027000	[N/A]	1	1	1
01:30_22/07/1960	96.0 *	1.225	2047.781000	[N/A]	1	1	1
01:30_02/08/1960	97.0 *	1.551	3782.405000	[N/A]	1	1	1
01:30_16/08/1960	98.0 *	0.969	1010.435000	[N/A]	1	1	1
01:30_30/08/1960	99.0 *	0.811	508.887000	[N/A]	1	1	1
01:30_27/09/1960	100.0 *	1.414	3368.934000	[N/A]	1	1	1
01:30_03/11/1960	101.0 *	0.808	484.421000	[N/A]	1	1	1
01:30_23/11/1960	102.0 *	1.628	4839.315000	[N/A]	1	1	1
01:30_16/01/1961	103.0 *	0.642	151.688000	[N/A]	1	1	1
01:30_06/02/1961	104.0 *	0.564	53.825000	[N/A]	1	1	1
01:30_21/03/1961	105.0 *	0.579	68.503000	[N/A]	1	1	1
01:30_19/04/1961	106.0 *	0.576	53.825000	[N/A]	1	1	1
01:30_15/05/1961	107.0 *	0.622	97.863000	[N/A]	1	1	1
01:30_21/06/1961	108.0 *	0.683	166.367000	[N/A]	1	1	1
01:30_17/07/1961	109.0 *	0.634	100.310000	[N/A]	1	1	1
01:30_15/08/1961	110.0 *	0.710	225.084000	[N/A]	1	1	1

\* after gauge number indicates Weighted Mean Gauge Height method

Site 419022		NAMOI RIVER AT MANILLA RAILWAY BRIDGE					
VarFrom 100.00		Stream Water Level					
VarTo 141.00		Stream Discharge					
Period 01/10/1939 - 30/09/2011							
Date	Number	Stage	Flow	Deviation	Rating	-100% Deviation	100%
01:30_26/09/1961	111.0 *	0.735	298.481000	[N/A]	1	.....	.....
01:30_06/11/1961	112.0 *	0.677	200.618000	[N/A]	1	.....	.....
02:30_06/11/1961	113.0 *	0.671	185.939000	[N/A]	1	.....	.....
01:30_12/12/1961	114.0 *	0.832	631.216000	[N/A]	1	.....	.....
02:30_12/12/1961	115.0 *	0.829	616.536000	[N/A]	1	.....	.....
01:30_05/03/1962	116.0 *	0.872	423.257000	[N/A]	1	.....	.....
01:30_06/03/1962	117.0 *	0.869	376.773000	[N/A]	1	.....	.....
01:30_10/04/1962	118.0 *	1.896	6539.686000	[N/A]	1	.....	.....
02:30_10/04/1962	119.0 *	1.859	6520.120000	[N/A]	1	.....	.....
01:30_30/04/1962	120.0 *	0.835	354.753000	[N/A]	1	.....	.....
02:30_30/04/1962	121.0 *	0.835	325.395000	[N/A]	1	.....	.....
01:30_12/06/1962	122.0 *	0.890	413.470000	[N/A]	1	.....	.....
02:30_12/06/1962	123.0 *	0.890	425.703000	[N/A]	1	.....	.....
01:30_02/07/1962	124.0 *	0.814	212.851000	[N/A]	1	.....	.....
02:30_02/07/1962	125.0 *	0.814	222.638000	[N/A]	1	.....	.....
01:30_14/08/1962	126.0 *	0.850	310.714000	[N/A]	1	.....	.....
02:30_14/08/1962	127.0 *	0.850	322.947000	[N/A]	1	.....	.....
01:30_17/09/1962	128.0 *	0.896	455.062000	[N/A]	1	.....	.....
02:30_17/09/1962	129.0 *	0.896	415.918000	[N/A]	1	.....	.....
01:30_18/10/1962	130.0 *	0.792	173.706000	[N/A]	1	.....	.....
02:30_18/10/1962	131.0 *	0.792	185.939000	[N/A]	1	.....	.....
01:30_13/11/1962	132.0 *	0.811	386.558000	[N/A]	1	.....	.....
02:30_13/11/1962	133.0 *	0.811	396.345000	[N/A]	1	.....	.....
01:30_13/12/1962	134.0 *	0.811	274.015000	[N/A]	1	.....	.....
02:30_13/12/1962	135.0 *	0.811	298.481000	[N/A]	1	.....	.....
01:30_15/01/1963	136.0 *	0.841	574.945000	[N/A]	1	.....	.....
02:30_15/01/1963	137.0 *	0.841	572.498000	[N/A]	1	.....	.....
01:30_06/02/1963	138.0 *	0.814	455.062000	[N/A]	1	.....	.....
02:30_06/02/1963	139.0 *	0.814	481.975000	[N/A]	1	.....	.....
01:30_13/03/1963	140.0 *	0.625	73.396000	[N/A]	1	.....	.....
02:30_13/03/1963	141.0 *	0.625	70.951000	[N/A]	1	.....	.....
01:30_09/04/1963	142.0 *	0.884	665.468000	[N/A]	1	.....	.....
02:30_09/04/1963	143.0 *	0.884	606.751000	[N/A]	1	.....	.....
01:30_06/05/1963	144.0 *	0.933	782.904000	[N/A]	1	.....	.....
02:30_06/05/1963	145.0 *	0.933	765.777000	[N/A]	1	.....	.....
01:30_29/05/1963	146.0 *	1.210	2360.943000	[N/A]	1	.....	.....
02:30_29/05/1963	147.0 *	1.210	2312.011000	[N/A]	1	.....	.....
01:30_18/07/1963	148.0 *	1.317	3053.324000	[N/A]	1	.....	.....
02:30_18/07/1963	149.0 *	1.317	3173.206000	[N/A]	1	.....	.....
01:30_22/08/1963	150.0 *	0.933	961.503000	[N/A]	1	.....	.....
02:30_22/08/1963	151.0 *	0.933	988.417000	[N/A]	1	.....	.....
01:30_02/10/1963	152.0 *	1.177	2020.869000	[N/A]	1	.....	.....
02:30_02/10/1963	153.0 *	1.177	1998.850000	[N/A]	1	.....	.....
01:30_22/10/1963	154.0 *	0.902	785.350000	[N/A]	1	.....	.....
02:30_22/10/1963	155.0 *	0.902	848.962000	[N/A]	1	.....	.....
01:30_14/11/1963	156.0 *	0.890	719.292000	[N/A]	1	.....	.....
02:30_14/11/1963	157.0 *	0.890	751.099000	[N/A]	1	.....	.....
01:30_06/12/1963	158.0 *	0.747	423.257000	[N/A]	1	.....	.....
02:30_06/12/1963	159.0 *	0.747	420.811000	[N/A]	1	.....	.....
01:30_07/01/1964	160.0 *	0.689	269.123000	[N/A]	1	.....	.....
02:30_07/01/1964	161.0 *	0.689	271.569000	[N/A]	1	.....	.....
01:30_26/02/1964	162.0 *	0.884	362.092000	[N/A]	1	.....	.....
02:30_26/02/1964	163.0 *	0.884	371.879000	[N/A]	1	.....	.....
01:30_07/04/1964	164.0 *	0.890	308.268000	[N/A]	1	.....	.....
02:30_07/04/1964	165.0 *	0.890	308.268000	[N/A]	1	.....	.....

\* after gauge number indicates Weighted Mean Gauge Height method

## Site 419022 NAMOI RIVER AT MANILLA RAILWAY BRIDGE

VarFrom 100.00 Stream Water Level  
VarTo 141.00 Stream Discharge  
Period 01/10/1939 - 30/09/2011

Date	Number	Stage	Flow	Deviation	Rating	-100% Deviation	100%
01:30_20/05/1964	166.0	* 0.917	477.081000	[N/A]	1	1	1
02:30_20/05/1964	167.0	* 0.917	472.188000	[N/A]	1	1	1
01:30_24/06/1964	168.0	* 1.065	1103.404000	[N/A]	1	1	1
02:30_24/06/1964	169.0	* 1.062	1096.063000	[N/A]	1	1	1
01:30_28/07/1964	170.0	* 1.119	1475.282000	[N/A]	1	1	1
02:30_28/07/1964	171.0	* 1.119	1382.312000	[N/A]	1	1	1
01:30_19/08/1964	172.0	* 0.942	628.769000	[N/A]	1	1	1
01:30_22/09/1964	173.0	* 0.939	611.644000	[N/A]	1	1	1
00:00_29/09/1964	174.0	* 2.926	20260.000000	[N/A]	1	1	1
00:01_29/09/1964	175.0	* 2.895	19817.460000	[N/A]	1	1	1
00:00_30/09/1964	176.0	* 2.170	10547.292000	[N/A]	1	1	1
02:30_30/09/1964	177.0	* 1.868	6361.089000	[N/A]	1	1	1
01:30_29/10/1964	178.0	* 0.991	1003.095000	[N/A]	1	1	1
02:30_29/10/1964	179.0	* 0.991	1052.026000	[N/A]	1	1	1
01:30_20/11/1964	180.0	* 0.872	604.303000	[N/A]	1	1	1
02:30_20/11/1964	181.0	* 0.872	601.858000	[N/A]	1	1	1
01:30_06/01/1965	182.0	* 0.735	227.531000	[N/A]	1	1	1
02:30_06/01/1965	183.0	* 0.735	229.977000	[N/A]	1	1	1
01:30_09/02/1965	184.0	* 0.634	85.630000	[N/A]	1	1	1
02:30_09/02/1965	185.0	* 0.634	85.630000	[N/A]	1	1	1
01:30_11/03/1965	186.0	* 0.634	95.415000	[N/A]	1	1	1
02:30_11/03/1965	187.0	* 0.634	105.202000	[N/A]	1	1	1
01:30_11/04/1965	188.0	* 0.570	32.783000	[N/A]	1	1	1
02:30_11/04/1965	189.0	* 0.570	33.028000	[N/A]	1	1	1
01:30_22/04/1965	190.0	* 0.607	63.610000	[N/A]	1	1	1
01:30_26/05/1965	191.0	* 0.591	56.271000	[N/A]	1	1	1
02:30_26/05/1965	192.0	* 0.591	58.717000	[N/A]	1	1	1
01:30_07/07/1965	193.0	* 0.652	114.988000	[N/A]	1	1	1
02:30_07/07/1965	194.0	* 0.652	110.096000	[N/A]	1	1	1
01:30_17/08/1965	195.0	* 0.622	107.648000	[N/A]	1	1	1
02:30_17/08/1965	196.0	* 0.622	107.648000	[N/A]	1	1	1
01:30_16/02/1966	197.0	* 0.466	3.596000	[N/A]	1	1	1
02:30_16/02/1966	198.0	* 0.466	3.889000	[N/A]	1	1	1
01:30_07/03/1966	199.0	* 0.439	1.540000	[N/A]	1	1	1
01:30_13/04/1966	200.0	* 0.457	3.572000	[N/A]	1	1	1
02:30_13/04/1966	201.0	* 0.457	3.596000	[N/A]	1	1	1
01:30_20/05/1966	202.0	* 0.485	3.938000	[N/A]	1	1	1
02:30_20/05/1966	203.0	* 0.485	3.816000	[N/A]	1	1	1
01:30_23/06/1966	204.0	* 0.594	63.610000	[N/A]	1	1	1
02:30_23/06/1966	205.0	* 0.594	61.163000	[N/A]	1	1	1
01:30_04/08/1966	206.0	* 0.543	25.689000	[N/A]	1	1	1
02:30_04/08/1966	207.0	* 0.543	24.709000	[N/A]	1	1	1
01:30_02/09/1966	208.0	* 0.948	910.125000	[N/A]	1	1	1
01:30_11/10/1966	209.0	* 0.680	161.473000	[N/A]	1	1	1
01:30_10/11/1966	210.0	* 2.417	9541.636000	[N/A]	1	1	1
01:30_09/12/1966	211.0	* 0.826	543.139000	[N/A]	1	1	1
02:30_09/12/1966	212.0	* 0.826	540.692000	[N/A]	1	1	1
03:00_09/12/1966	213.0	* 0.799	489.314000	[N/A]	1	1	1
01:30_09/12/1966	214.0	* 0.799	506.441000	[N/A]	1	1	1
01:30_11/01/1967	215.0	* 0.762	371.879000	[N/A]	1	1	1
02:30_11/01/1967	216.0	* 0.762	381.666000	[N/A]	1	1	1
01:30_06/02/1967	217.0	* 0.600	88.077000	[N/A]	1	1	1
01:30_10/02/1967	218.0	* 0.564	57.250000	[N/A]	1	1	1
02:30_10/02/1967	219.0	* 0.564	58.717000	[N/A]	1	1	1
01:30_21/04/1967	220.0	* 0.680	198.172000	[N/A]	1	1	1

\* after gauge number indicates Weighted Mean Gauge Height method

Site 419022 NAMOI RIVER AT MANILLA RAILWAY BRIDGE			
VarFrom	100.00	Stream Water Level	
VarTo	141.00	Stream Discharge	
Period 01/10/1939 - 30/09/2011			
Date	Number	Stage	
01:30_17/05/1967	221.0	* 0.619	
01:30_07/06/1967	222.0	* 0.616	102.756000 [N/A] 1 .....   ..... 100%
01:30_11/07/1967	223.0	* 0.768	107.648000 [N/A] 1 .....   ..... 100%
01:30_05/09/1967	224.0	* 0.728	413.470000 [N/A] 1 .....   ..... 100%
02:30_05/09/1967	225.0	* 0.728	288.695000 [N/A] 1 .....   ..... 100%
01:30_21/09/1967	226.0	* 0.710	300.929000 [N/A] 1 .....   ..... 100%
02:30_21/09/1967	227.0	* 0.710	271.569000 [N/A] 1 .....   ..... 100%
01:30_24/10/1967	228.0	* 1.631	244.658000 [N/A] 1 .....   ..... 100%
02:30_24/10/1967	229.0	* 1.750	5377.566000 [N/A] 1 .....   ..... 100%
01:30_13/12/1967	230.0	* 0.585	5340.871000 [N/A] 1 .....   ..... 100%
02:30_13/12/1967	231.0	* 0.555	78.290000 [N/A] 1 .....   ..... 100%
01:30_05/01/1968	232.0	* 0.472	73.396000 [N/A] 1 .....   ..... 100%
01:30_13/02/1968	233.0	* 0.902	44.038000 [N/A] 1 .....   ..... 100%
01:30_13/03/1968	234.0	* 0.841	374.325000 [N/A] 1 .....   ..... 100%
02:30_13/03/1968	235.0	* 0.838	217.745000 [N/A] 1 .....   ..... 100%
01:30_08/05/1968	236.0	* 0.671	207.959000 [N/A] 1 .....   ..... 100%
02:30_08/05/1968	237.0	* 0.671	29.114000 [N/A] 1 .....   ..... 100%
01:30_08/07/1968	238.0	* 0.838	29.847000 [N/A] 1 .....   ..... 100%
02:30_08/07/1968	239.0	* 0.838	259.336000 [N/A] 1 .....   ..... 100%
01:30_21/08/1968	240.0	* 1.423	259.336000 [N/A] 1 .....   ..... 100%
01:30_30/10/1968	241.0	* 0.896	2578.688000 [N/A] 1 .....   ..... 100%
01:30_19/11/1968	242.0	* 0.792	386.558000 [N/A] 1 .....   ..... 100%
02:30_19/11/1968	243.0	* 0.768	266.677000 [N/A] 1 .....   ..... 100%
01:30_02/02/1969	244.0	* 0.847	227.531000 [N/A] 1 .....   ..... 100%
02:30_02/02/1969	245.0	* 0.850	291.141000 [N/A] 1 .....   ..... 100%
01:30_12/02/1969	246.0	* 0.838	293.589000 [N/A] 1 .....   ..... 100%
01:30_13/03/1969	247.0	* 0.610	362.092000 [N/A] 1 .....   ..... 100%
02:30_13/03/1969	248.0	* 0.610	11.328000 [N/A] 1 .....   ..... 100%
01:30_07/05/1969	249.0	* 0.680	11.033000 [N/A] 1 .....   ..... 100%
01:30_09/07/1969	250.0	* 0.887	61.163000 [N/A] 1 .....   ..... 100%
02:30_09/07/1969	251.0	* 0.887	318.055000 [N/A] 1 .....   ..... 100%
01:30_11/08/1969	252.0	* 0.920	318.055000 [N/A] 1 .....   ..... 100%
02:30_11/08/1969	253.0	* 0.920	335.180000 [N/A] 1 .....   ..... 100%
01:30_08/09/1969	254.0	* 2.006	318.055000 [N/A] 1 .....   ..... 100%
01:30_23/09/1969	255.0	* 1.076	5928.045000 [N/A] 1 .....   ..... 100%
02:30_23/09/1969	256.0	* 1.076	638.555000 [N/A] 1 .....   ..... 100%
01:30_12/12/1969	257.0	* 1.116	631.216000 [N/A] 1 .....   ..... 100%
01:30_30/12/1969	258.0	* 0.908	1206.161000 [N/A] 1 .....   ..... 100%
01:30_13/03/1970	259.0	* 0.942	391.452000 [N/A] 1 .....   ..... 100%
01:30_16/03/1970	260.0	* 0.905	364.540000 [N/A] 1 .....   ..... 100%
01:30_17/03/1970	261.0	* 0.957	166.367000 [N/A] 1 .....   ..... 100%
01:30_16/04/1970	262.0	* 0.832	286.248000 [N/A] 1 .....   ..... 100%
01:30_05/05/1970	263.0	* 0.863	95.415000 [N/A] 1 .....   ..... 100%
01:30_09/06/1970	264.0	* 0.872	1.395000 [N/A] 1 .....   ..... 100%
01:30_14/07/1970	265.0	* 0.826	130.647000 [N/A] 1 .....   ..... 100%
01:30_13/08/1970	266.0	* 0.768	97.373000 [N/A] 1 .....   ..... 100%
01:30_08/09/1970	267.0	* 0.799	54.314000 [N/A] 1 .....   ..... 100%
01:30_29/09/1970	268.0	* 1.865	95.660000 [N/A] 1 .....   ..... 100%
01:30_12/10/1970	269.0	* 0.957	6052.820000 [N/A] 1 .....   ..... 100%
01:30_03/11/1970	270.0	* 0.856	626.322000 [N/A] 1 .....   ..... 100%
01:30_15/12/1970	271.0	* 1.250	340.074000 [N/A] 1 .....   ..... 100%
01:30_24/03/1971	272.0	* 0.939	1844.717000 [N/A] 1 .....   ..... 100%
01:30_10/05/1971	273.0	* 0.856	978.629000 [N/A] 1 .....   ..... 100%
02:30_10/05/1971	274.0	* 0.856	437.936000 [N/A] 1 .....   ..... 100%
01:30_09/06/1971	275.0	* 0.823	440.384000 [N/A] 1 .....   ..... 100%
			396.345000 [N/A] 1 .....   ..... 100%

\* after gauge number indicates Weighted Mean Gauge Height method



## Site 419022 NAMOI RIVER AT MANILLA RAILWAY BRIDGE

VarFrom 100.00 Stream Water Level  
 VarTo 141.00 Stream Discharge  
 Period 01/10/1939 - 30/09/2011

Date	Number	Stage	Flow	Deviation	Rating	-100% Deviation	100%
01:30_16/07/1971	276.0	* 0.789	396.345000	[N/A]	1	.....	.....
01:30_11/08/1971	277.0	* 1.460	3351.807000	[N/A]	1	.....	.....
01:30_22/09/1971	278.0	* 2.637	12820.046000	[N/A]	1	.....	.....
01:30_28/09/1971	279.0	* 1.902	6825.936000	[N/A]	1	.....	.....
01:30_28/10/1971	280.0	* 0.789	393.899000	[N/A]	1	.....	.....
02:30_28/10/1971	281.0	* 0.792	430.596000	[N/A]	1	.....	.....
01:30_02/12/1971	282.0	* 1.058	1093.617000	[N/A]	1	.....	.....
01:30_03/02/1972	283.0	* 0.719	264.229000	[N/A]	1	.....	.....
01:30_29/02/1972	284.0	* 0.658	154.134000	[N/A]	1	.....	.....
01:30_22/03/1972	285.0	* 0.634	130.158000	[N/A]	1	.....	.....
01:30_10/04/1972	286.0	* 0.786	437.936000	[N/A]	1	.....	.....
01:30_18/05/1972	287.0	* 0.792	149.240000	[N/A]	1	.....	.....
01:30_30/05/1972	288.0	* 0.820	122.329000	[N/A]	1	.....	.....
01:30_29/06/1972	289.0	* 0.832	115.967000	[N/A]	1	.....	.....
01:30_12/07/1972	290.0	* 0.789	118.170000	[N/A]	1	.....	.....
01:30_06/09/1972	291.0	* 0.762	77.800000	[N/A]	1	.....	.....
01:30_28/09/1972	292.0	* 0.716	43.059000	[N/A]	1	.....	.....
01:30_04/10/1972	293.0	* 0.792	86.363000	[N/A]	1	.....	.....
01:30_11/10/1972	294.0	* 0.881	182.025000	[N/A]	1	.....	.....
01:30_14/12/1972	295.0	* 1.024	606.751000	[N/A]	1	.....	.....
01:30_24/01/1973	296.0	* 0.768	124.775000	[N/A]	1	.....	.....
01:30_14/03/1973	297.0	* 1.344	2407.427000	[N/A]	1	.....	.....
01:30_22/03/1973	298.0	* 0.713	92.969000	[N/A]	1	.....	.....
01:30_16/04/1973	299.0	* 0.686	61.163000	[N/A]	1	.....	.....
01:30_08/05/1973	300.0	* 0.735	116.701000	[N/A]	1	.....	.....
01:30_28/05/1973	301.0	* 0.695	58.228000	-11.40	115	.....*	.....
01:30_18/06/1973	302.0	* 0.893	371.879000	-0.01	115	.....*	.....
01:30_16/07/1973	303.0	* 0.930	437.936000	-5.68	115	.....*	.....
01:30_03/08/1973	304.0	* 0.875	337.626000	2.30	115	.....*	.....
01:30_14/08/1973	305.0	* 0.805	203.066000	1.53	115	.....*	.....
01:30_18/09/1973	306.0	* 1.003	692.381000	0.15	115	.....*	.....
01:30_26/10/1973	307.0	* 0.911	753.544000	-1.63	120	.....*	.....
01:30_09/11/1973	308.0	* 1.003	1017.775000	-5.84	120	.....*	.....
01:30_19/03/1974	309.0	* 0.847	259.336000	-4.39	125	.....*	.....
01:30_17/04/1974	310.0	* 0.814	207.225000	-3.08	125	.....*	.....
01:30_28/05/1974	311.0	* 0.850	276.462000	-0.15	125	.....*	.....
01:30_20/08/1974	312.0	* 0.877	308.000000	-8.02	125	.....*	.....
01:30_10/10/1974	313.0	* 1.056	897.999000	0.72	125	.....*	.....
01:30_27/11/1974	314.0	* 0.891	390.000000	6.06	125	.....*	.....
01:30_06/02/1975	315.0	* 0.538	1.1129000	-26.39	135	.....*	.....
01:30_11/03/1975	316.0	* 0.927	520.000000	-0.40	135	.....*	.....
01:30_08/04/1975	317.0	* 0.938	440.000000	-3.01	140	.....*	.....
01:30_07/05/1975	318.0	* 0.837	187.000000	-5.27	140	.....*	.....
01:30_25/06/1975	319.0	* 2.259	985.000000	-90.10	140	*.....	.....
01:30_08/08/1975	320.0	* 0.910	385.000000	2.50	140	.....*	.....
01:30_24/09/1975	321.0	* 0.945	456.999000	-3.72	140	.....*	.....
01:30_29/10/1975	322.0	* 1.113	1088.998000	-2.31	140	.....*	.....
01:30_03/12/1975	323.0	* 0.880	299.000000	1.22	140	.....*	.....
01:30_02/06/1976	324.0	* 0.910	373.000000	-0.70	140	.....*	.....
01:30_11/08/1976	325.0	* 0.926	625.000000	-0.77	145	.....*	.....
01:30_27/10/1976	326.0	* 0.987	807.000000	-5.19	145	.....*	.....
01:30_25/11/1976	327.0	* 0.975	822.000000	1.81	145	.....*	.....
01:30_11/01/1977	328.0	* 0.740	59.599000	-49.26	145	.....*	.....
01:30_16/02/1977	329.0	* 0.696	68.800000	2.04	145	.....*	.....
01:30_06/04/1977	330.0	* 0.924	600.000000	-3.64	145	.....*	.....

\* after gauge number indicates Weighted Mean Gauge Height method

## Site 419022 NAMOI RIVER AT MANILLA RAILWAY BRIDGE

VarFrom 100.00 Stream Water Level  
 VarTo 141.00 Stream Discharge  
 Period 01/10/1939 - 30/09/2011

Date	Number	Stage	Flow	Deviation	Rating	-100% Deviation	100%
01:30_03/05/1977	331.0	*	429.000000	-7.01	150	.....*	.....
01:30_09/06/1977	332.0	*	1799.998000	0.80	150	.....*	.....
01:30_20/07/1977	333.0	*	1270.998000	0.95	150	.....*	.....
01:30_24/08/1977	334.0	*	684.000000	7.18	150	.....*	.....
01:30_28/09/1977	335.0	*	512.000000	-1.37	150	.....*	.....
01:30_10/11/1977	336.0	*	195.999000	17.12	150	.....*	.....
01:30_08/12/1977	337.0	*	155.000000	-3.74	155	.....*	.....
01:30_01/03/1978	338.0	*	306.000000	-0.62	155	.....*	.....
01:30_12/04/1978	339.0	*	398.000000	-2.09	155	.....*	.....
01:30_10/05/1978	340.0	*	225.000000	-25.91	155	.....*	.....
01:30_21/06/1978	341.0	*	4719.995000	-0.13	160	.....*	.....
01:30_17/08/1978	342.0	*	1429.998000	4.35	160	.....*	.....
01:30_27/09/1978	343.0	*	1432.998000	7.02	160	.....*	.....
01:30_25/10/1978	344.0	*	638.000000	-17.08	160	.....*	.....
01:30_07/11/1978	345.0	*	3839.998000	-2.18	160	.....*	.....
01:30_28/11/1978	346.0	*	615.000000	-7.36	160	.....*	.....
01:30_23/01/1979	347.0	*	502.000000	5.24	160	.....*	.....
01:30_28/02/1979	348.0	*	117.000000	-7.28	165	.....*	.....
01:30_18/04/1979	349.0	*	212.000000	5.85	165	.....*	.....
01:30_30/05/1979	350.0	*	306.000000	-7.12	165	.....*	.....
01:30_11/07/1979	351.0	*	313.000000	-6.41	165	.....*	.....
09:00_21/08/1979	352.0	*	354.000000	-9.79	165	.....*	.....
16:00_31/10/1979	353.0	*	810.000000	-2.53	165	.....*	.....
16:00_05/12/1979	354.0	*	283.000000	13.64	165	.....*	.....
10:00_16/01/1980	355.0	*	142.000000	10.18	165	.....*	.....
10:50_20/02/1980	356.0	*	17.399000	-18.15	165	.....*	.....
15:45_19/03/1980	357.0	*	8.899000	-5.33	165	.....*	.....
13:00_22/04/1980	358.0	*	1.799000	-21.78	165	.....*	.....
15:30_29/05/1980	359.0	*	116.000000	8.41	165	.....*	.....
13:00_11/06/1980	360.0	*	112.000000	2.33	165	.....*	.....
14:00_09/07/1980	361.0	*	840.000000	-4.82	165	.....*	.....
15:30_12/08/1980	362.0	*	164.000000	1.86	165	.....*	.....
16:00_17/09/1980	363.0	*	40.799000	13.30	165	.....*	.....
09:30_22/10/1980	364.0	*	4.099000	43.49	165	.....*	.....
01:30_27/11/1980	365.0	*	-999.000000	[N/A]	165	.....*	.....
10:30_15/01/1981	366.0	*	0.689000	-77.77	165	.....*	.....
15:00_25/02/1981	367.0	*	49.999000	3.55	165	.....*	.....
09:00_23/04/1981	368.0	*	2.080000	19.70	165	.....*	.....
12:00_10/06/1981	369.0	*	1629.998000	-9.18	165	.....*	.....
16:00_21/07/1981	370.0	*	375.000000	-9.64	165	.....*	.....
14:00_02/09/1981	371.0	*	401.000000	-6.39	165	.....*	.....
16:00_27/10/1981	372.0	*	547.000000	-4.99	166	.....*	.....
10:00_15/12/1981	373.0	*	34.299000	4.00	166	.....*	.....
16:00_03/02/1982	374.0	*	2.220000	-2.15	166	.....*	.....
12:00_18/03/1982	375.0	*	654.000000	5.48	166	.....*	.....
16:00_06/07/1982	376.0	*	81.099000	3.34	166	.....*	.....
12:00_14/09/1982	377.0	*	13.800000	-5.47	166	.....*	.....
17:00_08/11/1982	378.0	*	10.800000	2.28	166	.....*	.....
12:00_05/01/1983	379.0	*	241.000000	0.89	166	.....*	.....
12:00_01/03/1983	380.0	*	223.000000	0.31	167	.....*	.....
12:00_16/03/1983	381.0	*	0.300000	115.17	167	.....*	.....
17:00_11/04/1983	382.0	*	4.510000	0.72	167	.....*	.....
11:00_09/06/1983	383.0	*	1349.998000	2.21	167	.....*	.....
10:00_22/07/1983	384.0	*	900.000000	2.39	167	.....*	.....
16:00_19/09/1983	385.0	*	1569.998000	-3.68	167	.....*	.....

\* after gauge number indicates Weighted Mean Gauge Height method

## Site 419022 NAMOI RIVER AT MANILLA RAILWAY BRIDGE

VarFrom 100.00 Stream Water Level  
VarTo 141.00 Stream Discharge  
Period 01/10/1939 - 30/09/2011

Date	Number	Stage	Flow	Deviation	Rating	-100% Deviation	100%
12:00_15/11/1983	386.0	*	323.000000	0.90	167	.....*	.....
14:00_09/07/1984	387.0	*	420.000000	-11.58	168	.....*	.....
10:00_01/08/1984	388.0	*	5939.995000	-0.58	169	.....*	.....
12:00_04/09/1984	389.0	*	933.000000	3.09	169	.....*	.....
12:00_14/02/1985	390.0	*	92.599000	-7.90	200	.....*	.....
10:30_10/05/1985	391.0	*	398.000000	-4.17	200	.....*	.....
09:30_12/07/1985	392.0	*	6409.996000	25.74	200	.....*	.....
13:30_12/09/1985	393.0	*	2179.999000	6.53	200	.....*	.....
12:10_05/12/1985	394.0	*	748.000000	1.13	200	.....*	.....
14:15_03/02/1986	395.0	*	109.000000	8.43	201	.....*	.....
13:00_20/02/1986	396.0	*	40.400000	-7.82	201	.....*	.....
15:30_17/03/1986	397.0	*	3.330000	-2.29	201	.....*	.....
14:30_29/04/1986	398.0	*	2.680000	15.64	202	.....*	.....
15:30_02/06/1986	399.0	*	59.500000	-0.12	202	.....*	.....
08:40_01/07/1986	400.0	*	30.000000	-5.81	203	.....*	.....
15:45_11/08/1986	401.0	*	1728.998000	-3.17	204	.....*	.....
12:30_23/10/1986	402.0	*	312.000000	0.58	204	.....*	.....
11:15_10/12/1986	403.0	*	90.400000	6.35	204	.....*	.....
11:30_23/01/1987	404.0	*	44.200000	-8.40	205	.....*	.....
14:35_09/04/1987	405.0	*	37.000000	0.55	205	.....*	.....
10:00_12/06/1987	406.0	*	58.500000	9.03	205	.....*	.....
09:15_22/07/1987	407.0	*	81.899000	-10.00	205	.....*	.....
09:30_08/09/1987	408.0	*	2329.999000	-7.80	205	.....*	.....
12:50_10/11/1987	409.0	*	125.000000	0.45	205	.....*	.....
00:00_05/01/1988	410.0	*	1590.000000	-8.49	205	.....*	.....
00:00_01/03/1988	411.0	*	136.000000	-0.55	210	.....*	.....
00:00_10/05/1988	412.0	*	284.000000	7.84	210	.....*	.....
00:00_27/06/1988	413.0	*	280.000000	-0.40	211	.....*	.....
00:00_23/08/1988	414.0	*	458.000000	-3.66	211	.....*	.....
00:00_07/10/1988	415.0	*	684.000000	-3.43	211	.....*	.....
00:00_24/11/1988	416.0	*	341.000000	-0.39	211	.....*	.....
00:00_19/01/1989	417.0	*	81.600000	0.57	211	.....*	.....
00:00_28/02/1989	418.0	*	52.000000	3.97	212	.....*	.....
00:00_18/04/1989	419.0	*	430.000000	-3.18	212	.....*	.....
00:00_05/06/1989	420.0	*	290.000000	3.87	212	.....*	.....
00:00_02/08/1989	421.0	*	10500.000000	2.74	212	.....*	.....
00:00_26/09/1989	422.0	*	212.000000	28.07	212	.....*	.....
00:00_27/11/1989	423.0	*	212.000000	6.71	212	.....*	.....
00:00_08/02/1990	424.0	*	1390.000000	-3.41	212	.....*	.....
00:00_27/03/1990	425.0	*	68.200000	-11.58	212	.....*	.....
00:00_23/05/1990	426.0	*	542.000000	-11.29	212	.....*	.....
10:00_25/07/1990	427.0	*	1180.000000	-3.44	212	.....*	.....
10:50_02/08/1990	428.0	*	9830.000000	4.68	212	.....*	.....
10:40_05/08/1990	429.0	*	18000.000000	4.79	212	.....*	.....
11:20_14/09/1990	430.0	*	1520.000000	-0.05	212	.....*	.....
11:40_24/09/1990	431.0	*	1150.000000	-4.80	212	.....*	.....
11:00_21/11/1990	432.0	*	86.800000	-7.96	212	.....*	.....
13:40_30/01/1991	433.0	*	1920.000000	[N/A]	212	.....*	.....
16:45_12/03/1991	434.0	*	83.500000	-6.72	212	.....*	.....
16:40_14/05/1991	435.0	*	36.600000	-0.87	213	.....*	.....
10:50_09/07/1991	436.0	*	114.000000	4.62	213	.....*	.....
16:35_02/09/1991	437.0	*	97.900000	2.03	213	.....*	.....
15:15_05/11/1991	438.0	*	83.800000	-0.24	213	.....*	.....
08:45_14/11/1991	439.0	*	4750.000000	3.61	213	.....*	.....
07:40_15/11/1991	440.0	*	2950.000000	-4.63	213	.....*	.....

\* after gauge number indicates Weighted Mean Gauge Height method

## Site 419022 NAMOI RIVER AT MANILLA RAILWAY BRIDGE

VarFrom 100.00 Stream Water Level

VarTo 141.00 Stream Discharge

Period 01/10/1939 - 30/09/2011

Date	Number	Stage	Flow	Deviation	Rating	-100% Deviation	100%
14:45_26/11/1991	441.0	0.833	469.000000	-5.13	213	.....*	.....
09:40_07/01/1992	442.0	1.165	1400.000000	-4.70	213	.....*	.....
14:30_10/03/1992	443.0	0.670	216.000000	16.29	213	.....	.....*
13:45_12/05/1992	444.0	0.597	93.300000	12.92	214	.....	.....*
15:00_08/07/1992	445.0	0.665	177.000000	-1.01	214	.....*	.....
17:00_08/09/1992	446.0	0.786	365.000000	-6.66	214	.....*	.....
11:15_16/11/1992	447.0	0.704	236.000000	-0.33	214	.....*	.....
11:30_20/01/1993	448.0	0.557	47.400000	5.66	214	.....	.....*
08:30_17/03/1993	449.0	0.488	11.600000	-0.27	214	.....*	.....
13:20_18/03/1993	450.0	0.527	26.500000	-0.05	214	.....*	.....
11:20_17/05/1993	451.0	0.555	43.100000	-0.85	214	.....*	.....
15:35_20/07/1993	452.0	0.869	492.000000	-3.64	215	.....*	.....
13:55_20/09/1993	453.0	0.807	377.000000	0.84	215	.....*	.....
12:10_16/11/1993	454.0	0.667	131.000000	-7.34	215	.....*	.....
15:30_17/11/1993	455.0	0.669	149.000000	3.16	215	.....*	.....
16:30_15/03/1994	456.0	0.602	78.100000	16.97	215	.....	.....*
14:20_12/05/1994	457.0	0.541	28.600000	2.63	215	.....*	.....
08:45_14/07/1994	458.0	0.575	43.500000	-6.82	215	.....*	.....
08:00_15/09/1994	459.0	0.520	21.800000	-5.22	216	.....*	.....
13:10_21/11/1994	460.0	0.523	26.100000	6.66	216	.....	.....*
07:50_18/01/1995	461.0	0.510	24.800000	31.27	216	.....	.....*
08:20_15/03/1995	462.0	0.734	262.000000	-9.27	216	.....*	.....
07:45_08/06/1995	463.0	0.598	84.200000	0.55	216	.....*	.....
14:40_08/08/1995	464.0	0.612	103.000000	1.34	216	.....*	.....
09:00_15/09/1995	465.0	0.681	195.000000	-3.21	216	.....*	.....
14:30_09/10/1995	466.0	1.007	873.000000	-8.02	216	.....*	.....
16:20_17/10/1995	467.0	0.716	238.000000	-7.15	216	.....*	.....
09:00_24/11/1995	468.0	1.160	1220.000000	-0.34	217	.....*	.....
11:30_22/02/1996	469.0	0.925	535.000000	-0.15	217	.....*	.....
12:45_12/03/1996	470.0	0.736	170.000000	-2.38	217	.....*	.....
15:15_29/04/1996	471.0	0.653	75.600000	-4.04	217	.....*	.....
11:40_10/05/1996	472.0	1.074	965.000000	3.41	217	.....*	.....
09:05_11/07/1996	473.0	0.845	371.000000	-0.85	217	.....*	.....
09:00_05/09/1996	474.0	1.725	4460.000000	5.37	218	.....	.....*
15:30_17/10/1996	475.0	0.985	802.000000	8.42	218	.....	.....*
11:20_28/10/1996	476.0	0.786	323.000000	1.45	218	.....*	.....
11:40_20/12/1996	477.0	2.150	8070.000000	1.23	218	.....*	.....
13:10_06/03/1997	478.0	1.135	1250.000000	5.34	218	.....	.....*
09:00_18/04/1997	479.0	0.707	146.000000	-22.85	218	.....*	.....
12:45_14/05/1997	480.0	0.700	144.000000	-20.01	218	.....*	.....
09:40_25/07/1997	481.0	0.652	120.000000	-3.07	218	.....*	.....
08:25_11/09/1997	482.0	0.693	167.000000	-2.36	218	.....*	.....
12:10_20/11/1997	483.0	0.695	177.000000	1.97	218	.....*	.....
06:20_17/12/1997	484.0	0.670	147.000000	2.61	218	.....*	.....
11:05_16/01/1998	485.0	0.560	41.200000	0.49	218	.....*	.....
11:20_25/02/1998	486.0	0.585	58.000000	-1.33	218	.....*	.....
13:20_26/03/1998	487.0	0.537	29.400000	-1.05	218	.....*	.....
13:10_21/04/1998	488.0	0.533	27.300000	-2.25	218	.....*	.....
08:00_14/05/1998	489.0	0.636	109.000000	1.51	218	.....*	.....
08:30_16/07/1998	490.0	0.926	578.000000	-3.02	218	.....*	.....
12:00_06/11/1998	491.0	1.115	866.000000	2.47	219	.....*	.....
10:20_26/11/1998	492.0	1.112	834.000000	-0.24	219	.....*	.....
13:00_28/01/1999	493.0	0.756	132.000000	-1.57	219	.....*	.....
12:30_18/03/1999	494.0	0.763	149.000000	5.73	219	.....	.....*
11:30_20/05/1999	495.0	0.764	138.000000	-2.75	219	.....*	.....

\* after gauge number indicates Weighted Mean Gauge Height method

Site 419022 NAMOI RIVER AT MANILLA RAILWAY BRIDGE							
VarFrom	100.00	Stream Water Level		Flow	Deviation	Rating	-100% Deviation 100%
VarTo	141.00	Stream Discharge					
Period 01/10/1939 - 30/09/2011							
Date	Number	Stage					
13:45_11/06/1999	496.0	0.818	188.000000	-6.21	219	.....*	.....
16:20_08/07/1999	497.0	0.940	316.000000	-1.52	220	.....*	.....
08:25_15/07/1999	498.0	0.878	215.000000	-5.04	220	.....*	.....
16:30_13/09/1999	499.0	0.928	286.000000	-5.01	220	.....*	.....
10:15_09/11/1999	500.0	1.975	6070.000000	-3.17	220	.....*	.....
13:05_18/01/2000	501.0	0.758	106.000000	1.46	220	.....*	.....
08:55_08/03/2000	502.0	0.740	85.400000	-6.52	220	.....*	.....
09:05_24/05/2000	503.0	0.751	96.600000	-2.67	220	.....*	.....
08:40_14/07/2000	504.0	0.840	198.000000	10.61	220	.....*	.....
12:35_14/09/2000	505.0	1.071	642.000000	-1.07	220	.....*	.....
12:30_15/09/2000	506.0	1.138	889.000000	1.53	220	.....*	.....
07:05_10/11/2000	507.0	0.967	382.000000	2.24	220	.....*	.....
07:20_11/01/2001	508.0	0.777	134.000000	12.26	220	.....*	.....
08:00_06/04/2001	509.0	0.968	450.000000	2.05	221	.....*	.....
10:00_23/05/2001	510.0	0.850	220.000000	0.00	221	.....*	.....
11:35_08/08/2001	511.0	0.885	271.000000	-1.25	221	.....*	.....
10:30_18/10/2001	512.0	1.072	731.000000	2.42	221	.....*	.....
06:50_14/12/2001	513.0	0.945	423.000000	8.69	221	.....*	.....
08:30_22/02/2002	514.0	0.770	135.000000	4.32	221	.....*	.....
08:45_24/04/2002	515.0	0.690	62.100000	0.53	221	.....*	.....
16:00_19/06/2002	516.0	0.734	89.500000	-7.23	221	.....*	.....
17:10_14/08/2002	517.0	1.280	1440.000000	0.23	221	.....*	.....
13:45_16/08/2002	518.0	1.633	3440.000000	0.14	221	.....*	.....
13:00_20/08/2002	519.0	1.520	2860.000000	6.01	221	.....*	.....
10:40_05/09/2002	520.0	1.278	1430.000000	0.10	221	.....*	.....
15:35_23/10/2002	521.0	1.359	1690.000000	-5.79	221	.....*	.....
12:55_07/11/2002	522.0	1.434	2220.000000	1.49	221	.....*	.....
13:00_07/11/2002	523.0	1.434	2230.000000	1.95	221	.....*	.....
15:10_12/12/2002	524.0	1.307	1520.000000	-2.01	221	.....*	.....
13:30_24/01/2003	525.0	1.184	1020.000000	-4.93	222	.....*	.....
12:45_30/01/2003	526.0	0.721	85.200000	-0.70	222	.....*	.....
15:15_11/02/2003	527.0	0.670	34.400000	0.55	223	.....*	.....
10:30_12/02/2003	528.0	0.652	23.500000	-6.50	223	.....*	.....
15:40_29/04/2003	529.0	0.833	200.000000	2.90	223	.....*	.....
11:05_03/07/2003	530.0	0.782	124.000000	-2.31	223	.....*	.....
13:55_12/08/2003	531.0	0.742	84.140000	-0.52	223	.....*	.....
08:00_14/08/2003	532.0	0.758	100.057000	0.10	223	.....*	.....
15:25_28/10/2003	533.0	0.784	124.312000	-3.91	223	.....*	.....
06:50_16/12/2003	534.0	0.738	93.268000	-6.69	224	.....*	.....
10:20_24/02/2004	535.0	0.822	207.906000	-1.20	225	.....*	.....
16:05_20/04/2004	536.0	0.700	68.699000	-0.34	225	.....*	.....
09:15_22/04/2004	537.0	0.676	43.888000	-3.40	225	.....*	.....
15:30_07/06/2004	538.0	0.738	104.841000	-5.91	225	.....*	.....
15:30_11/08/2004	539.0	0.897	304.235000	-3.48	225	.....*	.....
15:35_11/08/2004	540.0	0.897	308.055000	-2.27	225	.....*	.....
12:30_22/10/2004	541.0	0.863	261.361000	-0.82	225	.....*	.....
15:05_26/10/2004	542.0	1.118	814.221000	-4.74	225	.....*	.....
09:55_28/10/2004	543.0	0.989	507.720000	-0.12	225	.....*	.....
16:45_08/12/2004	544.0	0.818	197.219000	-4.12	225	.....*	.....
16:50_08/12/2004	545.0	0.818	191.328000	-6.98	225	.....*	.....
07:20_17/02/2005	546.0	0.655	40.850000	-0.57	226	.....*	.....
06:45_24/02/2005	547.0	0.672	48.631000	-6.70	226	.....*	.....
09:30_18/04/2005	548.0	0.662	41.888000	-7.53	226	.....*	.....
10:15_06/06/2005	549.0	0.637	28.007000	-8.91	226	.....*	.....
15:15_10/08/2005	550.0	0.929	346.953000	1.90	226	.....*	.....

\* after gauge number indicates Weighted Mean Gauge Height method

## Site 419022 NAMOI RIVER AT MANILLA RAILWAY BRIDGE

VarFrom 100.00 Stream Water Level  
 VarTo 141.00 Stream Discharge  
 Period 01/10/1939 - 30/09/2011

Date	Number	Stage	Flow	Deviation	Rating	-100% Deviation	100%
08:30_11/08/2005	551.0	0.907	310.438000	2.87	226	.....*	.....
15:15_12/10/2005	552.0	0.842	198.973000	-1.14	226	.....*	.....
11:00_27/10/2005	553.0	0.820	182.755000	5.43	226	..... *	.....
14:50_20/12/2005	554.0	0.902	259.185000	0.41	227	.....*	.....
14:55_20/12/2005	555.0	0.902	254.997000	-1.21	227	.....*	.....
08:30_20/02/2006	556.0	0.710	65.489000	8.00	227	..... *	.....
08:30_20/03/2006	557.0	0.661	29.034000	-6.58	227	..... *	.....
16:45_11/04/2006	558.0	0.715	65.545000	2.76	227	.....*	.....
16:50_11/04/2006	559.0	0.715	63.797000	0.02	227	.....*	.....
12:45_20/06/2006	560.0	0.788	130.628000	9.54	227	..... *	.....
12:50_20/06/2006	561.0	0.788	125.443000	5.20	227	..... *	.....
13:25_14/08/2006	562.0	0.717	63.879000	-1.82	227	.....*	.....
10:40_17/10/2006	563.0	0.612	15.304000	6.84	227	..... *	.....
10:45_05/12/2006	564.0	1.636	3022.174000	-1.59	227	.....*	.....
10:50_05/12/2006	565.0	1.636	2859.837000	-6.88	227	..... *	.....
10:44_15/01/2007	566.0	0.694	55.261000	-3.93	228	.....*	.....
08:20_30/01/2007	567.0	0.672	46.917000	6.07	228	..... *	.....
13:10_10/04/2007	568.0	0.630	23.293000	-2.62	228	.....*	.....
14:17_21/06/2007	569.0	0.723	71.648000	-8.41	228	..... *	.....
14:19_21/06/2007	570.0	0.723	74.453000	-4.83	228	.....*	.....
07:32_17/08/2007	571.0	0.703	65.256000	2.70	228	.....*	.....
07:34_17/08/2007	572.0	0.703	66.344000	4.41	228	.....*	.....
06:43_19/10/2007	573.0	0.646	28.020000	-8.88	228	..... *	.....
08:00_15/11/2007	574.0	0.895	258.328000	-9.28	228	.....*	.....
08:55_12/12/2007	575.0	1.223	1076.673000	-4.24	228	.....*	.....
09:00_12/12/2007	576.0	1.223	1111.366000	-1.16	228	.....*	.....
14:31_27/02/2008	577.0	0.893	210.396000	-8.93	229	..... *	.....
14:35_27/02/2008	578.0	0.893	213.292000	-7.67	229	..... *	.....
12:08_16/04/2008	579.0	0.655	26.378000	-7.93	230	..... *	.....
12:12_16/04/2008	580.0	0.658	27.411000	-8.59	230	..... *	.....
08:51_20/06/2008	581.0	0.892	293.262000	2.22	230	.....*	.....
08:57_20/06/2008	582.0	0.892	282.734000	-1.45	230	.....*	.....
07:55_08/07/2008	583.0	0.767	114.190000	-5.91	230	..... *	.....
15:18_05/08/2008	584.0	0.830	180.243000	-9.53	230	..... *	.....
10:08_16/10/2008	585.0	0.908	302.212000	-2.94	230	.....*	.....
10:15_16/10/2008	586.0	0.908	285.275000	-8.38	230	..... *	.....
07:20_16/12/2008	587.0	1.832	4194.480000	-6.00	230	.....*	.....
12:04_17/02/2009	588.0	0.767	133.000000	9.59	230	..... *	.....
08:08_08/04/2009	589.0	0.899	287.000000	-3.42	230	.....*	.....
14:35_15/06/2009	590.0	0.856	259.680000	9.53	230	..... *	.....
16:10_06/08/2009	591.0	0.813	182.650000	3.36	230	.....*	.....
07:40_14/10/2009	592.0	0.723	81.190000	6.00	230	..... *	.....

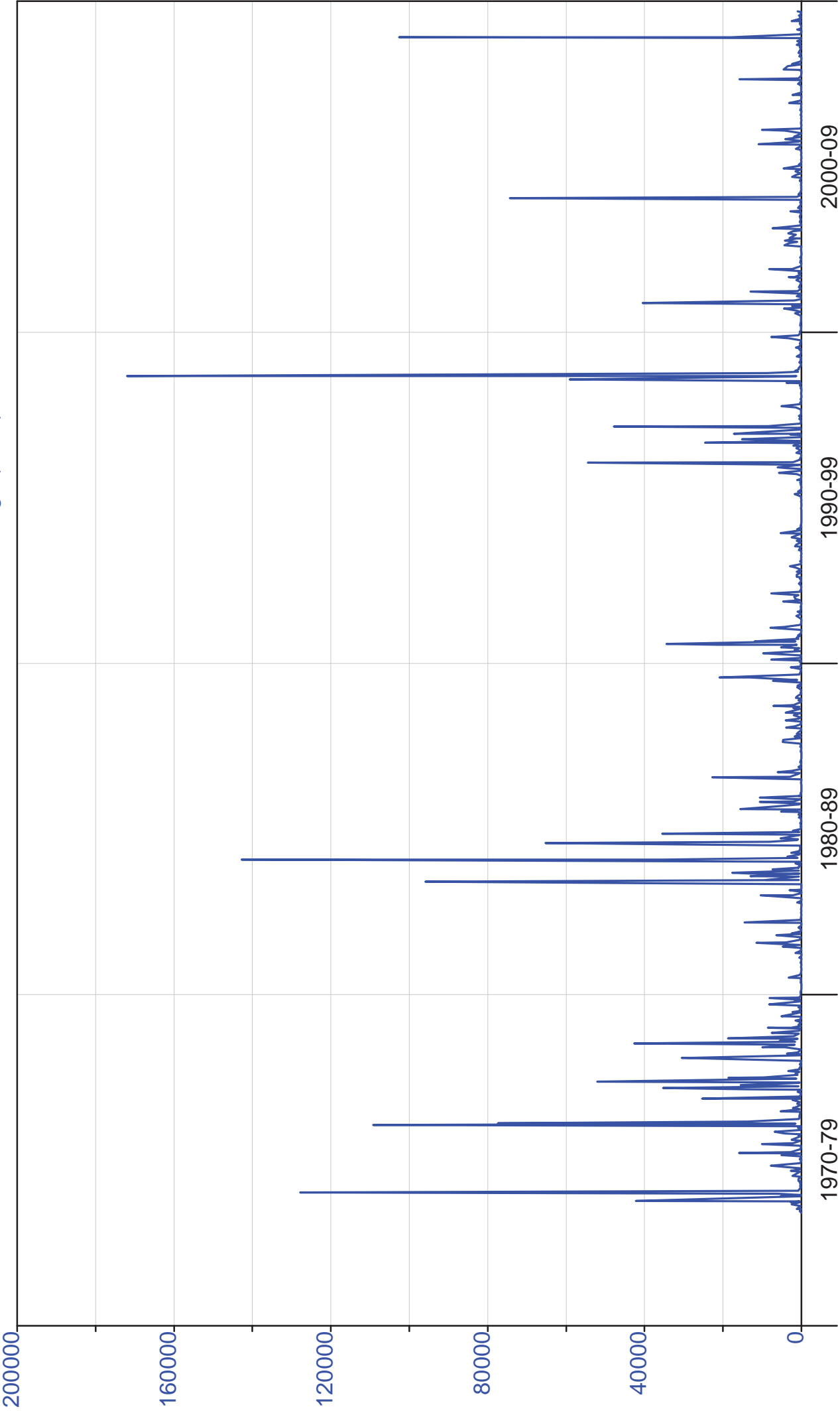
\* after gauge number indicates Weighted Mean Gauge Height method

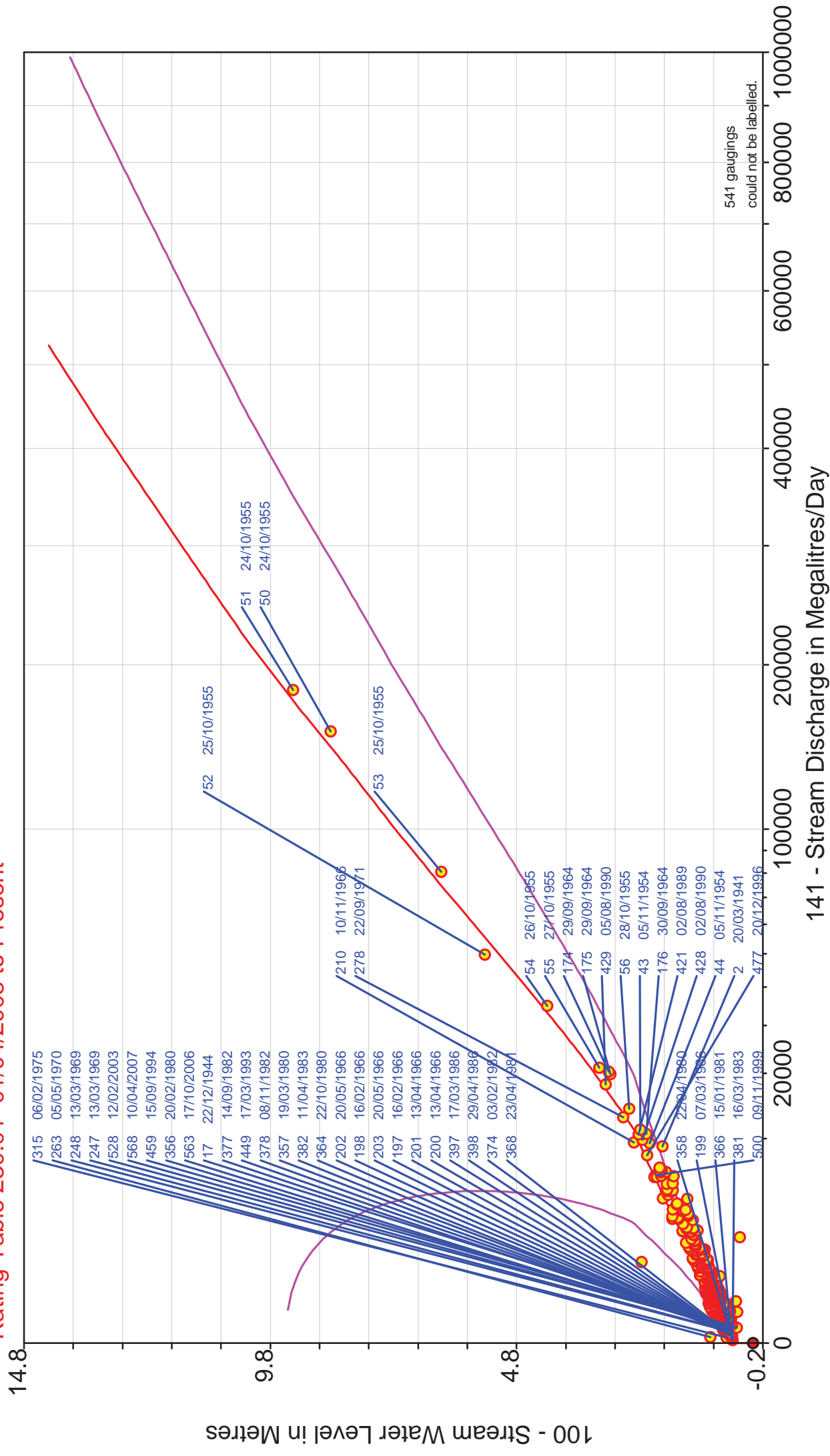
NSW Office of Water PINNEENA 9.3

Period 40 Year Plot Start 00:00\_01/01/1970  
Interval 1 Month Plot End 00:00\_01/01/2010

1970-10

— 419022 NAMOI @ MANILLA 141.00 Max & Min Discharge (ML/d)









## Appendix C

# Community Consultation

**MANILLA FLOOD STUDY  
CONSULTANT MEETING No 1**

**12 NOON, 20 JULY 2011, LEVEL 2, RAY WALSH HOUSE, TAMWORTH**

**PRESENT:-**

James McPherson [JM] - GHD  
Dr Rainer Berg [RB] - GHD [by phone link]  
Murray Russell [MR] - TRC  
Graeme McKenzie [GM] - TRC  
William Ash [WA] - TRC

**1. INTRODUCTION**

The meeting was convened to confirm the Manilla Flood Study methodology and Programme and to respond to any Request for Information (RFIs) GHD may have raised.

RB joined the meeting by phone link.

**2. PROJECT PLAN**

RB briefed the meeting on the proposed methodology.

Methodology is to comply with Section 4 Flood Study of the Consultants Brief and RB outlined the following components:-

- Modelling Platform.

RB reported that the level of detail in the Modelling Platforms was still to be determined. GHD will liaise with Office of Environment and Heritage (OEH) to finalise the model details.

All communication between GHD and OEH is to be documented and copied to TRC.

- Hydrology

RB stated RORB model set up GHD will develop the model then using hydraulic information including Frequency Analysis and measurements of events in creeks compare the actuals to the results of the model. The model will then be calibrated against a number of recorded historical and observed flood peaks using river gauging stations as benchmarks.

Survey information is not essential in building this model.

- Community Consultation

RB briefed the meeting on the consultation process and information sought from the Manilla community. GHD will draft the questionnaire and media material to be issued, TRC will review, approve and issue under cover of TRC letterhead and include contact details. Current documentation and process being employed on Coffs Harbour Flood Study will be amended and utilized for Manilla.

Issue of material will be by post based on TRC Rates database for property owners. Community Groups will be contacted. Letterboxing may be considered. Time frame/s for response will be specified.

Information requested will include historical floods, levels and impacts on property.

This item needs to be actioned ASAP.

- Hydraulic Models

RB reported GHD had built 2 'raw' models [TufLOW and Mikell] in the previous week. Models are to be tested using rainfall events and flood peaks. RB stated the type of model

is yet to be determined and the level of information to be included in the model [2 Flow or 2D] will be clarified within 2 weeks. The concurrence of OEH is required as to the type of model.

Both Hydrology and survey are required to be complete in order that the selected hydraulic model can be completed and calibrated against identified flood marks determined from TRC information and community knowledge.

GHD has obtained rainfall records from the Bureau of Meteorology [BOM] and has purchased flow data for waterways. This information will be used by GHD to determine the event it will use to calibrate the model/s. Results from 3-4 gauges with data periods with co-incidental large events would be utilized to determine large events for the purposes of calibration.

Once calibrated flood events can be modelled, properties at risk identified and estimates of damage prepared.

- Mapping

Maps are to be able to be interfaced with TRC GIS capability. GHD is to liaise with TRC GIS operator to ensure compatibility.

[Note following the meeting JM and WA met with TRC, GIS operator Joe Bagster to establish the lines of communication to ensure mapping meets with TRC requirements]

GHD is programming to have mapping complete by last week of October 2011.

- Climate Change

GHD stated the impact of climate change is difficult to model due to inputs being estimates only. Accordingly GHD is running its model with practical considerations as inputs.

In respect of predicted increased rainfall, GHD is proposing to run 200 and 500 year events through the model to estimate future flood levels and the impact of such events on the town.

Final inputs to be identified following further search of relevant data.

- Programme

GHD stated it would prepare and issue to TRC the community questionnaire during the last week of July 2011.

TRC requested that the programme show the Christmas/ New Year break.

TRC requested the programme include milestones, hold points and meetings, including meetings with Floodplain Management Committee.

A list of deliverables and dates of issue to TRC are to be included.

### **3. SURVEY**

RB reported GHD can continue to progress without survey at this time.

GHD has issued a brief to the preferred Surveyor [Peter Baxter] and is waiting on final pricing before advising TRC. This is expected during the first week of August with field work to commence immediately with a 6 week programme to complete.

Survey information is required to build the hydraulic model.

Initial tasks include site visit and walk to determine the information required is identified and ensure such information is correct. Information will include ground and creek levels and properties at risk locations and floor levels.

Information from survey will be used to further develop the hydraulic model/s and calibrate the model/s against historic flood levels established from historical information including TRC records and community response.

On completion of these tasks GHD will run the hydrographs through the model and map the flood effected areas.

#### **4. CLIENT [TRC] INFORMATION**

GHD requested TRC to provide the following information:-

- TRC LEP Map
- Plan of Cadesta
- Contour maps [if available]
- Ariel Photos
- Previous flood information and estimates
- Information on bridges, openings in culverts etc
- Information on Manilla weir
- Local historical information held by Community Groups etc
- Bridges blocked in previous flood events giving levels if available.
- Information on Split Rock Dam [State Water] including, time of construction, filling, water storage characteristics and discharge.

GHD requested it be allowed to access and use information available under Licence Agreements. TRC will facilitate this request.

GHD will email WA with a comprehensive list of requested information by 27 July 2011.

#### **5. DOCUMENTATION LISTING**

GHD to prepare a list of documents, issue to TRC and integrate into the programme.

#### **6. OTHER BUSINESS**

GHD requested TRC Purchase Orders for the Study.

There being no further business the meeting closed at 1.15 pm.

Next meeting to be advised.

WA and JM met with Joe Bagster following the meeting to discuss mapping information and interface with TRC GIS System.

# **MANILLA FLOOD STUDY**

## **NEWSLETTER NO.1, August 2011**

### **About the project**

As part of the NSW Government's Floodplain Risk Management Process, Tamworth Regional Council is developing a Flood Study for Manilla.

Manilla is located on the Namoi River at the junction of the Namoi and Manilla Rivers in the Namoi River catchment. The town is bisected by these two rivers and access is provided by a number of bridges. The Manilla River passes through Split Rock Reservoir upstream of the town and the Namoi River drains to Lake Keepit, downstream of the town.

Floodwaters rise quickly and can isolate properties and communities for several days. Many houses could be affected during a flood, necessitating evacuations.

Rainfall and river gauging data in the catchment is limited, however significant events have been recorded on a number of stream gauges.

The objective of this Flood Study is to define the flood behaviour under recent historical conditions and predict flood behaviour under existing and future conditions in the study area. The output will provide flood levels, depths, velocities, flows, hydraulic categories, and provisional hazard categories.

GHD have been contracted by Council to carry out the Manilla Flood Study.

### **Study Tasks**

In general terms, the following key tasks will be undertaken:

- Collect, compile and review available flood data relating to the study area in order to obtain a full understanding of flooding issues in the study area;
- Undertake community consultation to gather information on historical flooding in the area and the impact of flooding on the local community;
- Gather additional survey of watercourses and structures (for example bridges, culverts, road levels);
- Develop flood models to simulate historical and design floods. Calibrate and validate these models against recent historical flood data (for example the April 2009 event); and
- Map the extent of flooding, flood level contours and provisional flood hazards for the study area.

## **Community Information**

Council and GHD are committed to listening to the concerns and issues of the community and stakeholders, and strategies are in place to ensure that this information is integrated into the study. There will be opportunities throughout the study for the local community and stakeholders to feedback their concerns and issues.

As a first step, the project team is keen to learn when and where previous flooding has occurred in Manilla and the surrounding areas, and how you and your property were affected.

Attached is a survey questionnaire that all members of the community are encouraged to complete and return (postage paid). This is an opportunity to ensure that you have your say and that the information is captured.

### **For more information contact**

Lynne Clayton  
Stakeholder Engagement  
Manilla Flood Study  
Reply Paid 83475  
GHD, Level 15, 133 Castlereagh Street  
Sydney NSW 2000  
Facsimile – 02 9239 7199  
Email – [community.input@ghd.com.au](mailto:community.input@ghd.com.au)

**Project Information Line**  
**1800 810 680 (free call)**

## Manilla Flood Study Survey

Name: \_\_\_\_\_

Address: \_\_\_\_\_

Telephone: \_\_\_\_\_ Mob: \_\_\_\_\_

Please include my details in the stakeholder database to receive further information on the project ☐ (Please tick)

1. How long have you lived (or did you live) in Manilla? \_\_\_\_\_ Years \_\_\_\_\_ Months  
2. Have you ever seen or experienced a flood while living in Manilla? Yes ☐ No ☐ (Please tick)  
3. If yes, in what year/s did the floods take place?

4. What effects did the flood have on you and your property?  
\_\_\_\_\_  
\_\_\_\_\_

5. How quickly do flood waters rise, and how long do flood waters pond, or do they drain rapidly?  
\_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

6. Did you note any flood markers? (Flood markers could be lines on buildings that show the peak flood level during the flood event)  
\_\_\_\_\_  
\_\_\_\_\_

7. How do you prepare for a flood?  
\_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

6. Do you get isolated in a flood and is your property access restricted by flood waters?  
\_\_\_\_\_  
\_\_\_\_\_

7. Are there any other issues you would like us to take into consideration?  
\_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

**Thank you for taking the time to complete this survey.**

**Your feedback is invaluable in assisting the project team.**

# **BARRABA FLOOD STUDY**

## **NEWSLETTER NO.1, August 2011**

### **About the project**

As part of the NSW Government's Floodplain Risk Management Process, Tamworth Regional Council is developing a Flood Study for Barraba.

Barraba is located on the Manilla River upstream of Split Rock Reservoir in the Manilla River catchment. The town is located south of the river along The Fossickers Way. The Manilla River passes through Split Rock Reservoir downstream of the town.

Floodwaters rise quickly and can isolate properties and communities for several days. Many houses could be affected during a flood, necessitating evacuations.

Rainfall and river gauging data in the catchment is limited, however significant events have been recorded on a number of stream gauges.

The objective of this Flood Study is to define the flood behaviour under recent historical conditions and predict flood behaviour under existing and future conditions in the study area. The output will provide flood levels, depths, velocities, flows, hydraulic categories, and provisional hazard categories.

GHD have been contracted by Council to carry out the Barraba Flood Study.

### **Study Tasks**

In general terms, the following key tasks will be undertaken:

- Collect, compile and review available flood data relating to the study area in order to obtain a full understanding of flooding issues in the study area;
- Undertake community consultation to gather information on historical flooding in the area and the impact of flooding on the local community;
- Gather additional survey of watercourses and structures (for example bridges, culverts, road levels);
- Develop flood models to simulate historical and design floods. Calibrate and validate these model against recent historical flood data (for example the April 2009 event); and
- Map the extent of flooding, flood level contours and provisional flood hazards for the study area.



## **Community Information**

Council and GHD are committed to listening to the concerns and issues of the community and stakeholders, and strategies are in place to ensure that this information is integrated into the study. There will be opportunities throughout the study for the local community and stakeholders to feedback their concerns and issues.

As a first step, the project team is keen to learn when and where previous flooding has occurred in Barraba and the surrounding areas, and how you and your property were affected.

Attached is a survey questionnaire that all members of the community are encouraged to complete and return (postage paid). This is an opportunity to ensure that you have your say and that the information is captured.

### **For more information contact**

Lynne Clayton  
Stakeholder Engagement  
Barraba Flood Study  
Reply Paid 83475  
GHD, Level 15, 133 Castlereagh Street  
Sydney NSW 2000  
Facsimile – 02 9239 7199  
Email – [community.input@ghd.com.au](mailto:community.input@ghd.com.au)

**Project Information Line**  
**1800 810 680 (free call)**

## Barraba Flood Study Survey

Name: \_\_\_\_\_

Address: \_\_\_\_\_

Telephone: \_\_\_\_\_

Mob: \_\_\_\_\_

Please include my details in the stakeholder database to receive further information on the project ☐ (Please tick)

1. How long have you lived (or did you live) in Barraba?

\_\_\_\_ Years \_\_\_\_ Months

2. Have you ever seen or experienced a flood while living in Barraba?

Yes ☐ No ☐ (Please tick)

3. If yes, in what year/s did the floods take place?

4. What effects did the flood have on you and your property?

5. How quickly do flood waters rise, and how long do flood waters pond, or do they drain rapidly?

6. Did you note any flood markers? (Flood markers could be lines on buildings that show the peak flood level during the flood event)

7. How do you prepare for a flood?

6. Do you get isolated in a flood and is your property access restricted by flood waters?

7. Are there any other issues you would like us to take into consideration?

**Thank you for taking the time to complete this survey.**

**Your feedback is invaluable in assisting the project team.**

**MANILLA FLOOD STUDY  
CONSULTANT MEETING No 2  
3.00 PM, 3 AUGUST 2011,  
LEVEL 2, RAY WALSH HOUSE, TAMWORTH**

**PRESENT:-**

James McPherson [JM] - GHD  
Dr Rainer Berg [RB] - GHD  
William Ash [WA] - TRC

**APOLOGIES**

Murray Russell [MR] - TRC  
Graeme McKenzie [GM] - TRC

**1. INTRODUCTION**

The meeting was convened to discuss progress on the Manilla Flood Study, confirm methodology and to respond to any Request for Information (RFIs) GHD may have raised.

**2. PROJECT PLAN**

RB confirmed the proposed methodology for the Study.

Methodology is to comply with Section 4 Flood Study of the Consultants Brief and RB outlined the following components:-

- Modelling Platform.

RB reported that the level of detail in the Modelling Platforms was still to be determined. GHD will liaise with Office of Environment and Heritage (OEH) to finalise the model details and reported he would be meeting with Neal Albert [NA] of OEH immediately following this meeting. Matters concerning the methodology would be discussed with OEH.

WA confirmed communication between GHD and OEH is to be documented and copied to TRC.

- Hydrology

RB stated RORB model set up GHD will develop the model then using hydraulic information including Frequency Analysis and measurements of events in creeks compare the actuals to the results of the model. The model will then be calibrated against a number of recorded historical and observed flood peaks using river gauging stations as benchmarks.

Survey information is not essential in building this model.

- Community Consultation

RB briefed the meeting on the consultation process and information sought from the Manilla community. GHD will draft the questionnaire and media material to be circulated to Manilla residents and issue drafts to TRC on 5 Aug 2011. TRC will review, approve and issue under cover of TRC letterhead and include GHD contact details.

TRC confirmed Issue of material will be by post, based on TRC Rates database for property owners. In addition each property will be contacted to ensure tenants are included in the survey. Community Groups will be contacted. Time frame/s for response will be specified.

Information requested will include historical floods, levels and impacts on property.

This item needs to be actioned ASAP. GHD confirmed issue of draft documents by 5 Aug

- Hydraulic Models

RB confirmed GHD had built 2 'raw' models [Tuflow and Mikell] during the first week of the Study. Models are to be tested using rainfall events and flood peaks. RB stated the type of model is yet to be determined and the level of information to be included in the model [2 Flow or 2D] will be clarified in the immediate future. The concurrence of OEH is required as to the type of model. This will be an item for discussion with OEH following this meeting.

Both Hydrology and survey are required to be complete in order that the selected hydraulic model can be completed and calibrated against identified flood marks determined from TRC information and community knowledge.

GHD has obtained rainfall records from the Bureau of Meteorology [BOM] and has purchased flow data for waterways. This information will be used by GHD to determine the event it will use to calibrate the model/s. Results from 3-4 gauges with data periods with co-incidental large events would be utilized to determine large events for the purposes of calibration.

Once calibrated flood events can be modelled, properties at risk identified and estimates of damage prepared.

- Mapping

Maps to be interfaced with TRC GIS capability. GHD and TRC GIS operator are in communication to ensure compatibility.

GHD is programming to have mapping complete by last week of October 2011.

- Climate Change

GHD stated the impact of climate change is difficult to model due to inputs being estimates only. Accordingly GHD is running its model with practical considerations as inputs.

In respect of predicted increased rainfall, GHD is proposing to run 200 and 500 year events through the model to estimate future flood levels and the impact of such events on the town.

Final inputs to be identified following further search of relevant data.

- Programme

TRC had previously requested that the programme show the Christmas/ New Year break.

GHD tabled a revised programme including milestones, hold points, deliverables and meetings.

A list of and dates of issue to TRC have been included.

### **3. SURVEY**

RB confirmed GHD can continue to progress without survey at this time.

GHD has issued a brief to the preferred Surveyor [Peter Baxter] and is conducting a site inspection with Baxter tomorrow, 4 Aug 2011. Following the site inspection GHD will prepare draft plans showing survey limits and issue to TRC and OEH for approval.

The model areas have to be determined prior to preparing final pricing for the survey. Field work will commence on TRC approval of the survey limits and survey cost with a 6 week programme to complete.

GHD confirmed survey information is required to build the hydraulic model.

Initial tasks are to determine the information required is identified and ensure such information is correct. Information will include ground and creek levels and properties at risk locations and floor levels.

Information from survey will be used to further develop the hydraulic model/s and calibrate the model/s against historic flood levels established from historical information including TRC records and community response.

On completion of these tasks GHD will run the hydrographs through the model and map the flood effected areas.

#### **4. CLIENT [TRC] INFORMATION**

GHD confirmed it had signed the Licence Agreements that allowed accessing and using information held by TRC and that information is being made available.

GHD confirmed TRC has provided the following information:-

- TRC LEP Map
- Plan of Cadesta
- Contour maps
- Ariel Photos [limited]
- 

GHD confirmed it required the following information ASAP

- Previous flood information and estimates
- Information on bridges, openings in culverts etc
- Information on Manilla weir
- Local historical information held by Community Groups etc
- Bridges and/or other structures blocked in previous flood events, giving levels if available.
- Information on Split Rock Dam [State Water] including, time of construction, filling, water storage characteristics and discharge.

GHD has issued TRC with a comprehensive list of information requested.

TRC handed to GHD copies of documents containing historical information on previous floods.

A catalogue of these documents will be issued to GHD.

TRC will continue to search its archives for further documents and/or information relating to historical flood information.

#### **5. DOCUMENTATION LISTING**

GHD has issued a list of documents to TRC and integrated these into the programme.

#### **6. OTHER BUSINESS**

GHD requested TRC Purchase Orders for the Study.

There being no further business the meeting closed at 3.45 pm.

Next meeting to be advised.

**MANILLA FLOOD STUDY  
CONSULTANT MEETING No 3**

**10.00 am FRIDAY 16 SEPTEMBER 2011**

**LEVEL 2, RAY WALSH HOUSE, TAMWORTH**

**PRESENT:-**

James McPherson [JM] - GHD  
Dr Rainer Berg [RB] - GHD [by phone link]  
Neal Albert [NA] - Office of Environment and Heritage  
Andrew Falkenmire [AF] - Office of Environment and Heritage  
Peter Baxter [PB] - Peter Baxter Surveyors, Sub-Consultant to GHD  
Thomas Baxter [TB] - TRC  
William Ash [WA] - TRC

**APOLOGIES**

Murray Russell [MR] - TRC  
Graeme McKenzie [GM] - TRC

**1. INTRODUCTION**

The meeting was convened to review progress on and provide the management team with visibility of the technical aspects of the Manilla Flood Study and to respond to any Requests for Information (RFIs) GHD may have raised.

RB joined the meeting by phone link.

**2. PROJECT PROGRAMME STATUS**

RB briefed the meeting by phone link, on the progress against the programme and a summary of his report follows:-

**2.1 Programme**

- A site visit in concert with the ground survey team headed by PB has been carried out. The topography of the terrain was inspected. RB reported the area was not as heavily wooded as previously thought, the main wooded area being at the confluence of the Namoi River and Halls Creek. Flood channel was open and banks were not steep.
- From on site observations the Survey Brief was confirmed by GHD and PB.
- Number of houses to have floor levels established for damage calculations will be calculated using the PMF + 0.5M.
- PB reported survey control points have been established. These will be used by both the ground survey team and to calibrate the existing aerial survey information. Areometrex 'holes' will be filled in and 'rough' contours established.
- PB reported that structures affected by flooding i.e. bridges and culverts would be surveyed next week.
- PB. Model will be in HECRAS format. From the basic model areas inundated during PMF flood events can be delineated estimating from 100 yr x 3 beyond the form lines and the aerial information can be used to determine 'rough' DTM.
- PB reported GHD is on programme.



## **2.2 Hydrology**

- RB has previously stated RORB model set up GHD will develop the model then using hydraulic information including Frequency Analysis and measurements of events in creeks compare the actuals to the results of the model. The model will then be calibrated against a number of recorded historical and observed flood peaks using river gauging stations as benchmarks. Survey information is not essential in building this model.
- RB reported Hydrology model has been built and is ready for calibration using information from previous rain and flood events and set systems flow gauges for calibration. RB stated GHD has a good understanding of flows from historical records from previous information providing basic information on the magnitude of flows.

## **2.3 Hydraulics**

- Construction and accuracy of the Hydraulic Model is dependant on the provision of correct survey information. PB stated model would be TUFLOW and GHD would build a substantial part of the model using form lines the replace the inputted data with the more accurate DTM when this became available. PB stated GHD would commence to build the model/s during week commencing 19 September 2011.
- RB stated GHD is considering 2 model options and would discuss this with OEH to ensure the model meets OEHs criteria before making the final decision. NA stated OEH concurs with GHDs approach with the proviso the OEH needs to be comfortable with the 'raw' models flood frequency. OEH and GHD would discuss this matter after the meeting and would inform the meeting on the outcome of the discussions.

## **2.4 Survey**

- RB/PB confirmed site Information will include ground and creek levels and properties at risk locations and floor levels.
- PB confirmed that Manilla ground survey would start week commencing 19 Sept 2011. Control points will be established to allow calibration of Areometrex and confirm ground survey.
- PB stated the Areometrex has an accuracy of +/- 150mm. This is to be confirmed.
- TB to follow up with Areometrex to expedite transfer of information.
- A brief of survey and how this integrates with the modelling software is to be provided. TB to review.
- TB stated all survey information is to be available for use by all levels of Government without limitations.
- PB requested any historical information TRC holds in respect of previous flood levels including flood markers [if any].
- Maps are to be able to be interfaced with TRC GIS capability. GHD is to liaise with TRC GIS operator the ensure compatibility. GHD is programming to have mapping complete by last week of October 2011.

- In respect of Climate Change GHD has previously stated the impact of climate change is difficult to model due to inputs being estimates only. Accordingly GHD is running its model with practical considerations as inputs. In respect of predicted increased rainfall, GHD is proposing to run 200 and 500 year events through the model to estimate future flood levels and the impact of such events on the town. Final inputs to be identified following further search of relevant data.

## **COMMUNITY CONSULTATION**

- WA reported Newsletter and questionnaire had been distributed to the Manilla area. RB stated that responses to the questionnaire had been received in GHDs Sydney office and these are currently being reviewed.
- WA advised TRC is establishing the Floodplain Management Committee and is seeking representatives of the community to sit on the committee. Adverts seeking Expressions of Interest local residents to join the committee have been placed in local newspapers.
- The first Committee meeting is scheduled for 6 Oct 2011. The meeting noted this matter is urgent.
- RB stated community involvement through the Committee is fundamental in gaining the communities approval of the Study and ongoing Management Plans. Local knowledge of historical flood events greatly assists with calibration of the models and this knowledge can be utilized by 'running' these known events through the model and gaining Committee/community acceptance as to the accuracy of the model/s and confidence as to predicting future flood events.

..

## **4. CLIENT [TRC] INFORMATION**

- GHD requested TRC to expedite the Areometrex information

.

## **5. DOCUMENTATION LISTING**

GHD to prepare a list of deliverables and issue to TRC and integrate into the programme.

## **6. OTHER BUSINESS**

.

There being no further business the meeting closed at 11.00 a.m.

Next meeting to be advised.



# Barraba & Manilla Floodplain Risk Management

FPMC Meeting

19/10/2011



# Locations



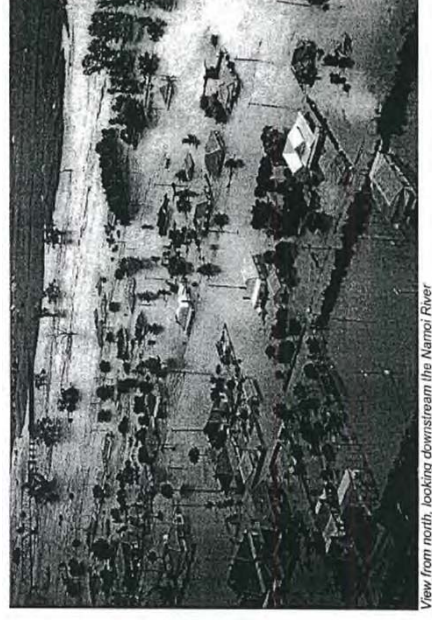
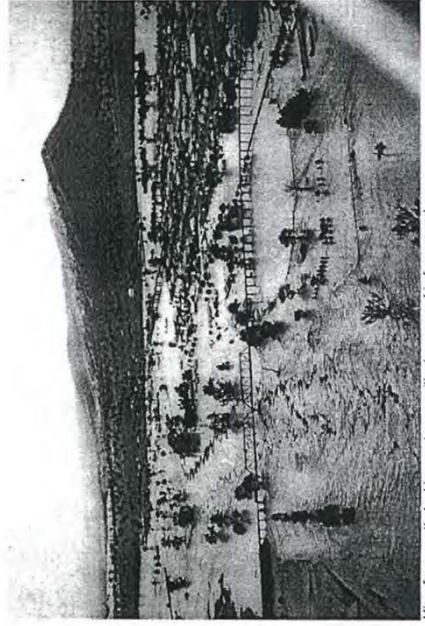
# Flood History

## Manilla

- 1840
- Feb 1864 – drowned 4/12
- 1910
- Since 1941 (gauging start) – 16 minor, 3 moderate & 2 major floods
- Feb 1955 – major, highest since 1910
- Jan 1964 – major, worst in 100yrs
- 1971, 1984, 1998, 2004....

## Barraba

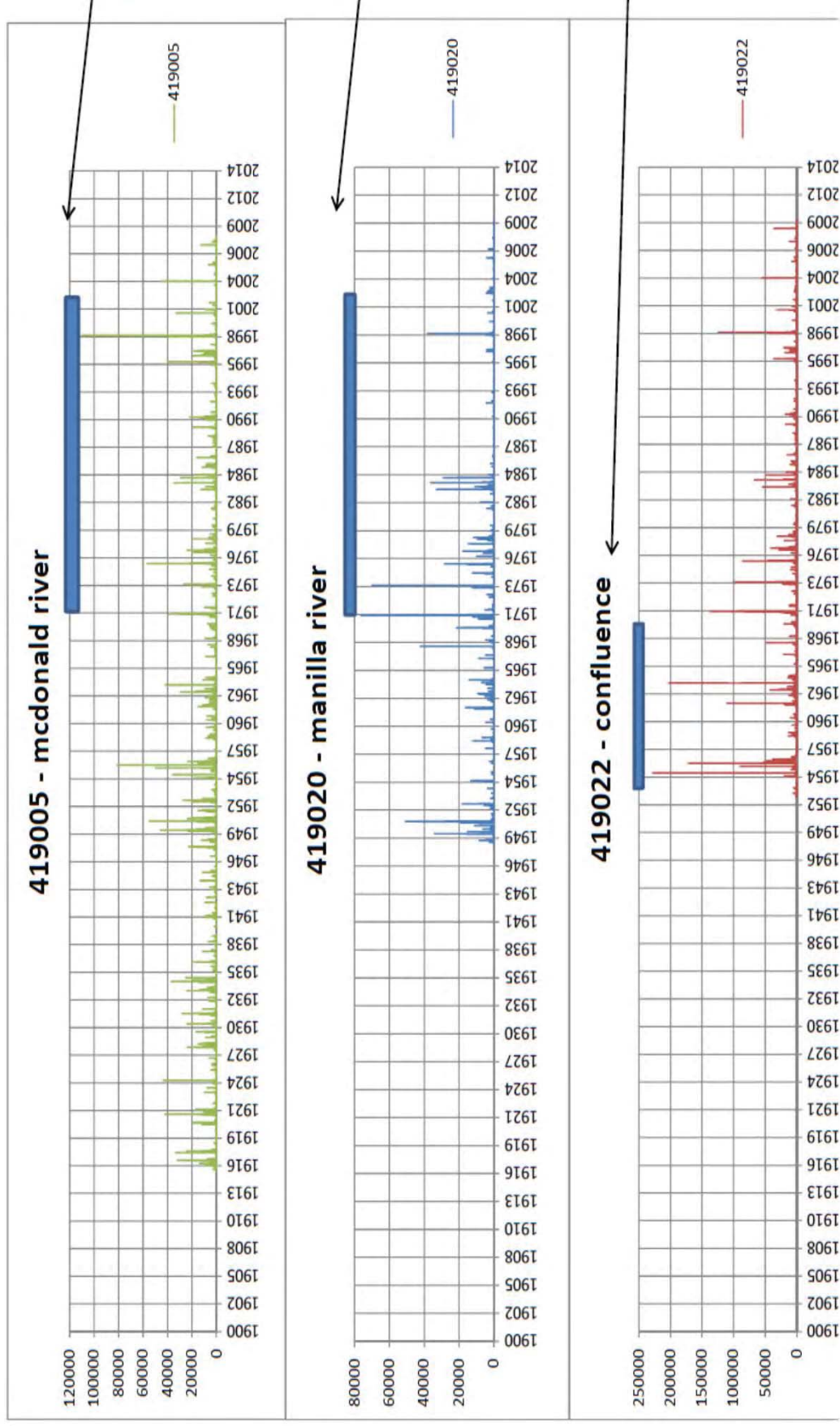
- 1864
- 1964 – worst in 100yrs
- 1955
- 1971
- 1974, 1984, 1998, 2003, 2004





# Manilla and Barraba Flood History

Daily Recorded Flows ML/day





# Floodplain Development Manual

## the management of flood liable land



New South Wales Government

- Reduce the social and financial costs from the risks associated with occupying the floodplain;
- Increase the sustainable benefits of using the floodplain; and
- Improve or maintain floodplain ecosystems dependent on flood inundation.

### 3 types of Flood Risks

- **Existing flood risk**, associated with current development on flood prone land. For example, this may be the risk to existing development areas that can be effectively managed by the construction of a levee;
- **Future flood risk**, associated with any new development on flood prone land. For example, this may be the risk to future development areas that can be managed by adopting appropriate development limits, and minimum fill levels for lots and minimum floor levels for buildings; and
- **Continuing flood risk**, is the risk remaining, in both existing and future development areas. This is the risk from rarer floods which may result in levee overtopping or flooding of. The consequences of these rarer floods may include danger to personal safety and damages to infrastructure, and both public and private property.



## Floodplain Management Committee

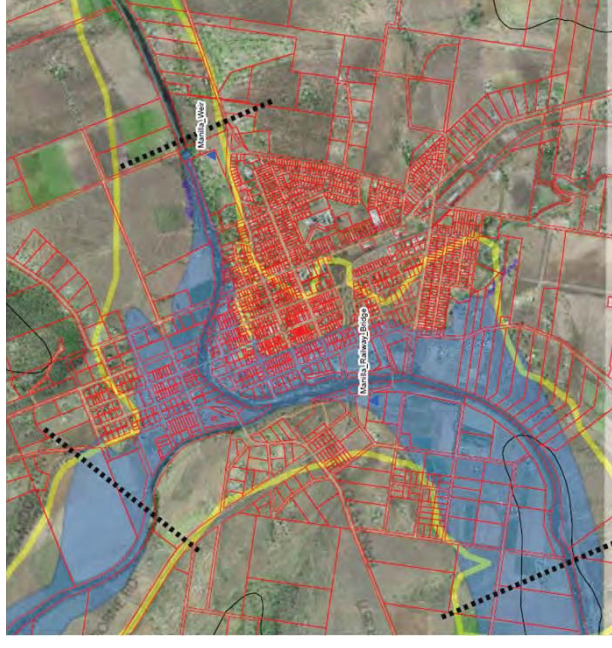
- Acts as both a focus and forum for the discussion of technical, social, economic, environmental and cultural issues and for the distillation of possibly differing viewpoints on these issues into a management plan
- Ensures that all stakeholders (often with competing desires) are equally represented. As such, the composition and roles of committee members are matters of key importance
- Does not have any formal powers. Rather, it has an advisory role, but an important one. The principal objective of the committee is to assist the Council in the development and implementation of a management plan for the area(s) under its jurisdiction

## Flood Study - Key Tasks

- Obtain Survey
- Community Involvement and Input
- Calculate Flood Peaks & Hydrographs
- Calculate Flood Levels
- Provide Flood Mapping
- Undertake Flood Damage Assessments

## Survey

- Currently underway
- Using photogrammetry
- Manilla being prioritised
- Bridge surveys done
- Barraba photogrammetry underway



# Community Involvement

- Media Release
- Newsletter & Survey

## For more information contact:

Lynne Clayton  
Stakeholder Engagement  
Manilla Flood Study  
Reply Paid 83475  
GHD, Level 15, 133 Castlereagh Street  
Sydney NSW 2000  
Facsimile – 02 9239 7199  
Email – [community.input@ghd.com.au](mailto:community.input@ghd.com.au)

**Project Information Line  
1800 810 680 (free call)**

## Survey Returns

- Manilla - 92
- Barraba - 60



## Manilla Flood Study Survey

Name: Mrs A. JENNIFER

Address: 48 MANOL STREET MANILLA 2346 N.S.W.

Telephone: 6787 474 Mob: \_\_\_\_\_

Please include my details in the stakeholder database to receive further information on the project ☐ (Please tick)

1. How long have you lived (or did you live) in Manilla? 80 Years 6 Months  
Yes ☒ No ☐ (Please tick)

2. Have you ever seen or experienced a flood while living in Manilla?

3. If yes, in what year/s did the floods take place?  
14TH JANUARY 1964

4. What effects did the flood have on you and your property?  
DAMAGED DAMAGED ALL FLOOR COVERINGS  
RISE QUICKLY  
DRAIN AWAY QUICKLY

5. How quickly do flood waters rise, and how long do flood waters pond, or do they drain rapidly?

6. Did you note any flood markers? (Flood markers could be lines on buildings that show the peak flood level during the flood event)  
Muddy Marks on walls

7. How do you prepare for a flood?  
LIKE ANYONE ELSE WOULD

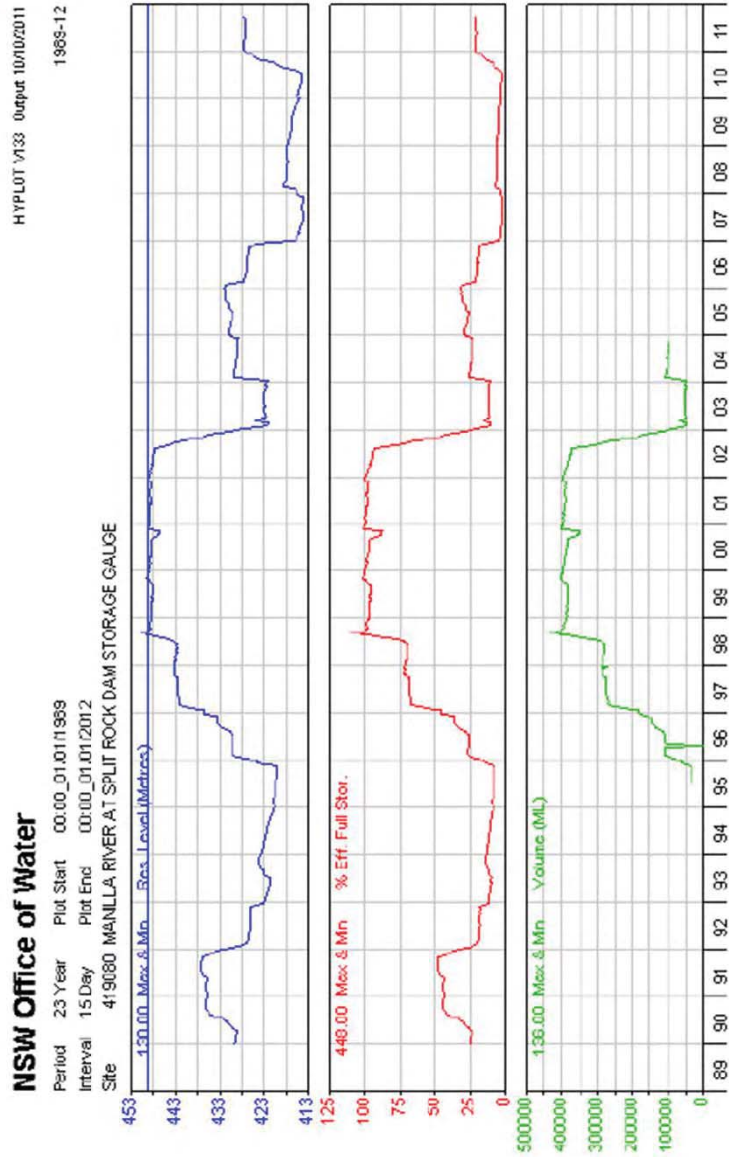
8. Do you get isolated in a flood and is your property access restricted by flood waters?  
ONLY THE OVER

7. Are there any other issues you would like us to take into consideration?  
DON'T WANT TO SEE ANY MORE

Thank you for taking the time to complete this survey.  
Your feedback is invaluable in assisting the project team.



# Split Rock Dam

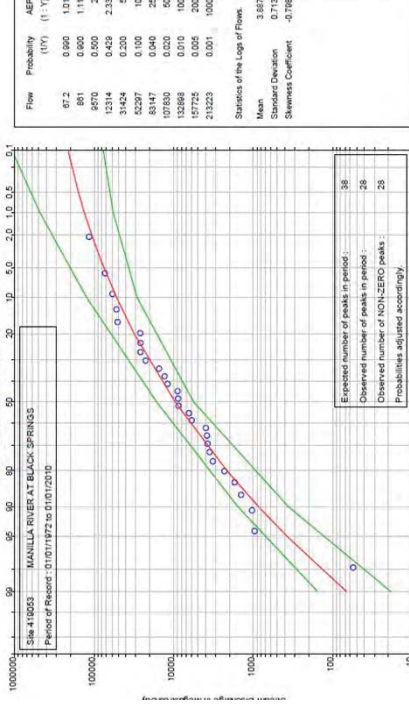
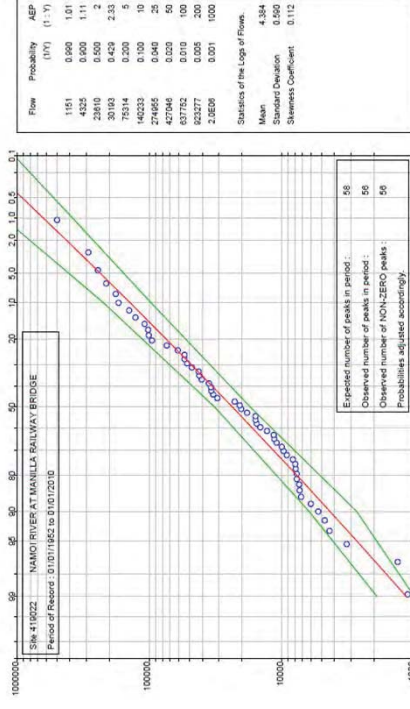
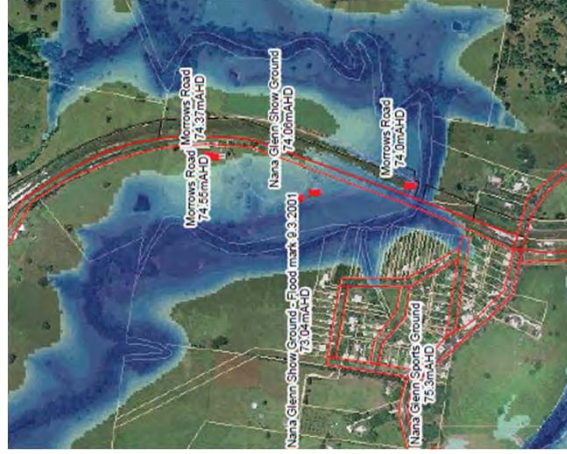




# Flood Levels & Maps

- Compile Flood Level model
- Calibrate Flood Level model
- Simulate Flood Level Model
- Produce Flood Mapping

Example  
Only



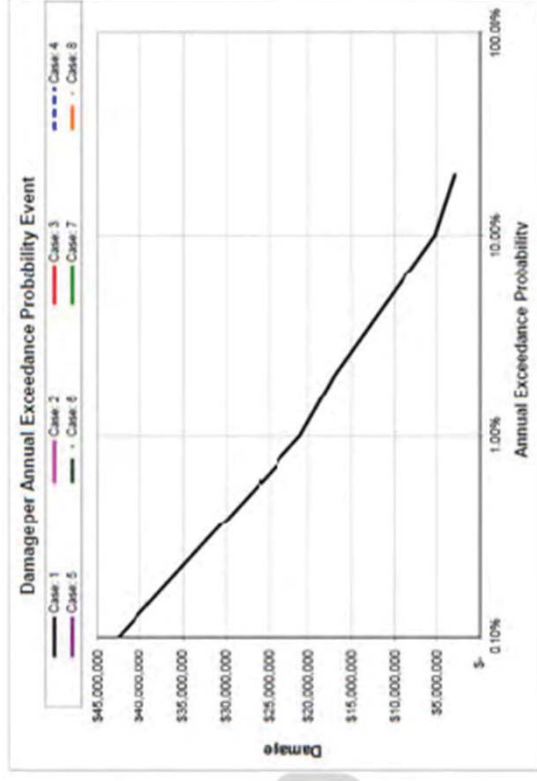
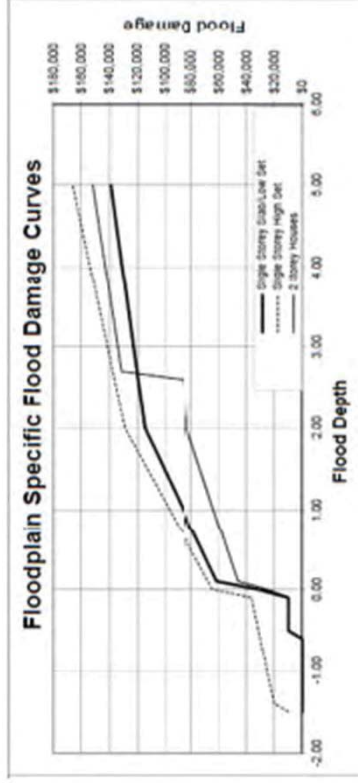
100yr Barraba approx 1000m3/s  
100yr Manilla approx 7400m3/s



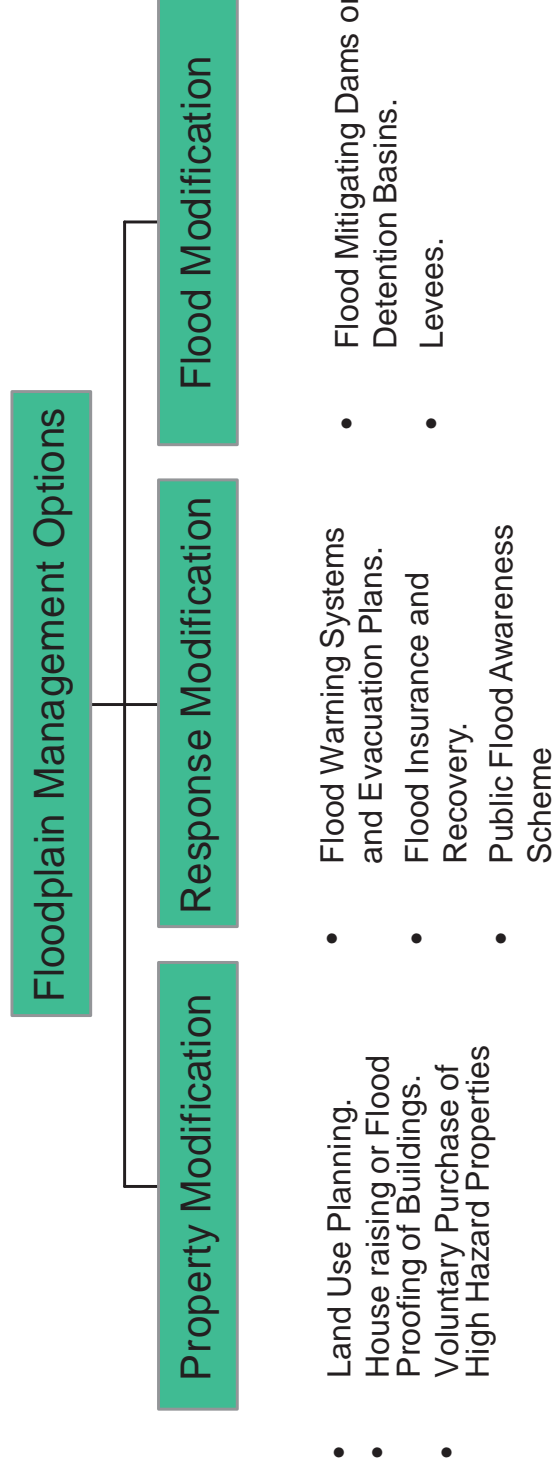
# Assessment - Flood Damage

- AAD \$?? million
- NPV over 20-years \$?? million

Flood Event	Number of Dwellings Inundated
5-year	4
10-year	10
50-year	148
100-year	188
PMF	335



# Floodplain Management Options

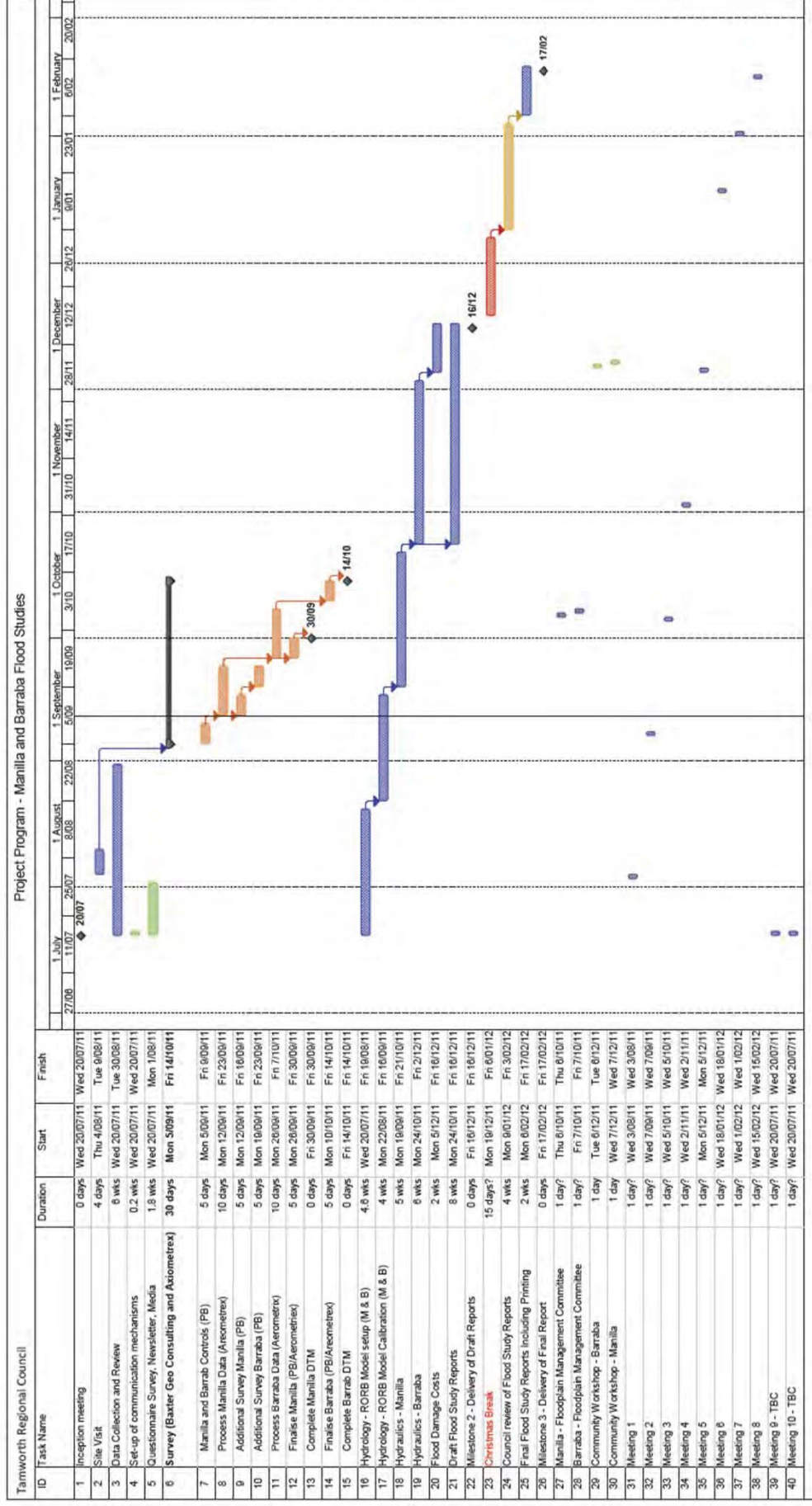


# Assessment of Options

Category	Issues
Social	<ul style="list-style-type: none"> <li>• The capacity of the option to reduce flood hazards and personal safety risks to the community;</li> <li>• How the option will influence property values;</li> <li>• The capacity of the option to promote community growth; and</li> <li>• The level of disruption to the community, either through implementing the option or through the resulting floodplain behaviour.</li> </ul>
Economic and Financial	<ul style="list-style-type: none"> <li>• The capital costs associated with implementing the option;</li> <li>• The ongoing or maintenance costs of the option; and</li> <li>• The costs or savings of flood damage after the option is implemented.</li> </ul>
Environmental	<ul style="list-style-type: none"> <li>• Change to ecology, habitats, riparian vegetation, and the “natural state” of the river;</li> <li>• Pollution;</li> <li>• Energy and resources required to implement the option; and</li> <li>• Energy and resources required for maintaining and decommissioning the option.</li> </ul>



# Progress





[www.ghd.com](http://www.ghd.com)



**TAMWORTH REGIONAL COUNCIL  
MANILLA FLOOD STUDY  
FLOODPLAIN RISK MANAGEMENT COMMITTEE**

**INAUGRAL MEETING  
MINUTES**

1.00pm Thursday 20 October 2011  
Ray Walsh House, Peel Street Tamworth

**Attendees**

Neal Albert	[NA]	Office of Environment and Heritage
Andrew Galvin	[AG]	State Emergency Services
Dr Rainer Berg	[RB]	GHD
Peter Resch	[PR]	Tamworth Regional Council (TRC)
Graeme McKenzie	[GM]	TRC
William Ash	[WA]	TRC
Tom Baxter	[TB]	TRC [part time]

**Apologies**

Andrew Falkenmire	[AF]	Office of Environment and Heritage
Murray Russell	[MR]	TRC
Ray Tait	[RT]	TRC ( detained on Council business)

**Item 1 Introduction**

WA opened the Inaugral Floodplain Risk Management Committee meeting noting those attending and apologies.

WA stated there had been a lack of response from residents of Manilla to sit on the Floodplain Risk Management Committee [Committee] despite newspaper advertisements inviting locals to apply and direct approach to specific individuals. Therefore Manilla community was not represented at this meeting.

RB stated a broad representation and community involvement through the Committee is fundamental in gaining the communities approval of the Study and ongoing Management Plans. Local knowledge of historical flood events greatly assists with calibration of the models and this knowledge can be utilized by 'running' these known events through the model/s, gaining Committee/community acceptance as to the accuracy of the model/s and a level of confidence as to predicting future flood events.

The Committee agreed that local representation is essential to the ultimate success of the Study in particular the Risk Management Plan and a number of actions to secure local members were canvassed including the drafting of TRC staff from the area and contact with the local Community Development Committees.

The meeting agreed that TRC Planning Department is to be represented on the Committee.

GM/WA to action.

**Item 2. Election of Committee Chairman**

The meeting proposed that RT be approached to Chair the Committee.

Following this proposal PR contacted RT by telephone. RT agreed to Chair the Committee. The Committee confirmed RT as its Chairman.

The Committee has no formal powers and acts as a forum for the discussion and over viewing of the reports and dissemination of information from the studies findings to the community and other interested parties.

It is important that all stakeholders be equally represented.

Matters for discussion include the technical, social, environmental, economic and cultural impacts of flooding in the areas the subject of the study.

**Item 3 Address by Dr Rainer Berg outlining the process, procedures and objectives of Flood Studies in flood prone areas.**

RB addressed the Meeting with an overhead presentation providing an overview of the process, procedures and objectives of Flood Studies generally and how these applied specifically to both Manilla and Barraba.

RB has provided an electronic copy of the presentation. This is being forwarded to all Committee members as an attachment to these minutes.

RB explained the structure of both Manilla and Barraba Flood Studies was based on and in accordance with the NSW Government Floodplain Management Manual [Manual] with the functions and status of the Committee being detailed in Appendix D.

RB stated the work being undertaken by GHD in its current commission is defined in the Manual in Section 2.3 Data Collection and Section 2.4 Flood Study.

The Study examines from historic events the existing and future flood risk to the subject area and the information gained is incorporated into the Floodplain Risk Management Plan and its implementation.

**Item 4. Report on current status of Manilla Flood Study and programme to completion of the Flood Study phase.**

RB provided a status report on the progress of the Manilla Flood Study using the Gantt Chart Programme as the demonstration tool and briefed the Committee on information gained to date including historical events and Progress against the programme.

The salient points are:-

- The major flood events in Manilla since the commencement of river gauging as measured from the gauging stations were in 1955, 1962, 1964 [worst in 100 years] 1971, 1984, 1998 and 2004.

RB stated that information from the river gauges varied in content and usefulness depending on the length of time they had been in operation and the frequency of data collection. Only a small number of gauges provide sufficient data to assist with accurate modelling. A log of flood records from a number of river gauges was included in GHDs presentation.

- The major flood impact on Manilla is downstream of the confluence of the Manilla and Namoi Rivers and Greenhatch Creek.
- A site visit in concert with the ground survey team determined the general topography of the area including vegetation and flood channels.
- From discussions with TRCs GIS operators and on-site observations the Survey Brief was confirmed as providing sufficient accurate information to build the models.

- In respect of flood damage assessment it is essential for each property deemed to be subject to flooding to have accurate information to determine the type of property [residential, commercial, industrial], type of construction and the height of the potential flood over the property using the PMF + 0.5m. The number of properties requiring floor levels to be determined for damage calculations have been estimated from previous events and projecting to PMF =0.5m
- PB reported survey control points have been established. These are being utilized to calibrate the existing aerial survey information. Areometrex 'holes' will be filled in and 'rough' contours established.
- PB reported that structures affected by flooding i.e. bridges and culverts will be surveyed next week and information 'fed' into the models.
- PB included record of water levels in Split Rock Dam and noted that the last major flood in 1998 coincided with high levels of water in the dam. Since the dam has lower levels [2003/2004 on] no major flooding has occurred. The hydraulic models will take the dam and its effect on Manilla flooding into consideration.
- PB stated response from the community to the questionnaire had been good and the information provided processed for use in developing the models.
- RB has previously stated RORB model set up and GHD will develop the model then, using hydraulic information including Frequency Analysis and measurements of events in creeks, compare the actuals to the results of the model. The model will then be calibrated against a number of recorded historical and observed flood peaks using river gauging stations and historical flood levels as benchmarks. Complete survey information is not essential in finalising the construction of this model. RB reported Hydrology model has been built and calibration is in train using information from previous rain and flood events and records from flow gauges for calibration. RB stated GHD has a good understanding of flows from historical records providing basic information on the magnitude of flows. The model will replicate rainfall events to match what happens in the waterways. The objective is to have the model able to replicate what actually happens in real terms.
- RB stated OEH has approved the use of the TUFLOW model for the hydraulics as meeting its [OEHs] criteria.
- Construction and accuracy of the Hydraulic Model is dependant on the provision of correct survey information. PB confirmed the model is TUFLOW and GHD will build a substantial part of the model using form lines then replace the data with the more accurate DTM when this became available. PB stated GHD had commenced the Model using current data. Model completion is dependant on finalisation of field survey and correlation with areometrex.

#### 4.1 Survey

- RB/PB confirmed Site Information will include ground and creek levels and properties at risk locations and floor levels.
- NA raised the matter of Quality Assurance in respect of survey citing problems with previous ariel survey information provided to OEH and noted that survey information gathered during the Flood Studies is required for use by NSW Government and therefore the need for accuracy is paramount.
- Thomas Baxter was invited to brief the Meeting on how QA of the survey information could be effected. TB stated that from previous experience LPI

was not 'geared' to QA the form of information being obtained by the systems used on this project but he would contact the LPI to investigate if that organisation could assist with QA. In the absence of LPI assistance GHD to investigate how the survey information can be subjected to QA.

The accuracy stated for areometrex as +/- 150mm is to be confirmed.

#### **Item 5 Community Consultation**

- RB stated that responses to the questionnaire had been received in GHDs Sydney office and the information contained is being processed and incorporated into the Study Data.
- As stated above there has been a lack of response from residents of Manilla to sit on the Committee despite newspaper advertisements and direct approach to specific individuals inviting locals to apply to be involved in the process.
- The Committee agreed that local representation is essential to the ultimate success of the Study in particular the Risk Management Plan and a number of actions to secure local members were identified including the drafting of TRC staff from the area and contact with the local Community Development Committees.
- RB proposed that the public meeting at Manilla be held on or about 7 December 2011. The Meeting concurred and TRC is to convene.

#### **Item 6 Discussion on format, presentation and release of Study findings in Final Report**

Report will comply with standards and format described in the Floodplain Risk Management Plan.

#### **Item 7 Discussion on actions post Flood Study Final Report**

Following acceptance of the Flood Study Final Report the actions will Comply with the Flow Chart as contained in Appendix C of the Floodplain Risk Management Plan.

#### **Item 8 General business**

There being no further business the Meeting closed at 2.30pm

Next Meeting: to be advised.



**NEWSPAPER INSTRUCTIONS  
REGIONAL COUNCIL NEWS ADS**

<b>PAPER</b>	<b>TO APPEAR (DATE)</b>	<b>JOB NUMBER</b>	<b>AUTHORISED BY</b>
MANILLA EXPRESS	TUESDAY 13 DECEMBER 2011	AUF010-1598-400	BILL ASH

**MANILLA FLOOD STUDIES – COMMUNITY INFORMATION SESSION**

Tamworth Regional Council has engaged consultants to carry out the Manilla Flood Study, to define flooding in Manilla and inform Council's planning for the management of flooding.

Residents who have questions about the study, or who have been affected by flooding previously, are invited to a Community Information Session being held at 6.00pm, 14 December at the Manilla Town Hall.

Should you have any enquiries about the information session or the flood study, please contact William Ash, Senior Engineer, on 6767 5738 or 0417 964 972.

**3 COURSES**

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Churches of Manilla and Barraba. The program covers a variety of activities, these include: Surfing/Swimming, Sand Modelling, Talks by the guest speaker Rev. Chris Brennan of Glen Innes, Discussion Groups, Music and Competitions. There is an Extreme Team Challenge and many other things to do. Registration Forms have been distributed, but for anyone wanting more detailed information please contact one of the following: John Barwick phone 6785 1251, Robert Bowman 6785 7315, Rev. Vince Wall 6785 1627, Joe or Estelle McDonald 67697569.

#### XMAS LIGHTS REMINDER

A reminder that entries in the 2011 Christmas Lights competition close this Friday, 16th December. Entry form is published on page 5 of today's paper. Enter and you could be in the running for \$400 worth of Coles Myers vouchers courtesy of Essential Energy.

#### ENGAGED

Di and Rod Taylor are delighted to announce that Bede and Nicole are engaged.

#### 5 DAY WEATHER FORECAST

Wednesday - 12 to 24, partly cloudy.

Thursday - 12 to 23, possible afternoon shower.

Friday - 12 to 27, partly cloudy.

Saturday - 13 to 27, partly cloudy.

Sunday - 15 to 25, shower or two.

#### DAM LEVELS

**Keepit:** Storage 407,318 megalitres, 95.7 percent of capacity average discharge 3,631 megalitres per day. **Split Rock:** Storage 161,122 megalitres, 40.3 percent of capacity, discharge 10 megalitres per day.

## MANILLA FLOOD STUDIES - COMMUNITY INFORMATION SESSION

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Should you have any enquiries about the information session or the flood study, please contact William Ash, Senior Engineer, on 6767 5738 or 0417 964 972.



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**We Specialise in Full and Partial Dentures,  
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**Absolutely FREE and no obligation discussions.**

**Come in to chat as you may be eligible  
for some services you were not aware of.**

**We are not happy till you are.**

**Call Robin on 6778 3150**

**to get in early for your appointments.**

**NEWSPAPER INSTRUCTIONS  
REGIONAL COUNCIL NEWS ADS**

<b>PAPER</b>	<b>TO APPEAR (DATE)</b>	<b>JOB NUMBER</b>	<b>AUTHORISED BY</b>
BARRABA GAZETTE	WEDNESDAY 14 DECEMBER 2011	AUD085-1598	BILL ASH

**BARRABA FLOOD STUDIES – COMMUNITY INFORMATION SESSION**

Tamworth Regional Council has engaged consultants to carry out the Barraba Flood Study, to define flooding in Barraba and inform Council's planning for the management of flooding.

Residents who have questions about the study, or who have been affected by flooding previously, are invited to a Community Information Session being held at 6.00pm, 15 December at the Barraba Council Chambers.

Should you have any enquiries about the information session or the flood study, please contact William Ash, Senior Engineer, on 6767 5738 or 0417 964 972.



Dragons. Come along evening.

## THE BARRABA GAZETTE & CHRISTMAS

December publishing dates for the Gazette are:

Wednesday 7th, 14th, 21st December

The final paper for the year, 21st December, will include our traditional Business House "Christmas Wish" section. Anyone wishing to place an advertisement in this section of the paper is asked to contact the Gazette Office before Thursday, 15th December.

The first paper for 2012 will be published on Wednesday 18th January.

Inc is planning a meeting late January to begin the planning for Frost over Barraba. A notice for this meeting will be in the first Gazette of the new year.

*Shop Locally  
And Save!*



ABN: 52 631 074 450  
*More than just a city. More than just one place.*

## BARRABA FLOOD STUDIES – COMMUNITY INFORMATION SESSION

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Should you have any enquiries about the information session or the flood study, please contact William Ash, Senior Engineer, on 6767 5738 or 0417 964 972.

GROW CHOICE – SPONSORS OF THE BARRABA RUGBY CLUB

Grow Choice Grassup 20L, now on sale from  
CB Hart – Landmark Barraba.

*Come in for quality product, backed by friendly service and advice.*



Controls a range of woody and herbaceous weeds  
Australian made and owned

LANDMARK

CB Hart Landmark - 02 6782 1006  
135 Queen Street, Barraba

Grow Choice Technical Support - 1800 817 676



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FREE DELIVERY

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\*Half Leg Hams \$9.90kg.

\*Legs of Pork \$8.99kg.

\*Turkey Rolls \$16.50kg.

\*Butchers Own Hams \$12.85kg.

\*Pickled Legs Pork \$8.99kg.

Barraba Gazette, Wednesday, 14th December, 2011.3.

## **NEWSPAPER INSTRUCTIONS REGIONAL COUNCIL NEWS ADS**

<b>PAPER</b>	<b>TO APPEAR (DATE)</b>	<b>JOB NUMBER</b>	<b>AUTHORISED BY</b>
NORTHERN DAILY LEADER	SATURDAY 10 DECEMBER 2011	AUD085-1598	BILL ASH

### **BARRABA FLOOD STUDIES – COMMUNITY INFORMATION SESSION**

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**NEWSPAPER INSTRUCTIONS  
REGIONAL COUNCIL NEWS ADS**

<b>PAPER</b>	<b>TO APPEAR (DATE)</b>	<b>JOB NUMBER</b>	<b>AUTHORISED BY</b>
NORTHERN DAILY LEADER	SATURDAY 10 DECEMBER 2011	AUF010-1598-400	BILL ASH

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Should you have any enquiries about the information session or the flood study, please contact William Ash, Senior Engineer, on 6767 5738 or 0417 964 972.



**You're Invited... COUNCIL MEETING**  
The public is invited to attend Council's DECEMBER Meeting as under:

Meeting	Day	Date	Time	Venue
Ordinary Council	Tuesday	132 December 2011	6.30pm	Council Chambers 4th Floor Ray Walsh House 437 Peel Street Tamworth

**Community Consultation:** Members of the public may address the Council on matters and issues listed in the Business Paper. Each speaker addressing the Council Meeting will be allowed three minutes.

Members of the public giving prior notification to Council's Administration Staff by 12 noon on the day of the Meeting of their intention to address the Council Meeting will be given priority.

Council's Administration Staff will be pleased to assist with enquiries – telephone 6767 5444 during normal business hours.

**Business Papers are available from:**

Friday afternoon prior to meeting date on Tamworth Regional Council Website [www.tamworth.nsw.gov.au/Council](http://www.tamworth.nsw.gov.au/Council) OR

Monday Morning prior to Meeting for collection from Tamworth Regional Council Customer Services Counter at Barraba, Manilla, Nundle or Tamworth Offices and available at the Meeting at Tamworth.

**PART CLOSURE OF TAMWORTH REGIONAL COUNCIL THURSDAY 15 DECEMBER 2011**

Please be advised that all Tamworth Regional Council Offices and Facilities will be closed due to a Council Function as under:

Barraba Office will close on Thursday 15 December 2011 at 12:00pm  
Manilla Office will close on Thursday 15 December 2011 at 12:00pm  
Nundle Office will close on Thursday 15 December 2011 at 12:00pm  
Tamworth Office will close on Thursday 15 December 2011 at 12:00pm

The following services will remain open to the public during normal operating hours for each facility:

Tamworth Airport/Kiosk, Basil Brown Drive, Tamworth  
All Waste Depots which would be normally open  
Recycling Centres  
All Swimming Pool Complexes  
Marsupial Park Endeavour Drive, Tamworth  
Botanic Gardens, Piper Street, Tamworth  
East Year Round Care, Tamworth Public School, Napier Street, Tamworth  
South Year Round Care, Hillvue Public School, Tamworth  
Community Centre, Darling Street, Tamworth  
South Library, Robert Street, Tamworth  
Library/Regional Gallery, Peel Street, Tamworth  
Tamworth Sports Dome

For Emergency please phone 1300 733 625

**BARRABA FLOOD STUDIES – COMMUNITY INFORMATION SESSION**

Tamworth Regional Council has engaged consultants to carry out the Barraba Flood Study, to define flooding in Barraba and inform Council's planning for the management of flooding.

Residents who have questions about the study, or who have been affected by flooding previously, are invited to a Community Information Session being held at 6.00pm, 15 December at the Barraba Council Chambers.

Should you have any enquiries about the information session or the flood study, please contact William Ash, Senior Engineer, on 6767 5738 or 0417 964 972.

**MANILLA FLOOD STUDIES – COMMUNITY INFORMATION SESSION**

Tamworth Regional Council has engaged consultants to carry out the Manilla Flood Study, to define flooding in Manilla and inform Council's planning for the management of flooding.

Residents who have questions about the study, or who have been affected by flooding previously, are invited to a Community Information Session being held at 6.00pm, 14 December at the Manilla Town Hall.

Should you have any enquiries about the information session or the flood study, please contact William Ash, Senior Engineer, on 6767 5738 or 0417 964 972.

**EXHIBITION OF A DEVELOPMENT APPLICATION**

Notice is hereby given that a Development Application has been submitted to Council by Hibbards Pty Limited - Tamworth for Construction of a Single Storey Detached Dual Occupancy and Torrens Title Subdivision into two lots on Lot 134 DP 1153123, 4 Red Cedar Cove, Oxley Vale.

The application and accompanying plans will be exhibited in Council's Customer Services Section, Ray Walsh House, Peel Street, Tamworth, between 8.30am and 5.00pm, Monday to Friday, for a period of 14 days from Monday, 12 December 2011.

If you wish to advise Council of your views on the proposal, you should prepare a written submission, to be made prior to 5.00pm on Tuesday, 3 January 2012. Please Note: The exhibition period has been extended due to its coincidence with the festive season and Council offices being closed from Friday 23 December 2011 to Monday 2 January 2012 inclusive. The issues you raise will be included in the evaluation of the development application, along with the other matters Council must consider. Any submissions **must** include disclosure of any reportable political contribution or gift made in the previous two years.

It should be noted that you may request that your name and address not be disclosed (by stating prominently "OBJECTION IN CONFIDENCE" on your submission) for reason that disclosure would result in detriment to you, however, Council may be obliged to release details of your complaint excluding your personal information under the Government Information (Public Access) Act 2009 even if these words are used in the submission. Further, submissions that do not contain the author's name and address may not be considered as Council will be unable to validate their authenticity.

**EXHIBITION OF A DEVELOPMENT APPLICATION**

Notice is hereby given that a Development Application has been submitted to Council by Ms S Moore for the establishment of a Function Centre in the existing Wine Cellar on Lot 200 DP 1040669, 80 Wyndham Close, Daruka.

The application and accompanying plans will be exhibited in Council's Customer Services Section, Ray Walsh House, Peel Street, Tamworth, between 8.30am and 5.00pm, Monday to Friday from Monday, 12 December 2011.

If you wish to advise Council of your views on the proposal, you should prepare a written submission, to be made prior to 5.00pm on Monday, 9 January 2012. Please note, the exhibition period has been extended as Council offices will be closed for the Christmas New Year period from 26 December 2011 to 2 January 2012.

The issues you raise will be included in the evaluation of the development application, along with the other matters Council must consider. Any submissions **must** include disclosure of any reportable political contribution or gift made in the previous two years.

It should be noted that you may request that your name and address not be disclosed (by stating prominently "OBJECTION IN CONFIDENCE" on your submission) for reason that disclosure would result in detriment to you, however, Council may be obliged to release details of your complaint excluding your personal information under the Government

**Information for Readers:** TRC business information is available from council offices at Tamworth, Barraba, Nundle and Manilla. [www.tamworth.nsw.gov.au](http://www.tamworth.nsw.gov.au) All correspondence to TRC should be addressed to:

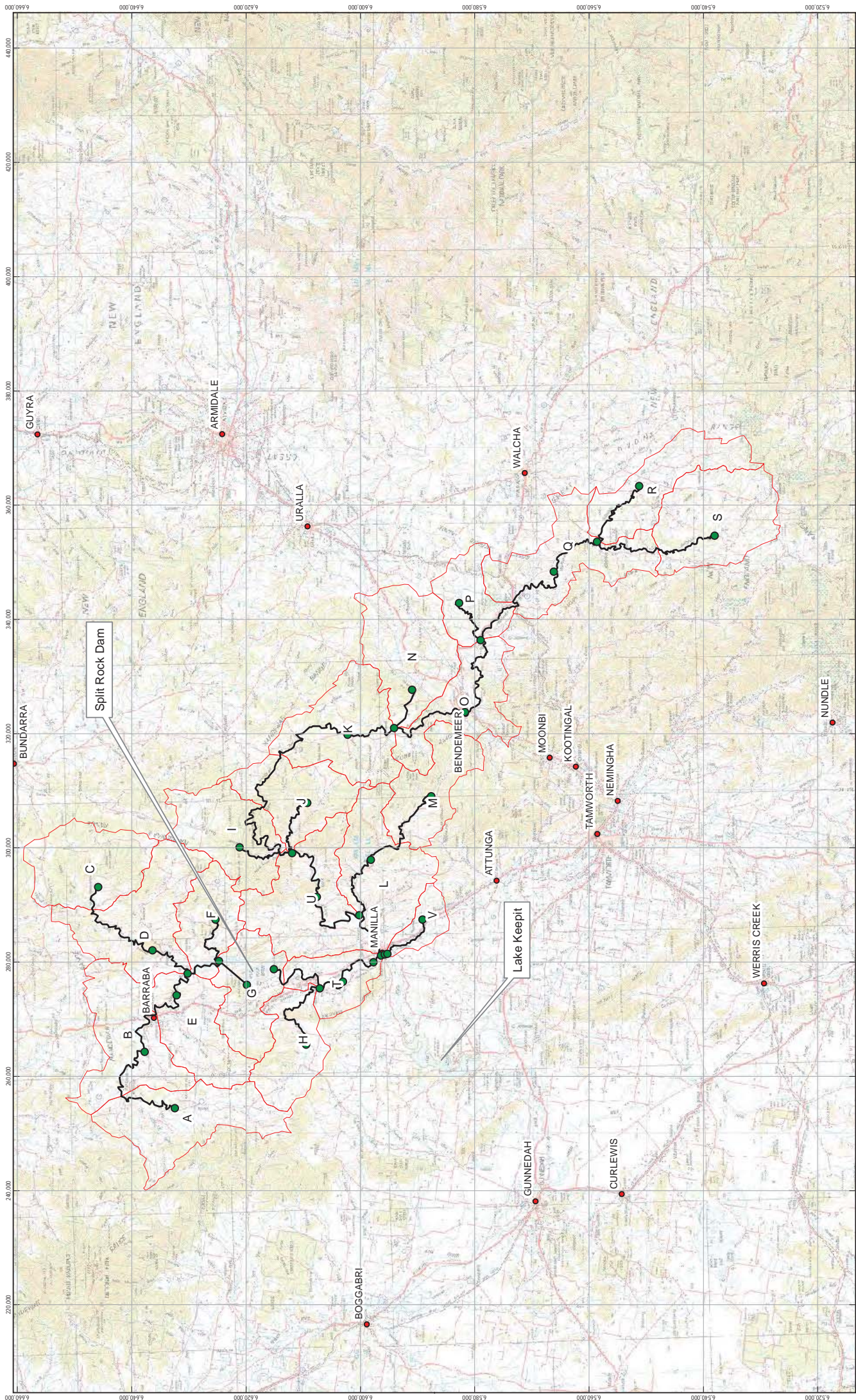
Northern Daily Leader 10 Dec 11



## Appendix D

# Hydrological Data (RORB)







Manilla - 1

C RORB\_GE 6.15

C WARNING - DO NOT EDIT THIS FILE OUTSIDE RORB TO ENSURE BOTH GRAPHICAL AND CATCHMENT DATA ARE COMPATIBLE WITH EACH OTHER

C THIS FILE CANNOT BE OPENED IN EARLIER VERSIONS OF RORB GE - CURRENT VERSION IS v6.15

C

C Manilla - 1

C

C #FILE COMMENTS

C 2

C File created using MiRORB version 1.1

C Original CATG file created on 9/01/2012 at 13:41:39

C

C #SUB-AREA AREA COMMENTS

C 1

C Sub-area areas in km2

C

C #IMPERVIOUS FRACTION COMMENTS

C 0

C

C #BACKGROUND IMAGE

C T F G:\22\15815\Tech\RORB\MANILLA 2012.12.11\MANILLA.wmf

C

C #NODES

C 36

C 1	28.658		76.977	1.000	1	0	2	A
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302.925	0.050	0	0					
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C

C 2	32.664		80.584	1.000	1	0	5	B
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286.801	0.050	0	0					
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C

C 3	44.440		86.087	1.000	1	0	4	D
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318.750	0.050	0	0					
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C

C 4	39.915		79.656	1.000	1	0	23	E
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223.710	0.050	0	0					
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C

C 5	36.716		76.791	1.000	1	0	23	C
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263.621	0.050	0	0					
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C

C 6	42.116		72.152	1.000	1	0	33	F
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159.620	0.050	0	0					
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C

C 7	37.467		68.406	1.000	1	0	33	G
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197.882	0.050	0	0					
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C

C 8	33.185		61.391	1.000	1	0	34	H
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186.251	0.050	0	0					
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C

C 9	47.280		69.282	1.000	1	0	29	Q
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245.543	0.050	0	0					
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C

C 10	50.448		61.234	1.000	1	0	29	R
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213.004	0.050	0	0					
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C

C 11	55.307		56.426	1.000	1	0	29	P
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293.431	0.050	0	0					
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C

C 12	46.395		53.674	1.000	1	0	30	U
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283.554	0.050	0	0			
C						
C 13	50.924			46.493	1.000	12 T
200.163	0.050	0	0			
C						
C 14	58.526			48.768	1.000	28 O
296.263	0.050	0	0			
C						
C 15	56.930			42.484	1.000	28 N
222.554	0.050	0	0			
C						
C 16	64.719			43.186	1.000	32 M
147.063	0.050	0	0			
C						
C 17	66.945			31.941	1.000	32 L
297.055	0.050	0	0			
C						
C 18	73.054			21.771	1.000	31 K
229.875	0.050	0	0			
C						
C 19	69.511			12.824	1.000	31 J
317.994	0.050	0	0			
C						
C 20	37.682			56.988	1.000	25 I
132.507	0.050	0	0			
C						
C 21	43.737			60.091	1.000	30 S
326.950	0.050	0	0			
C						
C 22	38.570			65.218	1.000	34 G1
0.000	0.000	0	1			
C						
C outlet node for storage						
C 23	38.263			75.470	1.000	33 F1
0.000	0.000	0	1			
C						
C confluence of ironbark creek, manilla river						
C 24	39.547			52.473	1.000	27 T1
0.000	0.000	0	1			
C						
C confluence at manilla						
C 25	39.074			53.360	1.000	24 T2
0.000	0.000	70	1			
C BRABRI						
C Upstream Manilla Junction - Manilla River						
C 26	40.741			53.193	1.000	24 U1
0.000	0.000	70	1			
C NORTH CUERINDI						
C Upstream Manilla Junction - Namoi River						
C 27	39.626			52.013	1.000	36 V2
0.000	0.000	70	1			
C RAILWAY						
C Outlet node						
C 28	55.788			50.916	1.000	11 O1
0.000	0.000	0	1			
C						
C confluence carlisle's gully - mcdonald river						
C 29	46.857			63.029	1.000	21 U2
0.000	0.000	0	1			
C						

C confluence Namoi River- Red Jack Mountain									
C	30	42.434	55.071	1.000	0	0	26	U3	
0.000		0.000	0	1					
C									
C Halls Creek - Namoi River Confluence									
C	31	69.083	26.805	1.000	0	0	17	R1	
0.000		0.000	0	0					
C									
C	32	62.071	40.650	1.000	0	0	15	P1	
0.000		0.000	0	0					
C									
C	33	39.155	71.739	1.000	0	0	22	G2	
0.000		0.000	70	0					
C BLKSPRNG									
C	34	37.218	59.749	1.000	0	0	20	T4	
0.000		0.000	0	0					
C									
C	35	42.121	47.551	1.000	1	0	36	V	
65.513		0.000	0	0					
C									
C	36	39.676	51.712	1.000	0	1	0	V1	
0.000		0.000	0	1					
C CREEK DS RAILWAY									
C Outlet node									
C									
C #REACHES									
C	34								
C	1 A-B		1	2		0	1	0	28.962
0.581	45	0							
C		28.831	28.688	28.557		28.653			
28.844		28.926	29.166	28.803		28.751			
28.874		29.178	28.911	29.025		29.163			
29.370		29.327	29.405	29.283		29.134			
29.130		29.319	29.414	29.490		29.844			
30.048		30.137	30.172	30.250		30.361			
30.806		31.021	31.214	31.666		31.733			
31.617		31.484	31.561	31.782		31.710			
31.928		32.096	32.208	32.220		32.339			
32.744									
C		77.594	77.637	77.978		78.060			
77.951		78.237	78.496	78.456		78.833			
79.417		79.678	79.987	80.255		80.747			
81.067		81.702	82.444	82.622		82.515			
82.719		82.807	83.047	83.363		83.326			
83.691		83.734	83.120	83.255		83.448			
83.569		83.434	83.107	82.731		82.520			
82.295		82.039	81.906	82.026		81.528			
81.383		81.832	81.783	81.540		81.388			
81.455									
C	2 B-C		2	5		0	1	0	20.324
0.000	31	0							
C		33.218	33.408	33.750		34.096			
34.352		34.401	34.573	34.663		34.767			
34.925		35.171	35.277	35.277		35.277			
35.375		35.776	36.002	35.858		35.820			
35.916		36.144	35.997	35.866		35.916			
35.758		35.922	36.047	36.112		36.292			
36.468		36.644							
C		80.733	81.012	80.951		81.692			
81.305		80.937	80.760	80.160		80.016			



80.050	80.635	80.473	80.091	79.335	
79.232	79.395	79.360	79.109	78.801	
78.413	78.074	77.950	77.786	77.486	
77.198	76.585	76.599	76.845	77.049	
77.022	77.008				
C	3 C-F1	5	23	0 1 0	8.452
0.000	14 0				
C	36.628	36.734	36.865	36.865	
36.960	37.281	37.413	37.479	37.576	
37.724	37.882	37.605	37.839	38.017	
C	76.558	76.367	76.408	76.667	
76.809	77.088	77.008	76.558	76.353	
76.449	76.329	75.552	75.382	75.819	
C	4 D-E	3	4	0 1 0	24.967
0.467	44 0				
C	44.231	44.004	43.797	43.537	
43.363	43.187	43.093	42.905	42.808	
42.676	42.502	42.529	42.463	42.357	
42.038	41.890	41.931	41.776	41.776	
41.657	41.563	41.596	41.672	41.465	
41.481	41.334	41.129	40.908	40.818	
40.841	40.818	40.651	40.573	40.597	
40.541	40.294	40.433	40.651	40.316	
40.447	40.204	39.951	39.806	39.975	
C	86.377	86.408	86.896	87.015	
86.664	86.578	86.609	86.841	86.773	
86.637	86.918	86.637	86.378	86.117	
86.473	86.201	85.860	85.600	85.068	
84.878	84.850	84.223	83.980	84.086	
83.623	83.473	83.650	83.432	83.023	
82.619	82.422	82.425	82.477	82.177	
81.933	81.863	81.769	81.672	81.357	
80.749	80.922	80.677	80.142	79.967	
C	5 E-F1	4	23	0 1 0	10.501
0.000	19 0				
C	39.803	39.861	39.806	39.588	
39.574	39.769	39.558	39.427	39.515	
39.386	39.247	39.124	38.961	38.757	
38.780	38.666	38.559	38.567	38.551	
C	79.381	79.122	78.952	78.515	
78.235	77.898	77.867	77.616	77.300	
77.199	77.403	77.103	76.766	77.033	
76.258	76.012	75.835	75.576	75.453	
C	6 T2-T1	25	24	0 1 0	1.917
0.000	5 0				
C	39.075	39.172	39.414	39.491	
39.504					
C	53.273	53.109	52.979	52.754	
52.602					
C	7 U1-T1	26	24	0 1 0	3.294
0.000	5 0				
C	40.688	40.385	39.988	39.715	
39.642					
C	53.126	52.876	52.679	52.754	
52.712					
C	8 T1-V2	24	27	0 1 0	0.706
0.000	1 0				
C	39.586				
C	52.243				
C	9 T-T2	20	25	0 1 0	9.559

0.000	27	0				
C		37.682	37.682	37.682	37.682	37.682
37.682		37.682	37.682	37.682	37.682	37.682
37.682		37.682	37.682	37.682	37.682	37.682
37.890		38.432	38.555	38.456		38.619
38.857		39.224	39.218	38.811		38.723
38.824		38.974	39.074			
C		56.988	56.988	56.988	56.988	56.988
56.988		56.988	56.988	56.988		56.988
56.988		56.988	56.988	56.988		56.988
57.131		57.014	56.500	56.238		55.957
56.064		55.133	54.755	54.319		53.989
53.725		53.601	53.360			
C	10	H-T4	8	34	0 1 0	19.558
2.633	43	0				
C		33.239	33.442	33.505	33.645	
33.622		33.744	33.913	34.099	34.320	
34.483		34.634	34.645	34.901	34.994	
34.960		34.936	35.061	35.197	35.309	
35.526		35.720	35.679	35.769	35.611	
35.675		35.797	35.820	36.036	36.184	
36.153		36.415	36.609	36.703	36.762	
36.748		36.821	36.888	36.914	36.915	
36.987		37.073	37.124	37.310		
C		61.613	61.688	61.755	61.628	
61.440		61.342	61.493	61.842	62.017	
62.288		62.288	62.618	62.734	62.845	
63.102		63.471	63.883	63.815	63.965	
64.010		63.785	63.604	63.371	63.063	
62.958		62.908	62.346	62.071	62.123	
61.717		61.514	61.446	61.371	61.198	
61.033		60.989	61.025	60.892	60.695	
60.589		60.567	60.853	60.717		
C	11	G1-T4	22	34	0 1 0	15.918
0.000	24	0				
C		38.426	38.531	38.432	37.844	
37.529		37.461	37.659	37.881	38.178	
38.368		38.530	38.584	38.431	38.287	
38.174		37.971	37.831	37.836	37.917	
37.998		37.980	38.237	38.030	37.592	
C		65.040	64.556	64.413	64.528	
64.288		64.071	63.625	63.529	63.002	
62.807		62.762	62.220	61.905	61.897	
61.815		61.935	61.709	61.409	61.288	
60.905		60.552	60.311	60.003	59.965	
C	12	J-R1	19	31	0 1 0	31.414
0.334	97	0				
C		69.564	69.437	69.381	69.311	
69.353		69.388	69.332	69.318	69.304	
69.247		69.381	69.367	69.325	69.254	
69.085		68.965	68.909	68.902	68.930	
68.980		68.895	68.768	68.775	68.642	
68.487		68.458	68.501	68.501	68.444	
68.416		68.409	68.501	68.487	68.465	
68.312		68.267	68.407	68.379	68.321	
68.398		68.342	68.240	68.164	68.206	
68.303		68.276	68.488	68.484	68.571	
68.627		68.527	68.409	68.296	68.332	
68.395		68.564	68.613	68.501	68.550	
68.479		68.430	68.444	68.374	68.366	

68.529	68.634	68.550	68.571	68.627
68.691	68.881	68.895	68.958	68.860
68.733	68.634	68.592	68.564	68.515
68.488	68.571	68.684	68.761	68.733
68.627	68.578	68.620	68.637	68.564
68.493	68.507	68.599	68.574	68.613
68.782	68.838	68.937		
C	13.123	13.370	13.252	13.370
13.487	13.605	13.769	13.921	14.074
14.215	14.438	14.684	14.766	14.872
14.919	14.966	15.036	15.236	15.353
15.494	15.588	15.717	15.870	16.280
16.292	16.327	16.445	16.609	16.926
17.055	17.172	17.372	17.618	17.752
17.627	17.778	17.815	18.055	18.078
18.454	18.671	18.477	18.514	18.668
18.755	18.928	19.168	19.409	19.484
19.766	19.820	19.989	20.142	20.541
20.811	21.022	21.280	21.468	21.761
21.961	22.184	22.407	22.653	22.747
22.817	22.794	23.005	23.275	23.428
23.592	23.522	23.768	23.956	24.132
23.944	24.050	24.097	24.355	24.578
24.716	24.813	24.625	24.648	25.012
25.212	25.235	25.364	25.468	25.529
25.588	25.763	25.928	26.287	26.338
26.526	26.432	26.679		
C	13 K-R1	18	31	0 1 0
0.761	48 0			19.506
C	72.965	72.796	72.726	72.648
72.747	72.740	72.606	72.409	72.331
72.275	72.198	72.240	72.141	71.958
71.853	71.923	71.916	71.810	71.768
71.648	71.543	71.719	71.662	71.515
71.451	71.367	71.254	71.059	71.198
71.092	70.956	70.856	70.739	70.726
70.698	70.641	70.578	70.590	70.465
70.240	70.085	69.888	69.712	69.599
69.514	69.381	69.268	69.106	
C	21.889	21.796	21.901	22.089
22.253	22.394	22.500	22.629	22.746
22.394	22.476	22.676	22.793	22.922
23.063	23.204	23.404	23.404	23.709
23.979	24.237	24.213	24.460	24.448
24.612	24.577	25.105	25.039	24.777
24.718	25.129	25.325	25.355	25.129
25.000	25.047	25.223	25.603	25.763
26.021	26.197	26.244	26.303	26.397
26.631	26.502	26.103	26.350	
C	14 O-O1	14	28	0 1 0
0.000	11 0			8.788
C	58.444	58.148	57.697	57.514
57.373	57.035	56.796	56.472	56.260
56.190	56.035			
C	48.439	48.627	48.979	49.355
49.707	49.379	49.473	49.613	49.989
50.153	50.435			
C	15 N-O1	15	28	0 1 0
0.000	46 0			17.884
C	56.959	56.982	57.028	57.050

57.156	57.171	57.073	56.906	56.831
56.785	56.853	56.861	56.748	56.808
56.838	56.861	56.944	56.952	56.884
56.800	56.800	56.724	56.619	56.580
56.429	56.245	56.103	55.990	55.920
55.974	55.906	55.740	55.690	55.732
55.776	55.712	55.702	55.580	55.247
55.126	55.144	55.338	55.406	55.541
55.686	55.762			
C	42.721	42.936	43.087	43.378
43.504	43.693	43.895	43.971	44.085
44.299	44.539	44.691	44.885	45.133
45.335	45.499	45.600	45.878	46.105
46.357	46.660	46.812	46.873	47.128
47.582	47.625	47.506	47.456	47.580
47.734	47.923	48.264	48.445	48.617
48.700	48.780	48.870	48.971	48.883
48.996	49.211	49.317	49.700	49.903
49.893	50.208			
C	16 T-U	13	12	0 1 0
1.029	43 0			22.067
C	50.912	50.992	50.918	50.827
50.822	50.867	50.782	50.606	50.527
50.533	50.431	50.317	50.232	50.125
50.011	49.575	49.416	49.116	49.065
48.934	48.781	48.747	48.577	48.521
48.209	48.237	47.914	47.869	47.897
47.534	47.319	47.251	47.155	47.104
47.166	47.387	47.330	47.308	47.075
46.837	46.729	46.718	46.661	
C	46.716	46.877	47.028	46.972
47.028	47.198	47.302	47.501	47.482
47.246	47.189	47.633	47.718	48.048
48.086	48.644	49.022	49.201	49.664
50.004	49.976	50.382	50.259	50.051
50.231	49.938	49.749	49.985	50.231
50.231	50.326	50.552	50.401	50.562
50.864	51.478	51.676	51.931	52.678
53.235	53.329	53.651	53.717	
C	17 U-U3	12	30	0 1 0
0.000	34 0			16.003
C	46.304	45.930	45.998	45.624
45.409	45.375	45.329	45.397	45.261
45.125	45.069	45.159	45.080	44.887
44.740	44.536	44.524	44.354	44.298
44.173	44.094	44.003	43.878	43.697
43.425	43.346	43.198	43.198	42.915
42.869	42.801	42.688	42.575	42.563
C	54.161	54.029	54.444	54.841
54.709	54.935	55.049	55.389	55.616
55.502	55.672	55.880	55.918	55.956
55.937	55.956	55.786	55.710	55.994
55.880	55.918	55.748	55.729	55.408
55.276	54.841	54.841	54.615	54.445
54.558	54.350	54.634	54.709	55.031
C	18 R-U2	10	29	0 1 0
3.366	49 0			15.125
C	50.334	50.283	50.323	50.289
50.215	50.192	50.226	50.130	50.102
50.028	49.982	49.858	49.790	49.625

49.625	49.484	49.376	49.342	49.370	
49.404	49.291	49.132	49.132	49.013	
48.883	48.781	48.605	48.469	48.424	
48.299	48.327	48.180	48.106	48.061	
48.072	47.942	47.908	47.846	47.766	
47.647	47.670	47.591	47.245	47.273	
47.177	47.103	47.143	47.149	47.052	
C	61.272	61.405	61.594	61.764	
61.839	61.971	62.217	62.293	62.434	
62.293	62.500	62.397	62.264	62.236	
62.519	62.623	62.699	62.841	62.982	
63.143	63.152	63.266	63.171	63.162	
63.304	63.389	63.332	63.247	63.153	
63.039	63.398	63.304	63.398	63.521	
63.663	63.710	63.559	63.540	63.719	
63.682	63.512	63.474	63.380	63.181	
63.172	63.219	63.361	63.436	63.408	
C	19 U2-S	29	21	0 1 0	15.291
0.000	30 0				
C	47.029	46.781	46.609	46.582	
46.663	46.808	46.763	46.718	46.524	
46.627	46.542	46.325	46.127	45.930	
45.775	45.635	45.459	44.954	44.873	
44.895	44.990	44.918	44.587	44.435	
44.381	44.300	44.372	44.345	44.191	
44.047					
C	62.715	62.158	62.151	61.993	
61.707	61.610	61.301	61.241	61.076	
60.625	60.505	60.550	60.738	60.886	
60.452	60.332	60.309	60.768	60.716	
60.272	59.926	59.813	59.977	59.941	
59.814	59.994	60.047	60.159	60.197	
60.024					
C	20 S-U3	21	30	0 1 0	17.908
0.000	31 0				
C	43.379	43.243	43.028	42.994	
42.643	42.139	42.116	42.044	41.968	
41.962	41.872	41.724	41.656	41.683	
41.634	41.611	41.535	41.543	41.498	
41.861	41.968	42.044	42.042	41.991	
42.127	42.297	42.495	42.654	42.615	
42.501	42.291				
C	60.167	60.396	60.188	59.621	
59.470	59.453	59.190	59.152	59.280	
59.583	59.735	59.735	59.527	59.265	
59.025	58.784	58.559	58.375	57.978	
58.053	57.624	57.235	56.967	56.306	
56.164	56.249	56.646	56.674	56.387	
55.767	55.456				
C	21 U3-U1	30	26	0 1 0	6.574
0.000	16 0				
C	42.456	42.274	42.110	42.002	
41.895	41.668	41.566	41.481	41.481	
41.299	41.203	41.180	40.857	40.741	
40.741	40.741				
C	54.794	54.539	54.511	54.766	
54.955	55.002	54.917	54.672	54.417	
54.048	53.727	53.548	53.312	53.193	
53.193	53.193				
C	22 O-U2	9	29	0 1 0	13.166



1.451	28	0				
C	47.041		46.961	47.007	46.831	
46.667	46.633		46.610	46.474	46.633	
46.542	46.519		46.406	46.565	46.599	
46.463	46.570		46.655	46.718	46.627	
46.650	46.871		46.842	46.950	46.695	
46.740	46.905		46.882	46.781		
C	68.877		68.518	68.272	68.282	
68.140	67.895		67.536	67.271	67.082	
66.988	66.705		66.478	66.289	66.081	
65.911	65.845		65.987	65.807	65.628	
65.439	65.193		64.825	64.589	64.438	
64.116	63.937		63.408	63.233		
C	23	01-K	28	11	0 1 0	11.485
0.000	17	0				
C	55.249		55.353	55.609	55.921	
55.774	55.787		55.579	55.554	55.652	
55.603	55.237		55.176	55.298	55.188	
55.298	55.188		55.298			
C	51.844		52.017	51.976	52.404	
52.566	52.913		53.014	53.238	53.462	
53.666	53.992		54.480	54.643	54.969	
55.233	55.742		55.987			
C	24	P-U2	11	29	0 1 0	58.472
1.335	123	0				
C	55.473		55.523	55.397	55.685	
55.807	55.725		55.708	56.015	56.116	
55.982	55.807		55.785	55.757	55.909	
55.921	55.835		55.860	55.395	55.310	
55.067	55.004		54.885	54.918	54.797	
54.715	54.639		54.565	54.406	54.247	
54.242	54.180		54.100	54.036	53.978	
54.125	54.064		53.935	53.741	53.624	
53.538	53.425		53.282	53.187	53.028	
52.818	52.359		52.239	52.182	52.135	
52.020	51.924		51.932	51.806	51.730	
51.688	51.747		51.755	51.675	51.530	
51.449	51.242		50.985	50.900	50.863	
50.570	50.558		50.130	50.106	49.984	
50.057	49.974		49.898	49.762	49.667	
49.568	49.577		49.519	49.434	49.379	
49.311	49.165		49.077	48.860	48.903	
48.959	48.959		49.040	49.040	48.901	
48.607	48.486		48.346	48.238	48.134	
48.035	47.931		47.678	47.570	47.457	
47.507	47.737		47.904	47.931	48.021	
48.084	48.093		47.976	47.728	47.406	
47.247	47.076		47.101	47.394	47.479	
47.430	47.247		47.088	46.950	46.695	
46.740	46.905		46.882	46.835		
C	56.887		57.187	57.654	57.759	
58.052	58.287		58.443	58.488	58.613	
59.020	58.924		59.142	59.533	59.794	
60.588	60.791		60.954	61.789	62.583	
62.127	61.809		61.911	62.097	62.542	
62.713	62.750		62.787	62.909	63.214	
63.382	63.474		63.784	63.998	64.191	
64.558	64.639		64.450	64.495	64.415	
64.668	64.713		64.924	64.989	65.097	
65.128	65.790		65.871	66.044	66.247	

66.264	66.394	66.555	66.563	66.655	
66.856	67.034	67.205	67.202	67.370	
67.429	67.124	67.103	66.839	66.594	
66.493	66.045	66.371	66.717	66.839	
67.165	67.398	67.918	67.946	68.142	
68.162	67.909	67.714	67.694	67.751	
68.142	68.549	68.623	68.529	68.380	
68.187	67.901	67.706	67.465	67.473	
67.300	67.563	67.616	67.466	67.496	
67.646	67.684	67.225	67.240	67.150	
66.789	66.721	66.834	66.466	66.376	
66.225	65.962	65.947	66.143	66.269	
65.944	65.822	65.374	65.007	64.681	
64.437	64.478	64.682	64.589	64.438	
64.116	63.937	63.408	63.193		
C	25 F1-G2	23	33	0 1 0	8.170
0.000	10 0				
C	38.246	38.681	38.836	38.709	
38.844	38.794	38.971	38.939	38.734	
38.844					
C	74.645	74.634	74.328	73.880	
73.480	72.843	72.608	72.290	71.994	
71.721					
C	26 G-G2	7	33	0 1 0	6.423
0.000	1 0				
C	38.311				
C	70.072				
C	27 R1-L	31	17	0 1 0	13.182
0.000	87 0				
C	69.112	69.101	68.987	68.904	
68.867	68.855	68.883	68.929	68.951	
68.985	68.993	68.970	68.955	68.962	
69.082	69.201	69.232	69.247	69.244	
69.225	69.145	69.097	69.072	69.094	
69.129	69.129	69.116	69.083	69.043	
69.016	68.994	68.995	68.989	68.981	
68.966	68.923	68.889	68.890	68.901	
68.923	68.941	68.946	68.941	68.923	
68.883	68.839	68.786	68.735	68.692	
68.672	68.674	68.697	68.703	68.655	
68.618	68.465	68.469	68.422	68.395	
68.383	68.383	68.372	68.340	68.304	
68.283	68.217	68.153	68.146	68.094	
68.079	68.018	67.976	67.926	67.879	
67.817	67.789	67.774	67.742	67.700	
67.647	67.601	67.543	67.460	67.375	
67.246	67.163	67.137			
C	26.844	26.934	26.999	27.048	
27.087	27.159	27.226	27.285	27.312	
27.424	27.521	27.566	27.615	27.687	
27.842	27.856	27.881	27.919	27.975	
28.045	28.087	28.132	28.173	28.218	
28.285	28.366	28.411	28.438	28.438	
28.483	28.578	28.657	28.744	28.800	
28.870	28.915	28.956	28.996	29.055	
29.091	29.122	29.192	29.250	29.311	
29.374	29.424	29.484	29.536	29.567	
29.608	29.657	29.693	29.741	29.808	
29.864	30.035	30.343	30.429	30.440	
30.413	30.366	30.330	30.339	30.380	

30.420	30.994	31.081	31.176	31.270
31.362	31.455	31.506	31.547	31.590
31.599	31.551	31.488	31.423	31.392
31.394	31.457	31.529	31.565	31.583
31.520	31.554	31.626		
C	28 L-P1	17	32	0 1 0
0.000	133 0			28.839
C	66.893	66.874	66.866	66.855
66.825	66.791	66.613	66.544	66.535
66.531	66.465	66.423	66.378	66.330
66.132	66.082	66.002	65.961	65.928
65.871	65.852	65.843	65.789	65.816
65.775	65.822	65.957	65.965	65.965
65.943	65.903	65.832	65.781	65.763
65.766	65.828	65.959	66.046	66.103
66.149	66.206	66.238	66.261	66.183
66.094	66.029	65.978	65.945	65.891
65.856	65.802	65.799	65.956	66.069
66.080	66.077	66.021	65.910	65.856
65.824	65.764	65.710	65.702	65.740
65.802	65.888	65.967	65.932	65.829
65.764	65.716	65.573	65.570	65.508
65.451	65.305	65.260	65.103	64.789
64.708	64.677	64.694	64.734	64.725
64.652	64.554	64.497	64.489	64.428
64.474	64.434	64.336	64.339	64.371
64.371	64.342	64.274	64.288	64.299
64.220	64.115	64.056	64.002	63.975
63.953	63.880	63.824	63.794	63.721
63.662	63.581	63.527	63.494	63.456
63.388	63.295	63.249	62.947	62.917
62.929	62.929	62.919	62.878	62.810
62.726	62.667	62.460	62.360	62.316
62.282	62.262	62.184	62.130	
C	31.934	31.889	31.786	31.693
31.597	31.527	31.480	31.581	31.718
31.790	31.905	31.932	31.907	31.853
31.828	31.786	31.700	31.667	31.669
31.707	31.790	31.878	32.026	32.186
32.337	32.434	32.519	32.604	32.660
32.661	32.621	32.637	32.699	32.787
32.899	33.062	33.149	33.385	33.383
33.322	33.270	33.268	33.315	33.484
33.596	33.713	33.749	33.749	33.844
33.898	33.949	34.078	34.415	34.536
34.586	34.631	34.671	34.698	34.712
34.698	34.743	34.739	34.865	34.923
34.928	35.045	35.189	35.346	35.445
35.495	35.495	35.657	35.850	35.958
35.976	36.151	36.318	36.507	36.800
36.703	36.609	36.502	36.422	36.328
36.281	36.131	36.145	36.399	36.572
36.799	37.015	37.222	37.344	37.402
37.470	37.569	37.668	37.839	37.951
38.059	38.037	38.068	38.144	38.257
38.347	38.383	38.365	38.432	38.500
38.522	38.500	38.576	38.815	38.977
39.161	39.358	39.407	39.388	39.438
39.639	39.639	39.851	40.044	40.162
40.230	40.344	40.443	40.401	40.428

40.488	40.579	40.648	40.644		
C 29 M-P1		16	32	0 1 0	12.190
2.029 91 0					
C	64.669	64.660	64.656	64.656	
64.637	64.606	64.592	64.560	64.492	
64.410	64.328	64.296	64.296	64.301	
64.301	64.287	64.241	64.200	64.191	
64.191	64.178	64.155	64.114	64.078	
64.014	63.968	63.991	64.009	64.059	
64.073	64.091	64.041	63.996	63.914	
63.918	63.946	63.946	63.923	63.905	
63.868	63.850	63.836	63.809	63.786	
63.755	63.750	63.750	63.736	63.700	
63.673	63.659	63.663	63.659	63.627	
63.577	63.536	63.500	63.468	63.486	
63.486	63.427	63.377	63.286	63.277	
63.268	63.195	63.136	63.054	62.999	
62.958	62.922	62.890	62.849	62.822	
62.808	62.776	62.721	62.676	62.699	
62.653	62.608	62.535	62.498	62.453	
62.394	62.344	62.294	62.271	62.193	
62.143	62.093				
C	43.292	43.398	43.489	43.543	
43.611	43.641	43.550	43.444	43.353	
43.262	43.156	43.110	43.019	42.936	
42.868	42.822	42.799	42.784	42.761	
42.708	42.663	42.579	42.542	42.504	
42.481	42.451	42.390	42.329	42.299	
42.246	42.193	42.177	42.170	42.147	
42.011	41.942	41.897	41.813	41.753	
41.730	41.753	41.791	41.829	41.859	
41.844	41.806	41.745	41.700	41.662	
41.677	41.730	41.813	41.889	41.942	
41.950	41.897	41.844	41.783	41.738	
41.715	41.707	41.685	41.571	41.518	
41.465	41.374	41.343	41.245	41.169	
41.116	41.017	40.941	40.941	40.994	
41.116	41.199	41.305	41.419	41.533	
41.571	41.548	41.525	41.457	41.442	
41.427	41.328	41.230	41.154	40.987	
40.919	40.896				
C 30 P1-N		32	15	0 1 0	21.785
0.000 126 0					
C	62.030	61.975	61.952	61.934	
61.932	61.927	61.884	61.834	61.779	
61.752	61.718	61.627	61.543	61.470	
61.324	61.245	61.217	61.185	61.154	
61.076	61.058	61.028	60.994	60.933	
60.794	60.824	60.810	60.890	60.867	
60.846	60.853	60.814	60.801	60.753	
60.596	60.431	60.409	60.398	60.389	
60.368	60.342	60.290	60.263	60.190	
60.052	60.004	59.999	60.017	60.057	
60.091	60.134	60.133	60.114	60.059	
59.944	59.910	59.875	59.851	59.833	
59.798	59.759	59.706	59.664	59.612	
59.572	59.542	59.532	59.531	59.526	
59.521	59.506	59.473	59.438	59.416	
59.394	59.386	59.389	59.382	59.368	
59.346	59.303	59.247	59.207	59.163	

59.142	59.129	59.128	59.111	59.087
59.060	59.036	58.995	58.963	58.946
58.907	58.879	58.854	58.810	58.773
58.744	58.728	58.656	58.605	58.562
58.292	58.139	58.012	57.905	57.882
57.750	57.646	57.564	57.436	57.436
57.354	57.345	57.259	57.266	57.238
57.197	57.193	57.170	57.031	56.977
56.949	56.930			
C	40.577	40.547	40.513	40.316
40.191	40.138	40.085	40.081	40.141
40.282	40.316	40.267	40.111	40.062
39.935	39.859	39.848	39.874	39.950
40.083	40.087	40.018	40.056	40.170
40.265	40.360	40.458	40.599	40.697
40.758	40.856	41.031	41.088	41.213
41.262	41.472	41.531	41.673	41.709
41.734	41.709	41.629	41.597	41.599
41.749	41.681	41.599	41.538	41.468
41.349	41.172	41.085	41.043	40.987
40.943	40.902	40.804	40.733	40.669
40.629	40.618	40.622	40.661	40.680
40.661	40.637	40.589	40.542	40.504
40.436	40.415	40.405	40.406	40.459
40.523	40.574	40.625	40.680	40.718
40.775	40.817	40.833	40.860	40.924
40.987	41.064	41.137	41.197	41.258
41.311	41.339	41.326	41.243	41.188
41.059	40.958	40.888	40.894	40.892
40.917	40.974	41.184	41.239	41.349
41.343	41.306	41.268	40.962	40.765
40.655	40.777	40.777	40.906	41.004
41.103	41.228	41.440	41.565	41.641
41.687	41.770	41.816	41.886	41.924
42.023	42.484			
C	31 F-G2	6	33	0 1 0
1.992	68 0			12.680
C	42.011	41.861	41.834	41.734
41.697	41.656	41.615	41.524	41.388
41.319	41.160	41.128	41.107	41.068
41.065	41.063	41.041	40.980	40.919
40.868	40.776	40.756	40.745	40.714
40.711	40.680	40.679	40.642	40.521
40.429	40.343	40.257	40.213	40.170
40.157	40.142	40.111	40.086	40.087
40.075	40.055	40.016	39.983	39.972
40.006	40.047	40.074	40.071	40.027
40.021	40.074	40.039	39.978	39.873
39.881	39.837	39.837	39.775	39.714
39.690	39.559	39.484	39.418	39.358
39.285	39.220	39.234	39.208	
C	72.198	72.115	71.933	71.887
71.826	71.864	71.948	71.986	72.206
72.426	72.486	72.555	72.571	72.529
72.470	72.421	72.402	72.425	72.449
72.497	72.637	72.736	72.882	72.958
73.096	73.140	73.335	73.396	73.331
73.420	73.420	73.470	73.475	73.456
73.379	73.324	73.280	73.246	73.182
73.121	73.051	73.034	73.024	72.973

72.848	72.717	72.618	72.467	72.432	
72.373	72.155	72.151	72.170	72.081	
71.958	71.920	71.774	71.817	71.889	
71.952	72.062	72.049	72.089	72.166	
72.284	72.204	72.022	71.817		
C	32 T4-T	34	20	0 1 0	6.266
0.000	20 0				
C	37.159	37.195	37.245	37.363	
37.481	37.586	37.636	37.645	37.604	
37.522	37.441	37.391	37.359	37.268	
37.268	37.186	37.186	37.263	37.368	
37.591					
C	59.522	59.371	59.303	59.287	
59.250	59.060	58.826	58.606	58.477	
58.424	58.424	58.341	58.250	58.212	
58.008	57.720	57.463	57.251	57.114	
57.092					
C	33 V3-V1	35	36	0 1 1	11.010
1.143	31 0				
C	41.973	41.868	41.749	41.590	
41.508	41.248	41.202	41.170	41.047	
40.960	40.919	40.824	40.646	40.678	
40.623	40.623	40.436	40.450	40.286	
40.272	40.240	40.250	40.214	40.109	
40.045	39.995	39.936	39.849	39.769	
39.724	39.676				
C	47.620	47.567	47.445	47.422	
47.339	47.399	47.491	47.673	47.810	
47.931	48.144	48.243	48.456	48.707	
48.805	48.927	49.064	49.353	49.505	
50.006	50.166	51.024	50.979	51.287	
51.382	51.298	51.355	51.488	51.480	
51.686	51.712				
C	34 V2-V1	27	36	0 1 0	0.460
0.000	1 0				
C	39.676				
C	51.712				
C					
C	#STORAGES				
C	1				
C	1 SPLIT ROCK DAM	33	22	39.020	69.943
1.000	3 0.000	2 1 0 0 0 0			
C	449.000	96.500			
C	462.890	484.000			
C	2.000	0.000	0.000	0	
C	0.000	0.000	1	0.000	0.000
0.000	0				
C	34				
C	397.00	398.00	400.00		
402.00	404.00	406.00	408.00		
410.00	412.00	414.00	416.00		
418.00	420.00	422.00	424.00		
426.00	428.00	430.00	432.00		
434.00	436.00	438.00	440.00		
442.00	444.00	446.00	448.00		
450.00	452.00	454.00	456.00		
458.00	460.00	462.90			
C	0.00	24000.00	197000.00		
514000.00	1060000.00	1890000.00	3160000.00		
5240000.00	8440000.00	12800000.00	18400000.00		



25600000.00	34700000.00	45500000.00	57900000.00
72000000.00	88000000.00	106000000.00	127000000.00
150000000.00	175000000.00	202000000.00	232000000.00
264000000.00	299000000.00	336000000.00	376000000.00
419000000.00	465000000.00	515000000.00	568000000.00
625000000.00	685000000.00	790000000.00	

C

C #INFLOW/OUTFLOW

C 0

C

C END RORB\_GE

C

C File created using MiRORB version 1.1

C Original CATG file created on 9/01/2012 at 13:41:39

1

1, 19.506, -99 ,Reach 13 node 18

Sub-area K, Reach K-R1 - Generate rainfall excess h'graph and route downstream

3

Store running hydrograph

1, 31.414, -99 ,Reach 12 node 19

Sub-area J, Reach J-R1 - Generate rainfall excess h'graph and route downstream

4

Add running h'graph to last stored h'graph

5, 13.182, -99 ,Reach 27

Reach R1-L - Route running h'graph downstream

2, 28.839, -99 ,Reach 28 node 17

Sub-area L, Reach L-P1 - Generate rainfall excess h'graph, add to running h'graph, and route downstream

3

Store running hydrograph

1, 12.190, -99 ,Reach 29 node 16

Sub-area M, Reach M-P1 - Generate rainfall excess h'graph and route downstream

4

Add running h'graph to last stored h'graph

5, 21.785, -99 ,Reach 30

Reach P1-N - Route running h'graph downstream

2, 17.884, -99 ,Reach 15 node 15

Sub-area N, Reach N-O1 - Generate rainfall excess h'graph, add to running h'graph, and route downstream

3

Store running hydrograph

1, 8.788, -99 ,Reach 14 node 14

Sub-area O, Reach O-O1 - Generate rainfall excess h'graph and route downstream

4

Add running h'graph to last stored h'graph

5, 11.485, -99 ,Reach 23

Reach O1-K - Route running h'graph downstream

2, 58.472, -99 ,Reach 24 node 11

Sub-area P, Reach P-U2 - Generate rainfall excess h'graph, add to running h'graph, and route downstream

3

Store running hydrograph

1, 13.166, -99 ,Reach 22 node 9

Sub-area Q, Reach O-U2 - Generate rainfall excess h'graph and route downstream

4

```

Add running h'graph to last stored h'graph
3
Store running hydrograph
1, 15.125, -99 ,Reach 18 node 10
Sub-area R, Reach R-U2 - Generate rainfall excess h'graph and route
downstream
4
Add running h'graph to last stored h'graph
5, 15.291, -99 ,Reach 19
Reach U2-S - Route running h'graph downstream
2, 17.908, -99 ,Reach 20 node 21
Sub-area S, Reach S-U3 - Generate rainfall excess h'graph, add to running
h'graph, and route downstream
3
Store running hydrograph
1, 22.067, -99 ,Reach 16 node 13
Sub-area T, Reach T-U - Generate rainfall excess h'graph and route
downstream
2, 16.003, -99 ,Reach 17 node 12
Sub-area U, Reach U-U3 - Generate rainfall excess h'graph, add to running
h'graph, and route downstream
4
Add running h'graph to last stored h'graph
5, 6.574, -99 ,Reach 21
Reach U3-U1 - Route running h'graph downstream
C Upstream Manilla Junction - Namoi River
7
PRINT
NORTH CUERINDI
5, 3.294, -99 ,Reach 7
Reach U1-T1 - Route running h'graph downstream
3
Store running hydrograph
1, 28.962, -99 ,Reach 1 node 1
Sub-area A, Reach A-B - Generate rainfall excess h'graph and route
downstream
2, 20.324, -99 ,Reach 2 node 2
Sub-area B, Reach B-C - Generate rainfall excess h'graph, add to running
h'graph, and route downstream
2, 8.452, -99 ,Reach 3 node 5
Sub-area C, Reach C-F1 - Generate rainfall excess h'graph, add to running
h'graph, and route downstream
3
Store running hydrograph
1, 24.967, -99 ,Reach 4 node 3
Sub-area D, Reach D-E - Generate rainfall excess h'graph and route
downstream
2, 10.501, -99 ,Reach 5 node 4
Sub-area E, Reach E-F1 - Generate rainfall excess h'graph, add to running
h'graph, and route downstream
4
Add running h'graph to last stored h'graph
5, 8.170, -99 ,Reach 25
Reach F1-G2 - Route running h'graph downstream
3
Store running hydrograph
1, 12.680, -99 ,Reach 31 node 6
Sub-area F, Reach F-G2 - Generate rainfall excess h'graph and route
downstream
4

```

Add running h'graph to last stored h'graph

3

Store running hydrograph

1, 6.423, -99 ,Reach 26 node 7

Sub-area G, Reach G-G2 - Generate rainfall excess h'graph and route downstream

4

Add running h'graph to last stored h'graph

7

PRINT

BLKSPRNG

16

Storage

SPLIT ROCK DAM

3, .000, 2, Spillway data (2 values x 2 spillways)

449.000, 96.500,

462.890, 484.000,

2.00, -99

C Elevation-storage relationship

1, 34, Elevation-storage table (2 values x 34 lines)

397.000, 0.000,

398.000, 24000.000,

400.000, 197000.000,

402.000, 514000.000,

404.000, 1060000.000,

406.000, 1890000.000,

408.000, 3160000.000,

410.000, 5240000.000,

412.000, 8440000.000,

414.000, 12800000.000,

416.000, 18400000.000,

418.000, 25600000.000,

420.000, 34700000.000,

422.000, 45500000.000,

424.000, 57900000.000,

426.000, 72000000.000,

428.000, 88000000.000,

430.000, 106000000.000,

432.000, 127000000.000,

434.000, 150000000.000,

436.000, 175000000.000,

438.000, 202000000.000,

440.000, 232000000.000,

442.000, 264000000.000,

444.000, 299000000.000,

446.000, 336000000.000,

448.000, 376000000.000,

450.000, 419000000.000,

452.000, 465000000.000,

454.000, 515000000.000,

456.000, 568000000.000,

458.000, 625000000.000,

460.000, 685000000.000,

462.900, 790000000.000,

-99

5, 15.918, -99 ,Reach 11

Reach G1-T4 - Route running h'graph downstream

3

Store running hydrograph

1, 19.558, -99 ,Reach 10 node 8

```

Sub-area H, Reach H-T4 - Generate rainfall excess h'graph and route
downstream
4
Add running h'graph to last stored h'graph
5, 6.266, -99
Reach T4-T - Route running h'graph downstream
2, 9.559, -99
Sub-area I, Reach T-T2 - Generate rainfall excess h'graph, add to running
h'graph, and route downstream
C Upstream Manilla Junction - Manilla River
7
PRINT
BRABRI
5, 1.917, -99
Reach T2-T1 - Route running h'graph downstream
4
Add running h'graph to last stored h'graph
5, .706, -99
Reach T1-V2 - Route running h'graph downstream
C Outlet node
7
PRINT
RAILWAY
5, .460, -99
Reach V2-V1 - Route running h'graph downstream
3
Store running hydrograph
11, 11.010, -99
Sub-area V, Reach V3-V1 - Generate rainfall excess h'graph and route
downstream
4
Add running h'graph to last stored h'graph
0
C Sub-area areas in km2
229.875, 317.994, 297.055, 147.063, 222.554,
296.263, 293.431, 245.543, 213.004, 326.950,
200.163, 283.554, 302.925, 286.801, 263.621,
318.750, 223.710, 159.620, 197.882, 186.251,
132.507, 65.513,
-99
C Impervious Fraction Data
1
0.050, 0.050, 0.050, 0.050, 0.050,
0.050, 0.050, 0.050, 0.050, 0.050,
0.050, 0.050, 0.050, 0.050, 0.050,
0.050, 0.050, 0.050, 0.050, 0.050,
0.050, 0.000,
-99

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## Appendix E

# Hydraulic Data (TUFLOW)





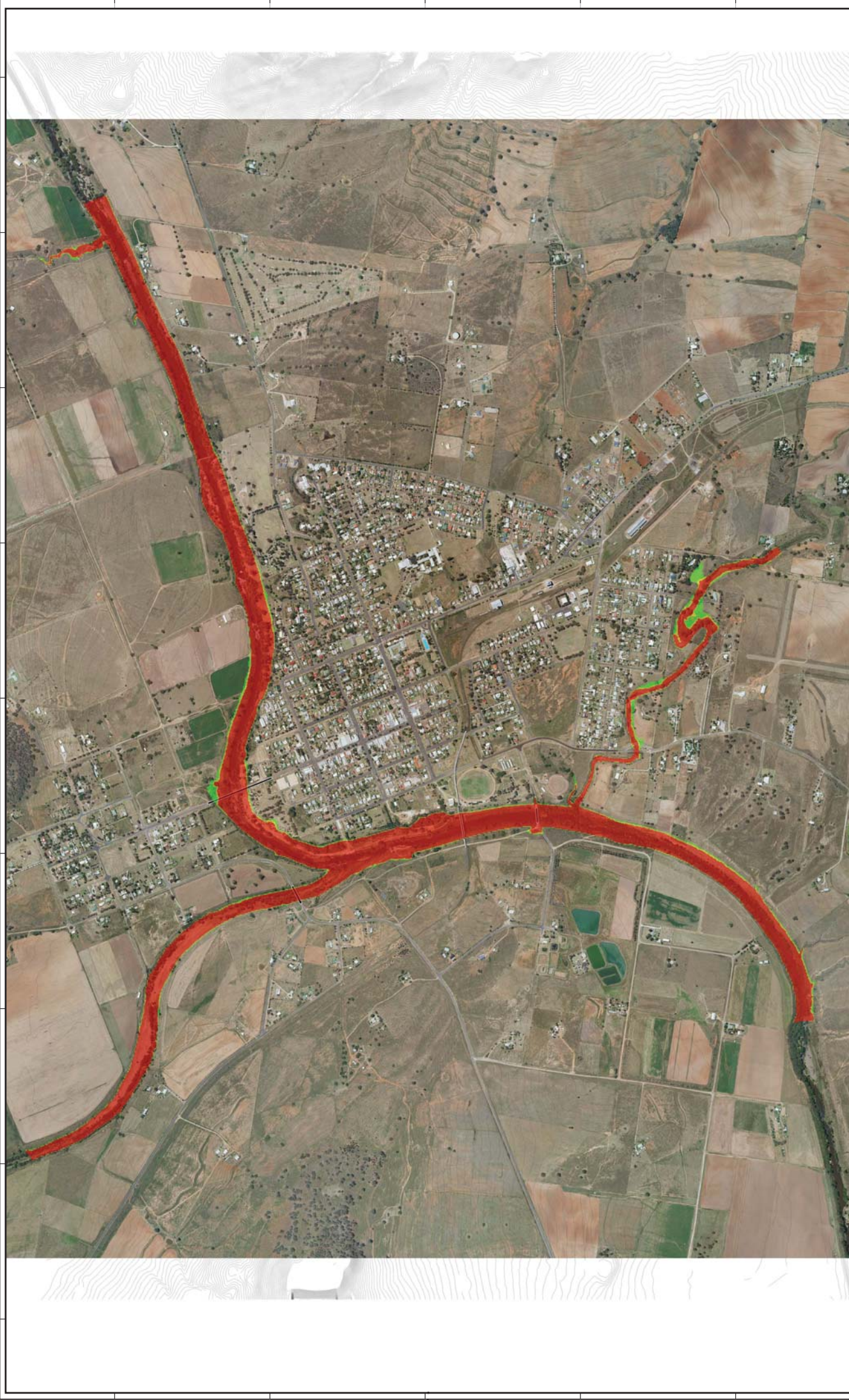


## Appendix F

# Design Flood Results and Mapping







Job Number  
Revision  
Date

Tamworth Regional Council  
**Manilla Flood Study**

22-15815  
A  
01.02.2012

---

Map Projection: Transverse Mercator  
Horizontal Datum: Geocentric Datum of Australia  
Grid: Map Grid of Australia 1964, Zone 58

Metres

0 1 2 3 4 5 6 7 8 9 10

1 - No Flood Hazard  
2 - Minor Flood Hazard  
3 - High Flood Hazard

Topo Contours (1m)

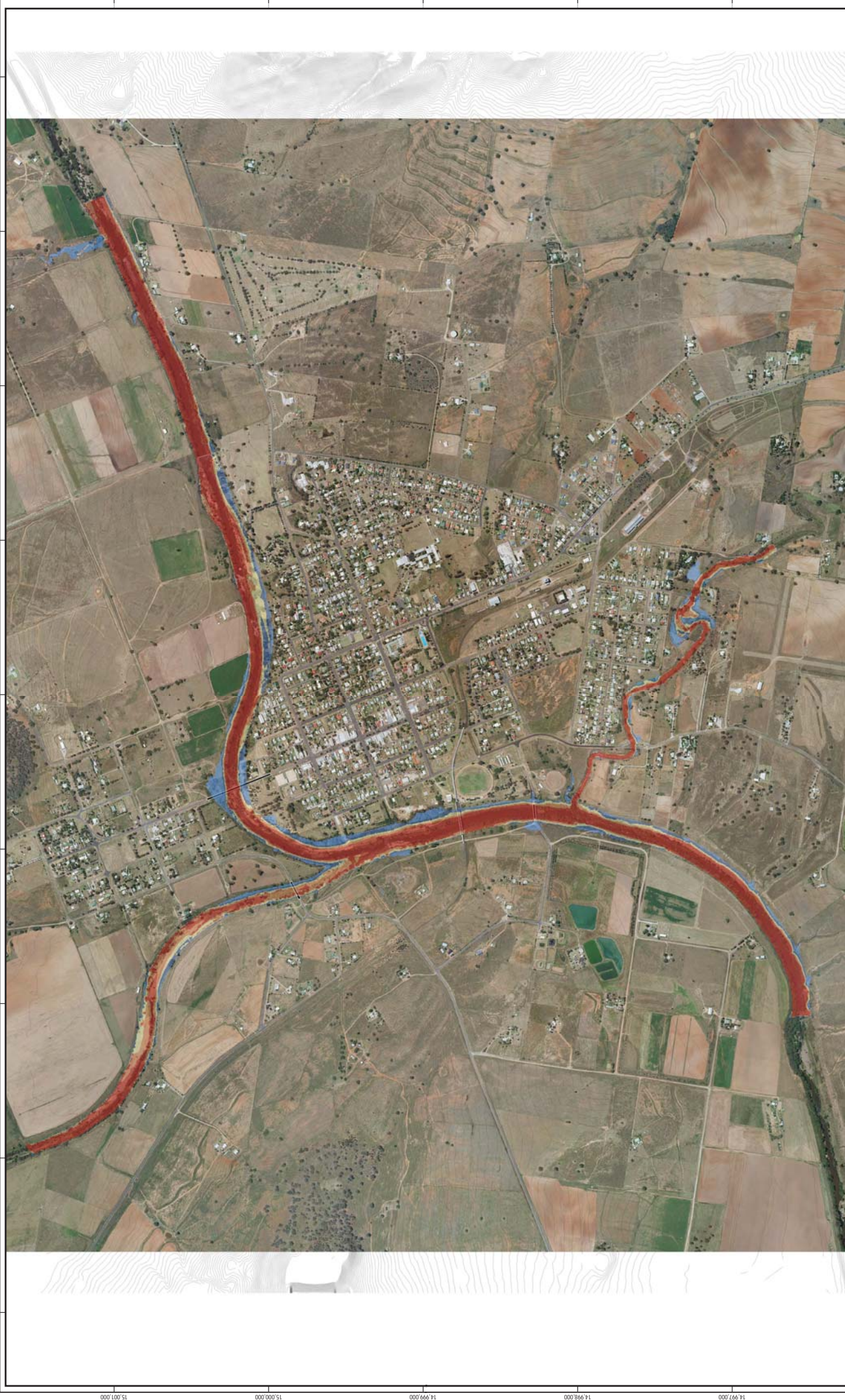
Clients | People | Performance

Flood Hazard Map  
20% AEP Flood Event

Figure F.1.2

<p> <a href="#">N:\AUM\Bourne\Apsr Resources\Local\ESRI\ArcGIS\Utilities..._TemplatesNew..._Templates_Rev0\Utilities..._Templates\A2-LANDSCAPE TECH.mxd</a>          © 2008. While GHD has taken care to ensure the accuracy of this product, GHD and DATA CUSTODIAN(S), make no representations or warranties about its accuracy, completeness or suitability for any particular purpose.          GHD and DATA CUSTODIAN cannot accept liability of any kind (whether in contract, tort or otherwise) for any expenses, losses, damages and/or costs (including indirect or consequential damage) which are or may be incurred as a result of the product being inaccurate, incomplete or unsuitable in any way and for any reason.          Data source: Data Custodian, Data Set Name/Title, Version/Date. Created by:       </p>	<p> <b>T</b> 61 2 6652 5600  <b>F</b> 61 2 6652 5601  <b>E</b> csmail@ghd.com.au  <b>W</b> www.ghd.com.au       </p>
---	--

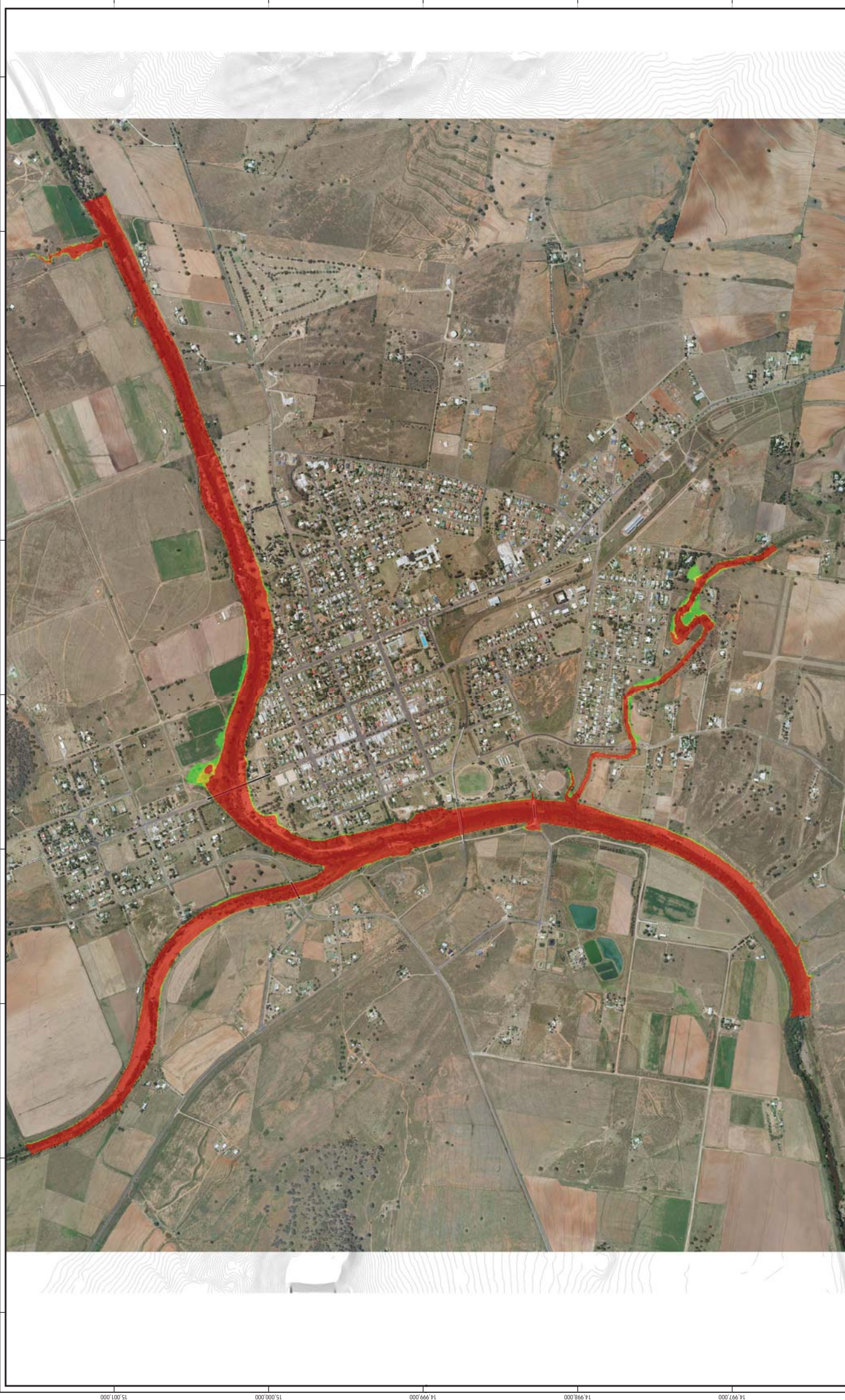












Flood Hazard

- 1 - Low Hazard  
2 - Intermediate Hazard  
3 - High Hazard  
Topo Contours (1m)



CLIENTS | PEOPLE | PERFORMANCE

Tamworth Regional Council  
Manilla Flood Study  
Flood Hazard Map  
10% AEP Flood Event

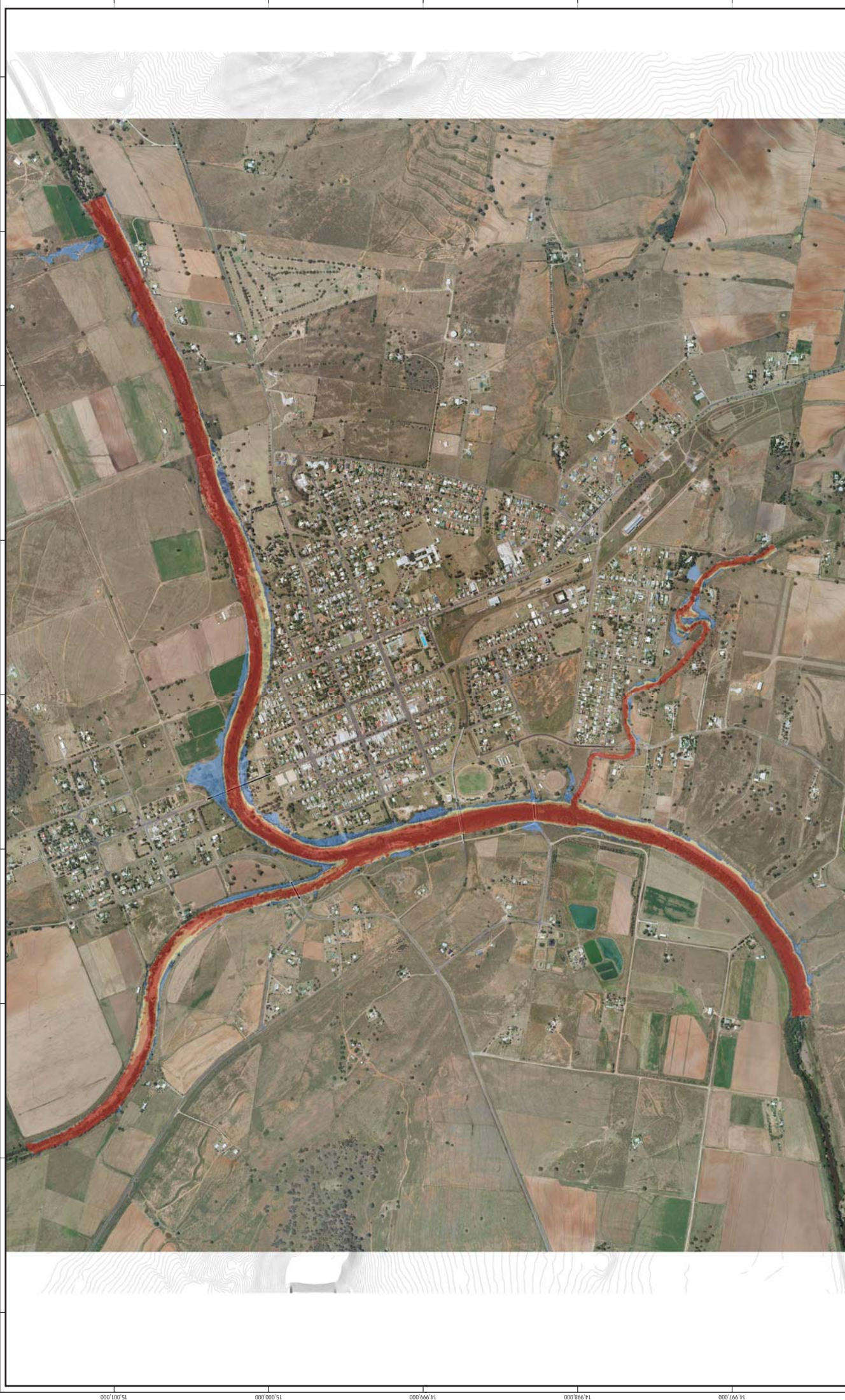
Job Number	22-15815
Revision	A
Date	01.02.2012

Figure F.2.1

Figure F.2.1

MAXIMUS/Maximops Resources/LocalESR/ACSR/SL/Inquiries\_Template.htm, TemplateNew\_Tripod.htm, RCVN/SL/Inquiries\_Template/CHD/AS/LANDSCAPE/ECH/mt  
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Data source: Data Custodian. Data Set Name: Title, Version/Date. Created by:

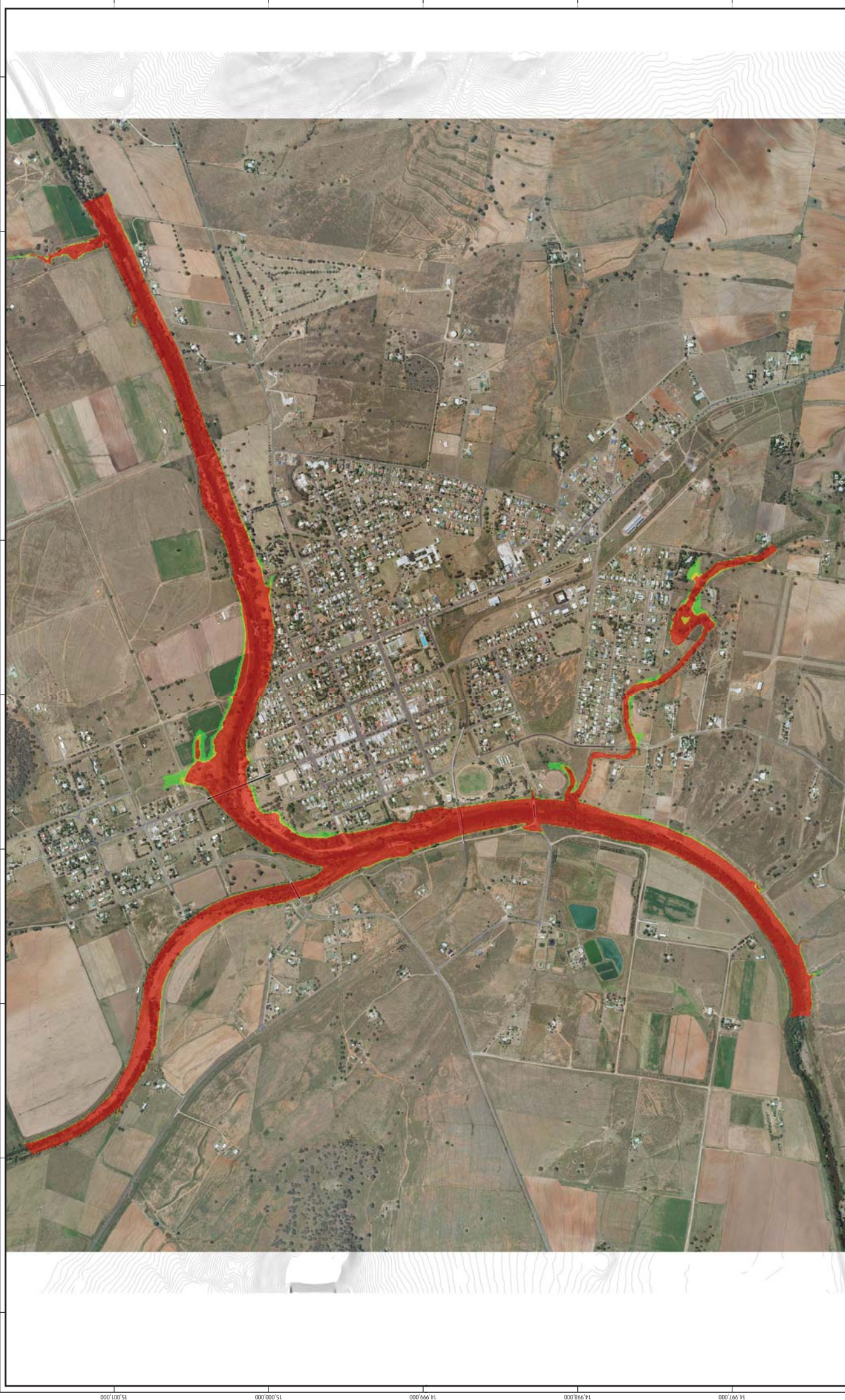












4950000 4970000 4990000 5010000 5030000 5050000 5070000 5090000 5110000 5130000 5150000 5170000 5190000 5210000 5230000 5250000 5270000 5290000 5310000 5330000 5350000 5370000 5390000 5410000 5430000 5450000 5470000 5490000 5510000 5530000 5550000 5570000 5590000 5610000 5630000 5650000 5670000 5690000 5710000 5730000 5750000 5770000 5790000 5810000 5830000 5850000 5870000 5890000 5910000 5930000 5950000 5970000 5990000 6010000 6030000 6050000 6070000 6090000 6110000 6130000 6150000 6170000 6190000 6210000 6230000 6250000 6270000 6290000 6310000 6330000 6350000 6370000 6390000 6410000 6430000 6450000 6470000 6490000 6510000 6530000 6550000 6570000 6590000 6610000 6630000 6650000 6670000 6690000 6710000 6730000 6750000 6770000 6790000 6810000 6830000 6850000 6870000 6890000 6910000 6930000 6950000 6970000 6990000 7010000 7030000 7050000 7070000 7090000 7110000 7130000 7150000 7170000 7190000 7210000 7230000 7250000 7270000 7290000 7310000 7330000 7350000 7370000 7390000 7410000 7430000 7450000 7470000 7490000 7510000 7530000 7550000 7570000 7590000 7610000 7630000 7650000 7670000 7690000 7710000 7730000 7750000 7770000 7790000 7810000 7830000 7850000 7870000 7890000 7910000 7930000 7950000 7970000 7990000 8010000 8030000 8050000 8070000 8090000 8110000 8130000 8150000 8170000 8190000 8210000 8230000 8250000 8270000 8290000 8310000 8330000 8350000 8370000 8390000 8410000 8430000 8450000 8470000 8490000 8510000 8530000 8550000 8570000 8590000 8610000 8630000 8650000 8670000 8690000 8710000 8730000 8750000 8770000 8790000 8810000 8830000 8850000 8870000 8890000 8910000 8930000 8950000 8970000 8990000 9010000 9030000 9050000 9070000 9090000 9110000 9130000 9150000 9170000 9190000 9210000 9230000 9250000 9270000 9290000 9310000 9330000 9350000 9370000 9390000 9410000 9430000 9450000 9470000 9490000 9510000 9530000 9550000 9570000 9590000 9610000 9630000 9650000 9670000 9690000 9710000 9730000 9750000 9770000 9790000 9810000 9830000 9850000 9870000 9890000 9910000 9930000 9950000 9970000 9990000 10010000 10030000 10050000 10070000 10090000 10110000 10130000 10150000 10170000 10190000 10210000 10230000 10250000 10270000 10290000 10310000 10330000 10350000 10370000 10390000 10410000 10430000 10450000 10470000 10490000 10510000 10530000 10550000 10570000 10590000 10610000 10630000 10650000 10670000 10690000 10710000 10730000 10750000 10770000 10790000 10810000 10830000 10850000 10870000 10890000 10910000 10930000 10950000 10970000 10990000 11010000 11030000 11050000 11070000 11090000 11110000 11130000 11150000 11170000 11190000 11210000 11230000 11250000 11270000 11290000 11310000 11330000 11350000 11370000 11390000 11410000 11430000 11450000 11470000 11490000 11510000 11530000 11550000 11570000 11590000 11610000 11630000 11650000 11670000 11690000 11710000 11730000 11750000 11770000 11790000 11810000 11830000 11850000 11870000 11890000 11910000 11930000 11950000 11970000 11990000 12010000 12030000 12050000 12070000 12090000 12110000 12130000 12150000 12170000 12190000 12210000 12230000 12250000 12270000 12290000 12310000 12330000 12350000 12370000 12390000 12410000 12430000 12450000 12470000 12490000 12510000 12530000 12550000 12570000 12590000 12610000 12630000 12650000 12670000 12690000 12710000 12730000 12750000 12770000 12790000 12810000 12830000 12850000 12870000 12890000 12910000 12930000 12950000 12970000 12990000 13010000 13030000 13050000 13070000 13090000 13110000 13130000 13150000 13170000 13190000 13210000 13230000 13250000 13270000 13290000 13310000 13330000 13350000 13370000 13390000 13410000 13430000 13450000 13470000 13490000 13510000 13530000 13550000 13570000 13590000 13610000 13630000 13650000 13670000 13690000 13710000 13730000 13750000 13770000 13790000 13810000 13830000 13850000 13870000 13890000 13910000 13930000 13950000 13970000 13990000 14010000 14030000 14050000 14070000 14090000 14110000 14130000 14150000 14170000 14190000 14210000 14230000 14250000 14270000 14290000 14310000 14330000 14350000 14370000 14390000 14410000 14430000 14450000 14470000 14490000 14510000 145

Map Projection: Transverse Mercator  
Horizontal Datum: Geocentric Datum of Australia  
Grid: Map Grid of Australia 1994, Zone 56

Metres

3 - High Hazard

Topo Contours (1 m)

CLIENTS | PEOPLE | PERFORMANCE

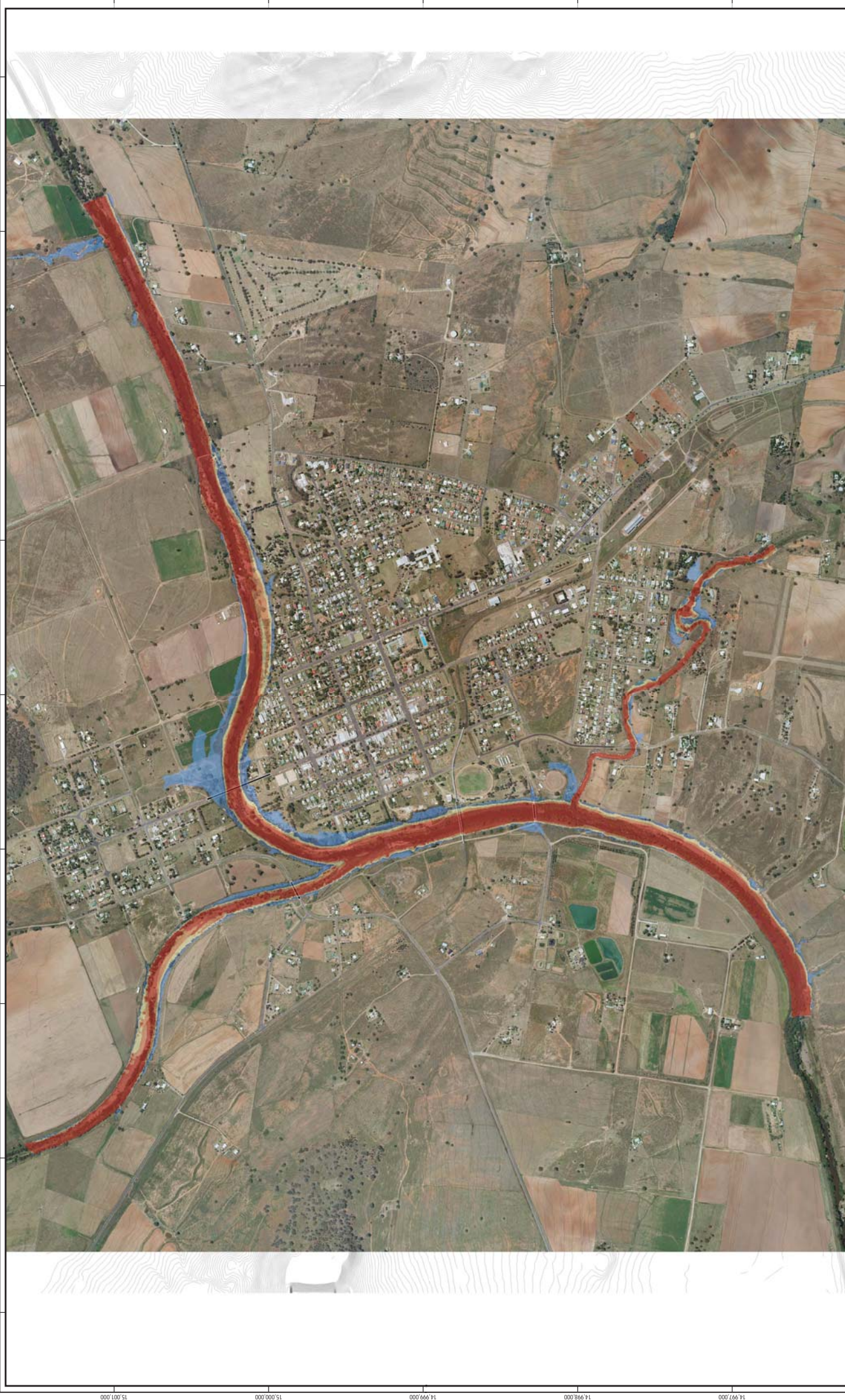
Flood Hazard Map

5% AEP Flood Event

Figure F.3.2

[illegible]

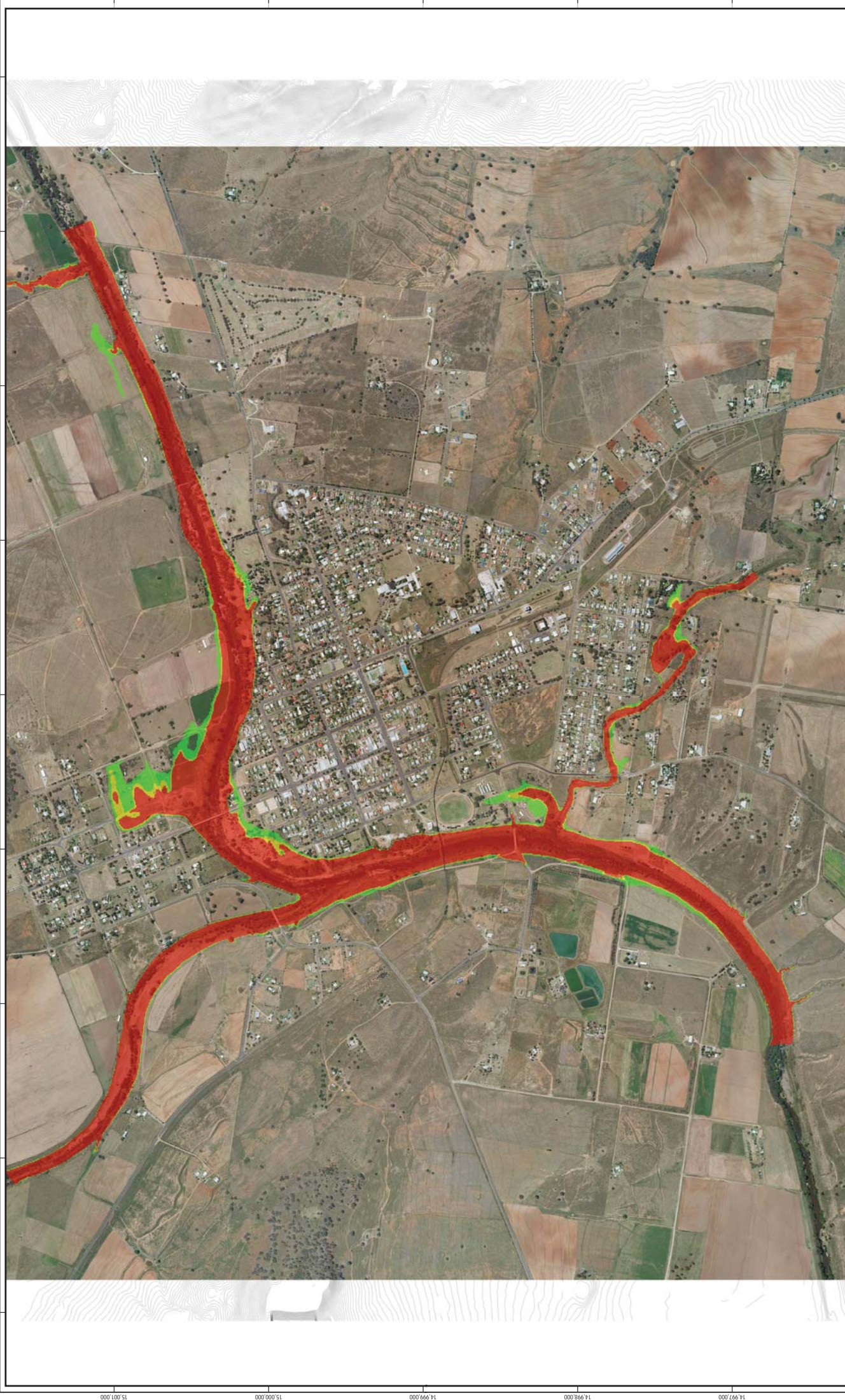










[illegible]

Map Projection: Transverse Mercator  
Horizontal Datum: Geocentric Datum of Australia  
Grid: Map Grid of Australia 1994, Zone 56

Metres

3 - High Hazard

Topo Contours (1m)

CLIENTS | PEOPLE | PERFORMANCE

Flood Hazard Map

2% AEP Flood Event

Figure F.4.1

[illegible]









Job Number 22-15815  
Revision A  
Date 01.02.2012

Tamworth Regional Council  
Manilla Flood Study  
Flood Map  
1% AEP Flood Event

Figure F.5.1



CLIENTS | PEOPLE | PERFORMANCE

Flood Extent and Depth (m)  
0.16 - 0.5  
0.5 - 1.0  
1.0 - 1.5  
1.5 - 2.0

Flood Level Contours (mAHD)  
2.0 - 3.0  
3.0 - 5.0  
5.0 - 10.0  
10.0 - 20.0

Topo Contours (1m)

1:15,000  
0 75 150 300 450 600  
Metres

Map Projection: Transverse Mercator  
Horizontal Datum: Geocentric Datum of Australia  
Grid: Map Grid of Australia 1994, Zone 56









14,997,000

15,000,000

15,001,000

496,000

497,000

498,000

499,000

500,000

501,000

502,000

503,000

504,000

14,997,000

14,998,000

14,999,000

15,000,000

15,001,000

496,000

497,000

498,000

499,000

500,000

501,000

502,000

503,000

504,000

14,997,000

14,998,000

14,999,000

15,000,000

15,001,000

496,000

497,000

498,000

499,000

500,000

501,000

502,000

503,000

504,000

14,997,000

14,998,000

14,999,000

15,000,000

15,001,000

496,000

497,000

498,000

499,000

500,000

501,000

502,000

503,000

504,000

14,997,000

14,998,000

14,999,000

15,000,000

15,001,000

496,000

497,000

498,000

499,000

500,000

501,000

502,000

503,000

504,000

1:15,000

0 75 150 300 450 600

Meters

Map Projection: Transverse Mercator

Horizontal Datum: Geocentric Datum of Australia

Grid: Map Grid of Australia 1994, Zone 56

Manilla 100yr Peak Velocity (m/s)

0 - 0.5

0.5 - 1.0

1.0 - 1.5

1.5 - 2.0

Greater than 2

Topo Contours (1m)

CHD

CLIENTS | PEOPLE | PERFORMANCE

Tamworth Regional Council

Manilla Flood Study

Flood Velocity Map

1% AEP Flood Event

Job Number 22-15815

Revision A

Date 01.02.2012

Figure F.5.3

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F 61 2 8662 5601

E cfsmail@ghd.com.au

W www.ghd.com.au

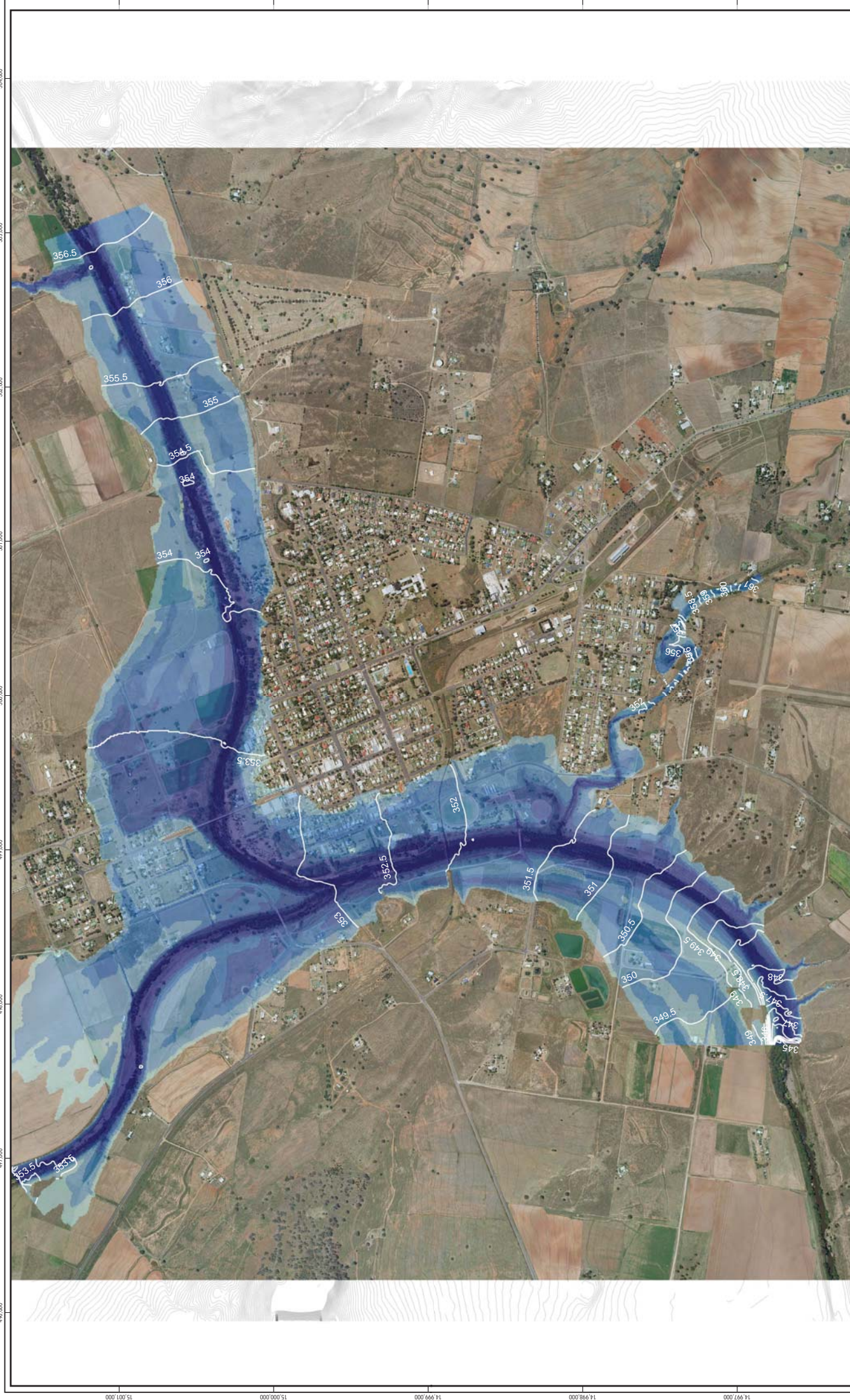
© 2008 GHD and DATA CUSTODIAN cannot accept liability of any kind (whether in contract, tort or otherwise) for any expenses, losses, damages and/or costs (including indirect or consequential damage) which are or may be incurred as a result of the product being inaccurate, incomplete or unsuitable in any way and for any reason.

Data source: Data Custodian, Data Set Name/Title, Version/Date. Created by:









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Map Projection: Transverse Mercator  
Horizontal Datum: Geocentric Datum of Australia  
Grid: Map Grid of Australia 1994, Zone 56

0.16 - 0.5

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1.0 - 1.5

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Flood Extent and Depth (m)

0.16 - 0.5

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1.0 - 1.5

1.5 - 2.0

Flood Level Contours (mAHD)

Topo Contours (1m)

2.0 - 3.0

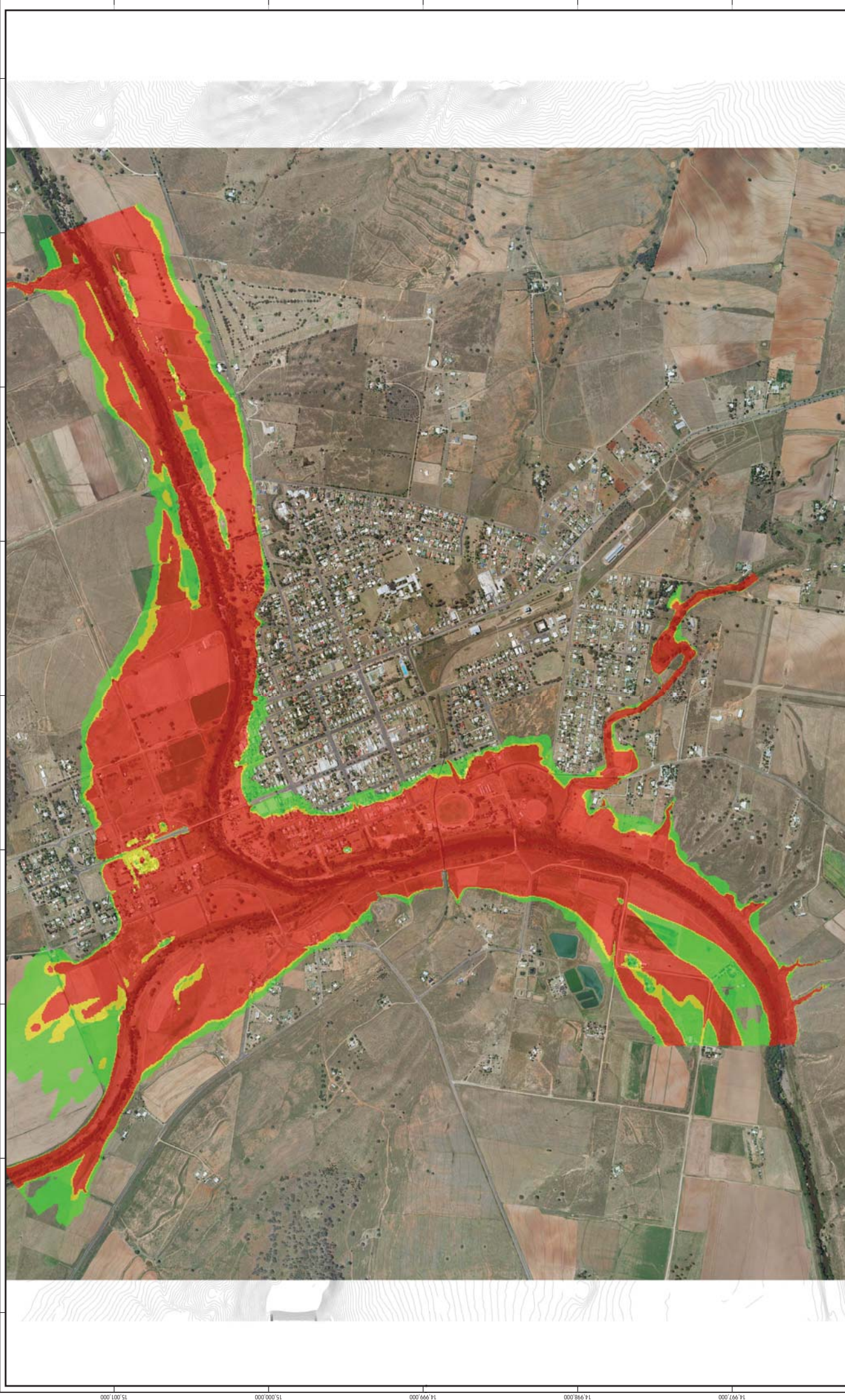
3.0 - 5.0

5.0 - 10.0

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		Job Number 22-15815  Revision A  Date 02.02.2012
<b>Tamworth Regional Council</b> <b>Manilla Flood Study</b>		

Map Projection: Transverse Mercator  
Horizontal Datum: Geocentric Datum of Australia  
Grid: Map Grid of Australia 1994, Zone 56

Metres

3 - High Hazard  
Topo Contours (1m)

CLIENTS | PEOPLE | PERFORMANCE

Flood Hazard Map  
0.5% AEP Flood Event

Figure F.6.2

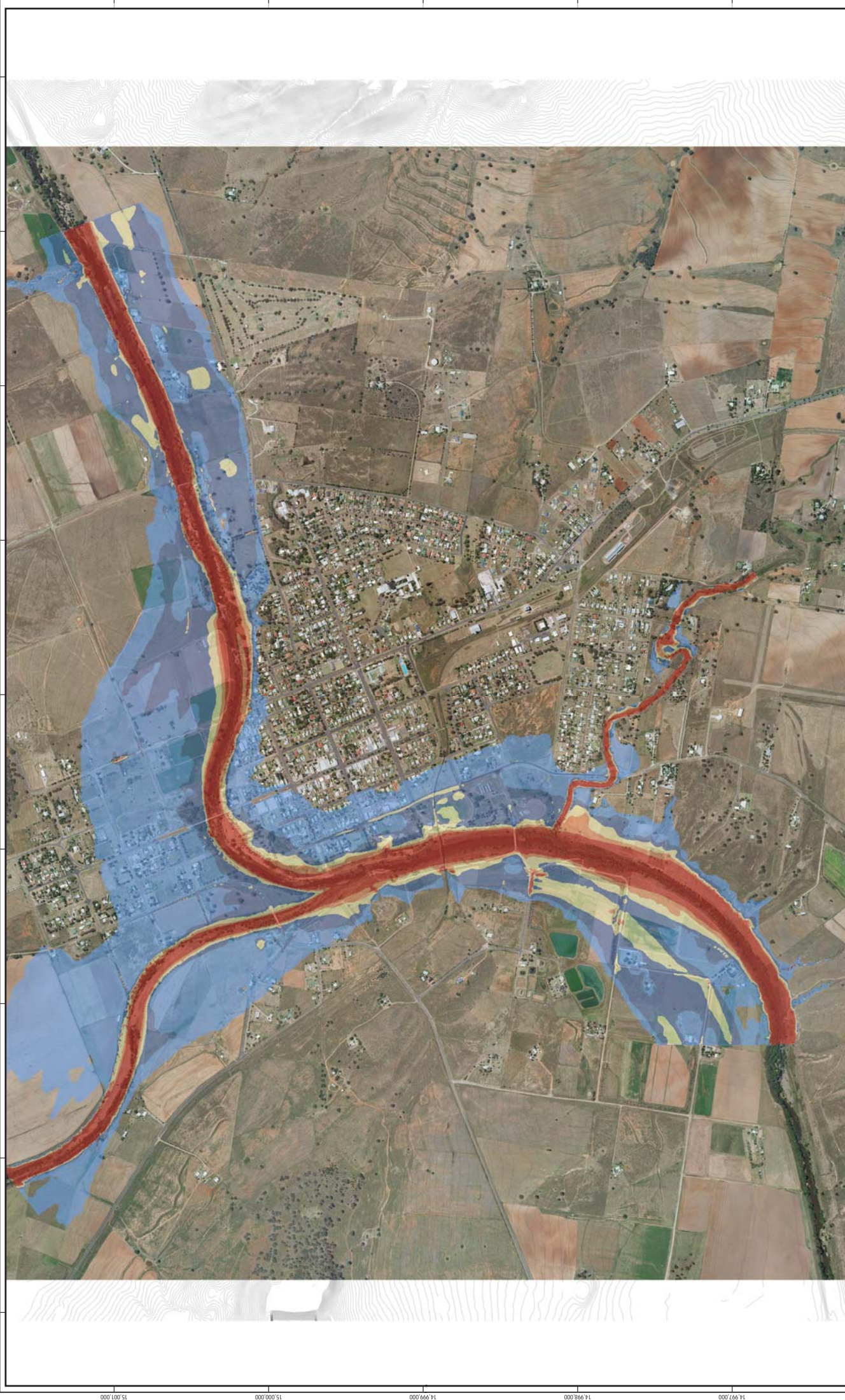
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Data source: Data Cession, Data Set Name>Title, Version/Date. Created by:

230 Harbour Drive Coffs Harbour NSW 2450 Australia	T	61 2 6652 5600	F	61 2 6652 5601	E	clsmail@ghd.com.au	W	www.ghd.com.au
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## Appendix G

# Sensitivity Assessments







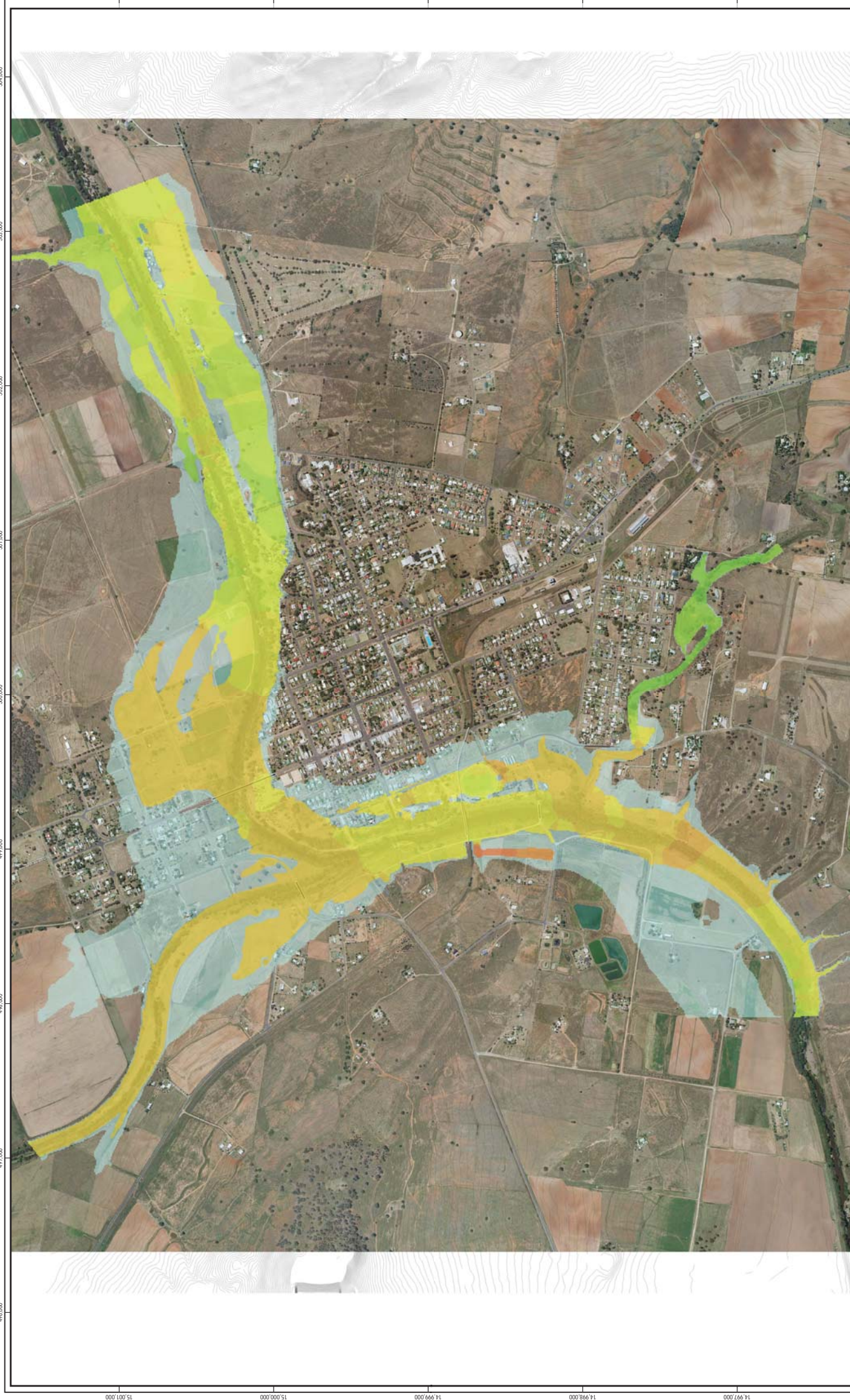












Job Number 22-15815  
Revision A  
Date 06.02.2012

Tamworth Regional Council  
Manilla Flood Study

Climate Change Sensitivity (20%)  
1% AEP Flood Event

CHD  
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220 Harbour Drive Coffs Harbour NSW 2450 Australia  
T 61 2 6652 5600 F 61 2 6652 5601 E dsmai@ghd.com.au W www.ghd.com.au

Change in Flood Depth (m)

-0.02 - 0	0.4 - 0.6	1.2 - 1.4	2.0 - 2.2
0 - 0.2	0.6 - 0.8	1.4 - 1.6	2.2 - 2.4
0.2 - 0.4	0.8 - 1	1.6 - 1.8	2.4 - 2.6
	1.0 - 1.2	1.8 - 2	

Additional Inundation Area  
Topo Contours (1m)

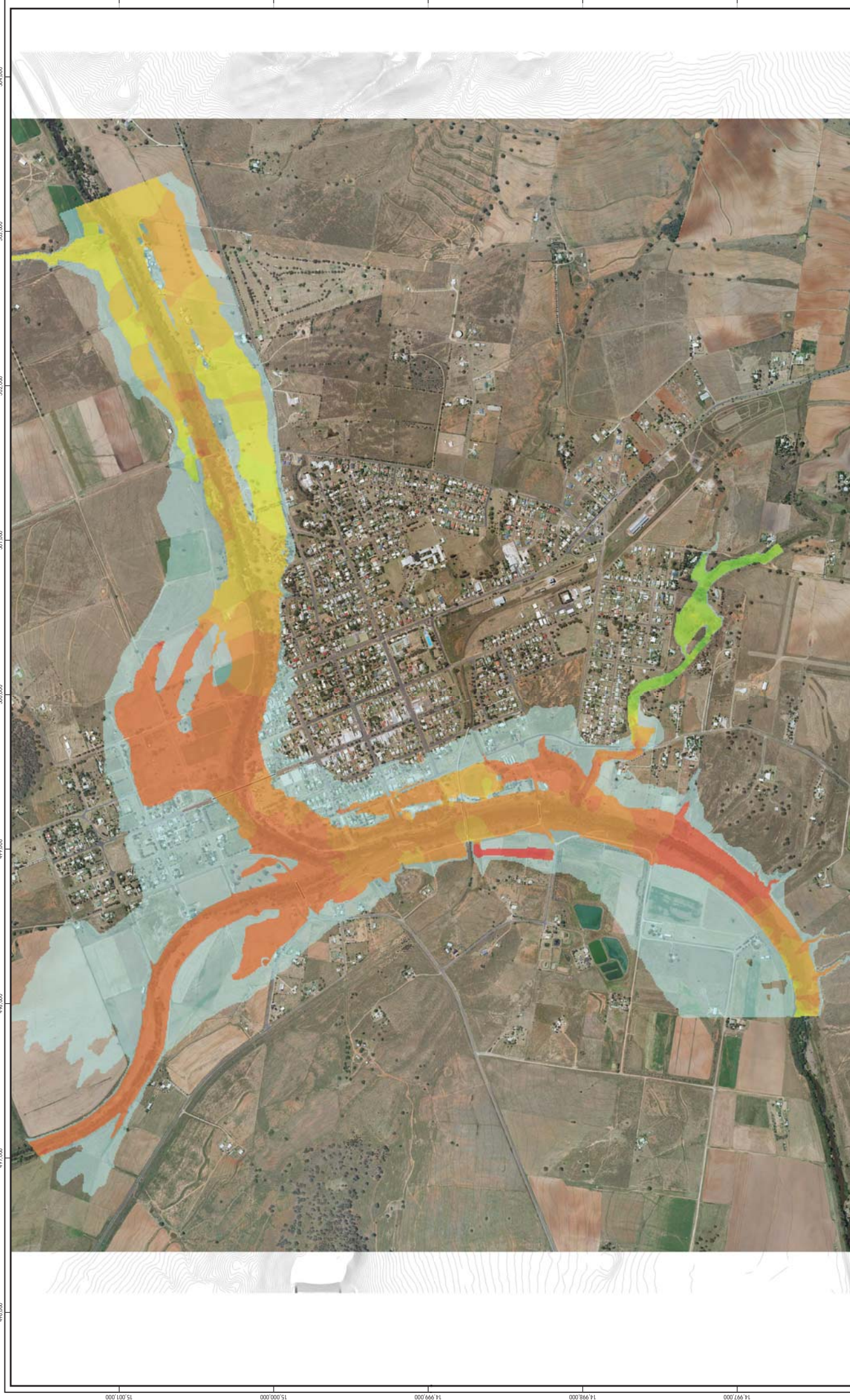
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Map Projection: Transverse Mercator  
Horizontal Datum: Geocentric Datum of Australia  
Grid: Map Grid of Australia 1994, Zone 56

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Figure G.5





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## Appendix H

# Flood Planning Map









## Appendix I

# QA Documentation

2/13 Hill Street  
 TAMWORTH NSW 2340

please address all correspondence to  
 PO Box 1402 TAMWORTH NSW 2340

general: okbaxter@baxtergeo.com  
 writers email: peter@baxtergeo.com

Our Ref: 0486

*File Location: Z:\0486 Manilla Flood Study\Doc\0486-120130 GHD Manilla Quality Doc.docx*

Your Ref: Manilla Flood Study

30<sup>th</sup> January 2012.

The General Manager  
 Tamworth Regional Council  
 P.O. Box 555  
 TAMWORTH NSW 2340

Dear Sir,

Re: Quality Control Manilla Flood Study – Joint Venture Aerometrex, Baxter Geo & GHD

## Survey Control

### Establish Survey Control

A SCIMS search was undertaken on 5<sup>th</sup> September 2011 to locate reliable Survey Control in the town of Manilla. PM142521 each being of Class A Order 1 Horizontal and Class B Order 2 Vertical was chosen as the control station for all survey work.

A GNSS base station and eccentric check station were established on the SCIMS co-ordinates and the primary control measured using two rovers, each on a range pole with bi-pod set at different heights. This methodology provides an independent check on heights. Measurements were made to PM 12834 (Class A Order 1 Horizontal – Class LC Order L3 Vertical), PM74722 (Class A Order 1 Horizontal – Class B Order 2 Vertical), PM117815 (Class A Order 1 Horizontal – Class B Order 2 Vertical), and PM151659 (Class A Order 1 Horizontal – Class B Order 2 Vertical). There was good agreement with the SCIMS data base and our initial calibration survey.

Mark	Mean Ovsevation			SCIMS			dE	dN	dH
	Easting	Northing	AHD	Easting	Northing	AHD			
PM142521	280223.213	6596881.885	359.132	280223.213	6596881.885	359.132			
PM 151659	280375.589	6595596.884	370.549	280375.579	6595596.879	370.552	-0.009	-0.005	0.003
PM12834	281739.410	6597226.378	352.586	281739.407	6597226.383	352.600	-0.003	0.005	0.014
PM 74722	282919.498	6595107.700	371.796	282919.504	6595107.704	371.776	0.006	0.004	-0.020
PM 117815	281795.090	6594655.660	357.603	281795.078	6594655.671	357.628	-0.012	0.011	0.026

The largest difference shown at PM117815 is over a distance of 2725.23m and represents an accuracy of 1:170,327 in distance and 1:104,815 in height.

### Eccentric Control Station

The co-ordinates of the eccentric control station were documented and on each subsequent survey a check measurement was taken to the eccentric station at the beginning and end of each day. The check co-

ordinates were compared to establish no errors were made in setting up the base station each day and that the base station had not been disturbed or tampered with during the days observations.

### **Establish Photo Control**

Aerometrex required photo control points to be established in six areas. An RTK survey was undertaken with two rovers set at different heights. A minimum of two control points (each determined by observations using two rovers) were taken in each of the six areas. This gave a redundancy of 100% for Aerometrex.

### **Establish Quality Control**

During the control survey a GNSS rover was fixed to the top of the car and random measurements at an approximate interval of 50m spacing.

Control Data was forwarded to Aerometrex for processing.

### **Processed Photogrammetric Data**

Aerometrex supplied digital data which was read into AutoCAD Civil 3D. The data was processed and a Digital Terrain Model created.

### **Checking and Quality Control**

The random heights were placed in the model and a data set of heights on the DTM for those points calculated. A total of 208 points were analysed. Of these points 133 points agreed to better than 0.15m. A further 57 points were in the range 0.15m to 0.3m. The remaining points were analysed visually to determine why larger difference were calculated. It is our conclusion the larger differences are due to the location of the check points being inappropriate, eg on the crown of a dirt road etc.

It is our opinion the data supplied by Aerometrex and the subsequent DTM is superior to what could be achieved by conventional ground survey. The anomalies mentioned above are not considered significant in the overall accuracy of the DTM for the purpose calculating water levels over a large area. A large number of points with a lower accuracy (X, Y, Z) and breaklines lines will provide a more superior DTM than a low number with highly accurate points (X, Y, Z). It is our opinion the aerial photography and the subsequent DTM are suitable for the purpose.

### **Additional Information**

Additional survey data was required to be collected by ground survey. This data included existing flood heights and bridges. This data was collected by GNSS and total station measurements. The Leica TPS 1200 instrument used to collect the data allows all total station measurements to be collected in MGA 56 in the same file as the GNSS data. Each total station setup was compared to control points in the GNSS data base at the beginning of each observation epoch.

A separate file containing the 3D models bridges was created and supplied to GHD.

### **Supply of data to GHD**

The electronic data was supplied by FTP to GHD.

### **Floor Level Survey**

A data set containing the 1:100 and PMF flood lines was provided to Baxter Geo by GHD. This data was loaded into the model and a series of field sheets produced to identify the floor levels required to be measured.

Floor levels were determined using GNSS and a laser level. The system was checked prior to field survey to ensure accuracy. The initial check showed what we believe to be a collimation error in the laser level. A new laser level was obtained and a further field check undertaken to confirm accuracy. This check proved satisfactory and the field survey was then undertaken using the previous GNSS control station and the eccentric control station. During the course of the floor level survey, measurements were taken to known



points and the values of the GNSS / laser level survey compared. All results showed good agreement. We are of the opinion our levels determined by this method are good to 0.05m or better.

The greatest source of uncertainty in determining the floor levels is picking a point on the outside of the building that is representative of the floor level inside. Our instruction was to determine the floor levels within +/- 0.1m. We are of the opinion that this criteria has been satisfactorily met.

Each measurement with the laser level was measured in metres and inches. Each value was entered into the data recorder. The observed inches were converted to metres on a handheld calculator and compared to the reading in metres entered into the data recorder before the data was stored. This methodology provided quality control in the field.

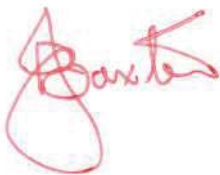
The GNSS and level observations were downloaded and converted to an Excel spreadsheet. The processed data was then read into the Civil 3D model and a visual check made against the aerial photography. This was then checked against a data file of Manilla showing the cadastral layout and street numbers supplied by Tamworth Regional Council. A number of amendments to the addressing collected in the field were required to be made to be consistent with Council's data set.

The processed floor levels were exported to an Excel spreadsheet and this was supplied to GHD as a Excel spreadsheet and a pdf print of the file.

## Personnel

All surveys were undertaken by the undersigned with the assistance of Dan Whale who is a competent field assistant and a qualified town planner.

Yours faithfully



**Peter J Baxter** *B.Surv M.I.S(NSW) S.S.S.I.*  
*Registered Surveyor / Licensed Strata Managing Agent*  
for Baxter Geo Consulting Pty. Ltd.



## GHD JFlow QA Documentation

2215816	▼ TRC - Barraba Flood Study (T074/2011)	James McPherson	Rainer Berg	93,237	Active	28/02/2012
	▼ DELIVERABLE: Calculations - Barraba RORB model check : Completed on 15/05/2012					
	▼ REVIEW: Online Review - Michelle S Kan : Completed on 01/03/2012					
	CHECKSHEET: Calculations - Michelle S Kan : Completed on 01/03/2012					
	▼ DELIVERABLE: Calculations - Barraba Tuflow Model Check : Completed on 15/05/2012					
	▼ REVIEW: Online Review - Michelle S Kan : Completed on 01/03/2012					
	CHECKSHEET: Calculations - Michelle S Kan : Completed on 01/03/2012					
	▼ DELIVERABLE: Reports - Barraba Flood Study Report : Due on 01/03/2012					
	REVIEW: Authorised Person to Sign - Rainer Berg : Completed on 15/05/2012					
	REVIEW: Authorised Person to Sign - Rainer Berg : Completed on 15/05/2012					
	▼ JOB REVIEW: 15% Job Review : Completed on 31/08/2011 by Rainer Berg					
	CHECKSHEET: 15% Job Review - by Rainer Berg due 09/09/2011 - completed on 31/08/2011					
	Risk: Not meeting completion date of 28 Feb 2012 - Complete					
	Risk: Package to include three studies. Will need high level of support and success will depend on availability of key personnel - Complete					
	Risk: Project Environmental Impact - Complete					
	Risk: Survey information incompatibility - Complete					
	SUBCONSULTANT: Aerometrex Pty Ltd - PI Current; PL Current; Registration None					
	SUBCONSULTANT: Baxter Geo Consulting Pty Ltd - PI Current; PL Current; Registration None					
2215815	▼ TRC - Manila Flood Study (T073/2011)	James McPherson	Rainer Berg	98,807	Active	28/02/2012
	▼ DELIVERABLE: Calculations - Catchment Delineation : Completed on 15/05/2012					
	REVIEW: Signed Check to be Kept on File - Samuel L Douglas : Completed on 29/02/2012					
	▼ DELIVERABLE: Calculations - RORB model Check : Completed on 15/05/2012					
	▼ REVIEW: Online Review - Michelle S Kan : Completed on 01/03/2012					
	CHECKSHEET: Calculations - Michelle S Kan : Completed on 01/03/2012					
	▼ DELIVERABLE: Calculations - tuflow model : Completed on 15/05/2012					
	▼ REVIEW: Online Review - Michelle S Kan : Completed on 01/03/2012					
	CHECKSHEET: Calculations - Michelle S Kan : Completed on 01/03/2012					
	▼ DELIVERABLE: Reports - Flood Study Report : Due on 01/03/2012					
	REVIEW: Authorised Person to Sign - Rainer Berg : Completed on 15/05/2012					
	REVIEW: Authorised Person to Sign - Rainer Berg : Completed on 15/05/2012					
	▼ JOB REVIEW: 15% Job Review : Completed on 31/08/2011 by Rainer Berg					
	CHECKSHEET: 15% Job Review - by Rainer Berg due 09/09/2011 - completed on 31/08/2011					
	JOB REVIEW: Project Review Group (PRG) Review : Completed on 15/05/2012 by Rainer Berg					
	Risk: Environmental impact - Complete					
	Risk: Not meeting completion date of 28 Feb 2012 - Complete					
	Risk: Survey information incompatibility - Complete					
	SUBCONSULTANT: Aerometrex Pty Ltd - PI Current; PL Current; Registration None					
	SUBCONSULTANT: Baxter Geo Consulting Pty Ltd - PI Current; PL Current; Registration None					

15 May 2012

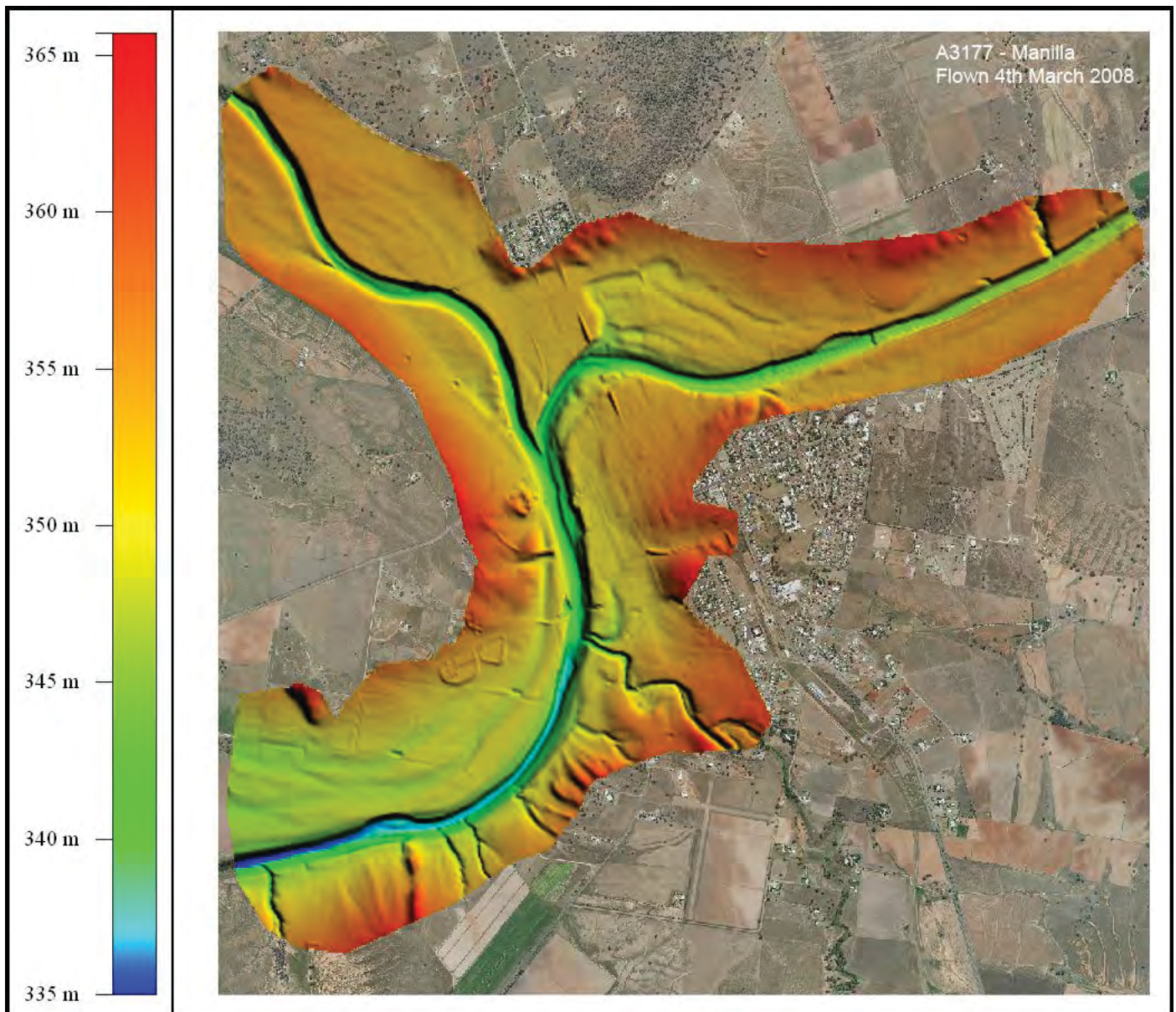
Dr. Rainer Berg  
GHD  
230 Harbour Drive  
Coffs Harbour NSW 2450

Dear Rainer,

Please find below survey specifications, accuracy statements & quality control check sheets for the aerial survey Aerometrex conducted over the township of Manilla in October 2011.

**Survey Reference: A3177 Manilla**

**Overview:**





## Quality Control Sheet:

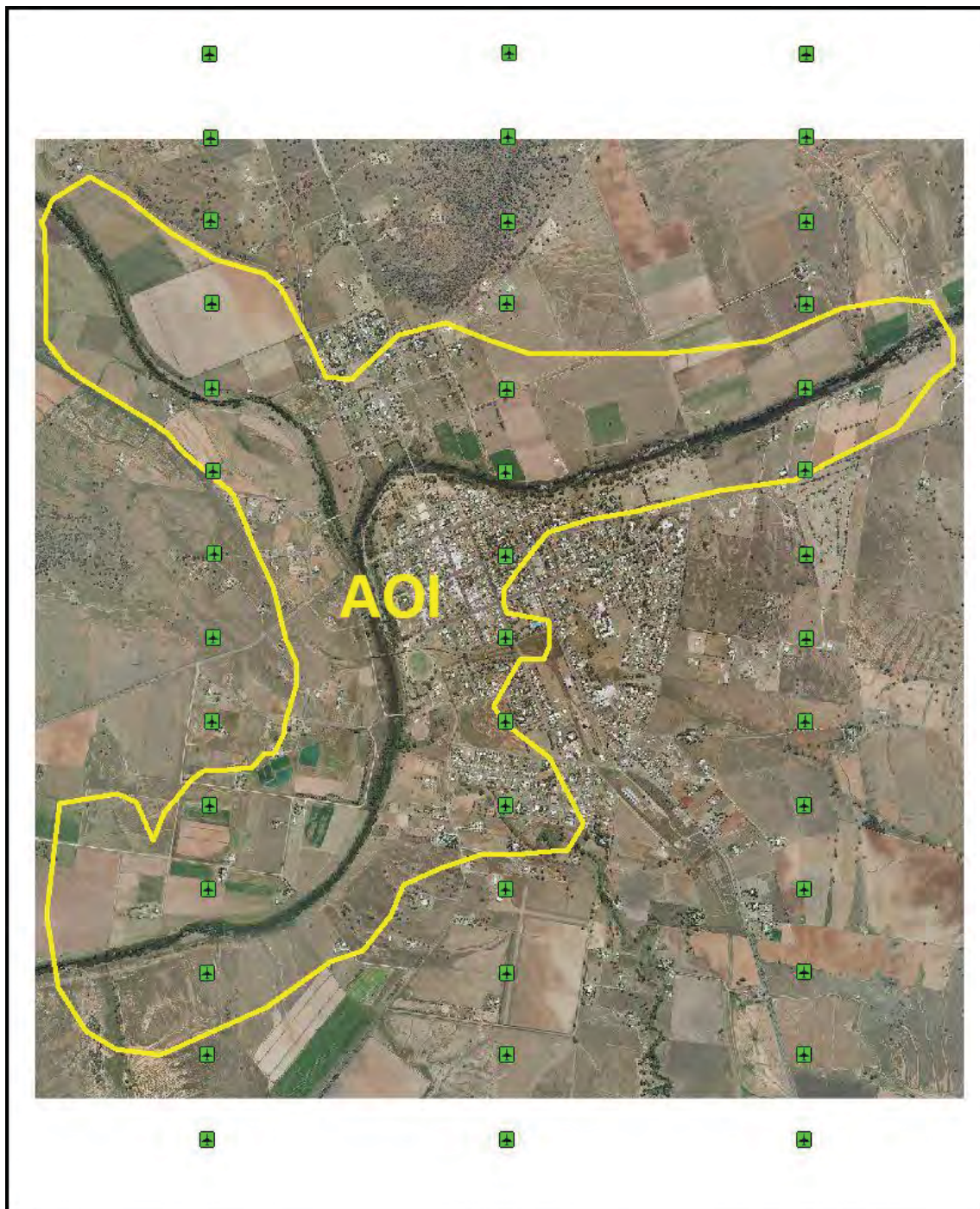
E:\AI Projects\A3177-Baxter\Manilla\_Barraba\_20cm\*\_DTMaster\_Data

NAME	DATE	TIME
GS	Monday 26/9/11	7.25 hrs
GS	27/9/11	4.00 hrs
Ines		
Petar	20/10/11 21/10/11	Pick up buildings 8.5 hrs
Kate	CHECK	9h

- FLOOD MAPPING
- LIMITED FEATURE
- \* 4 POINTS @ BASE OF EACH BUILDING REQUIRED
- USE LEVELS AS PER TEMPLATE IN DTMASTER
- PARTICULAR CARE AROUND CREEK ENBANKMENTS REQUIRED.
- WORK TO OTERMOST BUFFER OF SHAPE
- EXPORT AS DXF (INCLUDE FORMLINES)

## Survey Details:

Flown – 5<sup>th</sup> March 2008  
 Plane – Conquest VH-VEW  
 Capture Height – Approx. 8800 Feet  
 Number of Runs – 3  
 Number of Frames - 45  
 Direction of Runs – North / South  
 Ground Sample Distance – 20cm  
 Forward frame overlap: 70%  
 Side frame overlap: 30%



#### Sensor Details:

Sensor – Vexcel Ultracam D, Large Format Mapping Camera – UCD-SU-1-0047

Focal Length: 105.200mm

Image Extent: (-33.75, -51.75)mm (33.75, 51.75)mm

Image Format: 7500 pixels (width) x 11500 pixels (height)

Pixel Size: 9.000 micron x 9.000 micron

PPA: X\_ppa = -0.000 mm , Y\_ppa = 0.180 mm

Please refer to included document “Calib-Report\_0047\_V10\_short.pdf”

The sensor used in this survey was fully calibrated in Dec 2006.





## Flight Report:



## AIR PHOTOGRAPHY REPORT

Times in UTC

Camera Type - Vexcel UltraCam		Calibrated FL - 105.2mm				Date - 05MAR08A		Possible Images - 1337		
Capture Time Interval 3.9s		GPS Base Station Tamworth ARP				Pilot - George Scott		Navigator - John Murphy		
Max Aircraft Speed 220kts		Data Download Time -				Aircraft Registration - VH-VEW		Aircraft Time - 042200 - 050203		
JOB NO.	PROJECT	GSD	AMSL(ft)	RUN No.	DIRECTION	START_SU	START_Wpt	END_SU	END_Wpt	Comments
A2705	Tamworth	20	9635	26	181	78935	010	78944	001	OK
				25	001	78945	001	78985	041	OK
				24	181	78986	041	79026	001	OK
			9730	1	091	79027	001	79096	070	OK
				2	271	79097	070	79166	001	OK
				3	091	79167	001	79236	070	OK
				4	271	79237	070	79306	001	OK
				5	091	79307	001	79376	070	OK
				6	271	79377	070	79446	001	OK
				7	091	79437	001	79516	070	OK
			9665	8	271	79517	070	79586	001	OK
				9	091	79587	001	79656	070	OK
				10	271	79657	070	79726	001	OK
				11	091	79727	001	79796	070	OK
				12	271	79797	070	79866	001	OK
			9305	22	001	79867	001	79878	012	OK
				23	181	79879	012	79890	001	OK
			8815	19	181	79891	014	79904	001	OK
				20	001	79905	001	79918	014	OK
				21	181	79919	014	79932	001	OK

## Survey Accuracy:

The table below defines the accuracy Aerometrex is able to quote and obtain from this survey. The highest quality equipment combined with hundreds of surveys of experience allows Aerometrex to confidently quote these accuracies. Numerous cross checking techniques are employed, additional surveys over the Manilla township were used in one checking procedure.

Aerometrex has considerable resources and time over the past 10 years in research and development proving its ability to conform to specified accuracies.

Accuracy	20cm pixel resolution
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Horizontal (Point)	+/- 0.20m RMSE
Horizontal (Ortho)	+/- 0.40m RMSE
Vertical	+/- 0.17m (68% c.i., 1 $\sigma$ )
(With BGC ground survey inputs)	+/- 0.34m (95% c.i., 2 $\sigma$ )

The above vertical accuracies were obtained by incorporating ground control supplied by Baxter Geo Consulting.

**Supplied data details:**

10m spacing gridded DTM with breaklines.  
 10m spacing gridded DTM with breaklines converted to breakpoints every 1m.  
 1m Contours in dxf format.  
 4 DTM Points around the base of all buildings.

Please do not hesitate to contact me if any more information is required.

Yours sincerely,



**Todd Dunow**  
**Senior Account Manager**  
**Aerometrex Pty Ltd.**



**Ph. (08) 8361 3111**  
**Mobile: 0419 712 897**




## GHD

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Coffs Harbour NSW 2450  
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## Document Status

Rev No.	Author	Reviewer		Approved for Issue		
		Name	Signature	Name	Signature	Date
EARLYDRAFT	S Douglas/R Berg	R Berg		R Berg		17/01/2012
DRAFT	S Douglas/R Berg	R Berg		R Berg		07/02/2012
FINAL DRAFT	S Douglas/R Berg	R Berg		R Berg		27/02/2012
FINAL	S Douglas/R Berg	R Berg		R Berg		25/04/2012